Assessment of Liquefaction potential - Deterministic and Probabilistic Approach

A THESIS SUBMITTED IN PARTIAL FULFILLMENT OF THE REQUIREMENTS FOR THE DEGREE OF

Master of Technology In Structural Engineering

By

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I hereby declare that the project work entitled **"Assessment of Liquefaction potential - Deterministic and Probabilistic Approach"** submitted to Department of Civil Engineering, DTU is a record of an original work done by **Rahul Verma** under the guidance of **Mr. G.P.Awadhiya**, Associate Professor, Department of Civil Engineering, DTU, and this project work has not performed the basis for the award of any Degree or diploma/fellowship and similar project, if any.

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ABSTRACT

Seismic soil liquefaction continues to be a challenging problem, and attracts considerable attention from researchers all around the world. The term liquefaction has been used to define various different aspects of shear strength reduction, such as flow failure or cyclic softening.

The studies presented herein are directed towards the development of improved SPT-based correlations for both probabilistic and deterministic evaluation of potential for "triggering" or initiation of seismically-induced soil liquefaction.

This studies presents new correlations for assessment of the likelihood of initiation (or "triggering") of soil liquefaction and provide greatly reduced overall uncertainty and variance. Key elements in the development of these new correlations are:(1) accumulation of a significant database of field performance case histories (2) use of improved knowledge and understanding of factors affecting interpretation of standard penetration test data.(3) determining whether the soil is susceptible to liquefaction or not, at whereas depths up to 35m below ground level. (4) use of reliability method and Bayesian approach to determine the probability of liquefaction.

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CHAPTER 1

INTRODUCTION

1.1 General

Liquefaction is the phenomena when there is loss of strength in saturated and cohesion-less soils because of increased pore water pressures and hence reduced effective stresses due to dynamic loading. It is a phenomenon in which the strength and stiffness of a soil is reduced by earthquake shaking or other rapid loading.

Liquefaction occurs in saturated soils and saturated soils are the soils in which the space between individual particles is completely filled with water. This water exerts a pressure on the soil particles that. The water pressure is however relatively low before the occurrence of earthquake. But earthquake shaking can cause the water pressure to increase to the point at which the soil particles can readily move with respect to one another.

Although earthquakes often triggers this increase in water pressure, but activities such as blasting can also cause an increase in water pressure. When liquefaction occurs, the strength of the soil decreases and the ability of a soil deposit to support the construction above it.

Soil liquefaction can also exert higher pressure on retaining walls, which can cause them to slide or tilt. This movement can cause destruction of structures on the ground surface and settlement of the retained soil. Accordingly, the use of in situ "index" testing is the dominant approach in common practice. As summarized in the recent state of-the-art paper (Youd et al. 2001), four in situ test methods have now reached a level of sufficient maturity as to represent viable tools for this purpose, and these are: (1) the standard penetration test (SPT); (2) the cone penetration test; (3) measurement of in situ shear wave velocity Vs; and (4) the Becker penetration test. The oldest, and still the most widely used of these, is the SPT, and this will be the focus of this study.

1.2 OBJECTIVE OF THE PRESENT STUDY

The objectives of the research are as follows:

a) To determine the liquefaction potential by deterministic approach.

b) To determine the liquefaction potential by probabilistic approach.

c) to compare the results of the above two approaches.

d) to apply necessary corrections to get more precise results.

1.3 METHODOLOGY OF THE STUDY

This includes the thorough study of two methods to determine probability of liquefaction.

(i) *Deterministic Approach* : The most common procedure used in engineering practice for the assessment of liquefaction potential of sands and silts is the Simplified Procedure. The procedure may be used with either SPT blow count, CPT tip resistance or shear wave velocity. In this report SPT data is obtained of 10 bore holes and detailed calculation is performed using *Sixth Revision of IS 1893 (Part 1) February 2016.* The results are obtained whether the soil is liquefiable or not using factor of safety.

(ii) *Probabilistic Approach* : The above simplified methods for assessing soil liquefaction potential using a deterministic safety factor in order to judge whether liquefaction will occur or not. However, these methods are unable to determine the liquefaction probability related to a safety factor. An answer to this problem can be found by reliability analysis. This study presents a reliability analysis method based on the popular Seed'85 liquefaction analysis method. This

reliability method uses the empirical acceleration attenuation law to derive the probability density distribution function (PDF) and the statistics for the earthquake-induced cyclic shear stress ratio (CSR). The CSR and CRR statistics are used in conjunction with the first order and second moment method, to calculate the relation between the liquefaction probability, the safety factor and the reliability index. Based on the proposed method, the liquefaction probability related to a safety factor can be easily calculated.

1.4 ORGANIZATION OF THE STUDY

The study is organized according to the stages followed for the determination of the liquefaction potential by deterministic and probabilistic approach. Thus,

Chapter 1 introduces a general statement of the liquefaction, objective and methodology of this research.

Chapter 2 reviews the available literature that is required to understand the background theories of various aspects of liquefaction. This chapter also includes a literature survey on the different techniques used for determining the liquefaction potential like deterministic method using empirical relationships and probabilistic method using reliability function, normal distribution, probability function, etc.

Chapter 3 describes soil liquefaction and liquefaction hazard assessment

Chapter 4 describes the methodology of the project. Which include detailed analysis of deterministic approach followed by deterministic sample calculations and detailed probabilistic approach followed by probabilistic sample calculations.

Chapter 5 draws conclusions and results of the current work. Most of the results are in tabular form and are shown in appendix A and B.

CHAPTER 2

LITERATURE REVIEW

Armen Der Kiureghian; K. Onder Cetin; Raymond B. Seed; Kohji Tokimatsu4 ; Leslie F. Harder Jr; Robert E. Kayen; and Robert E. S. Moss [1] has presented the development of recommended new probabilistic and deterministic relationships for assessment of likelihood of initiation of liquefaction. Stochastic models for assessment of seismic soil liquefaction initiation risk have been developed within a Bayesian framework. In the course of developing the proposed stochastic models, the relevant uncertainties including (1) measurement/estimation errors, (2) model imperfection, (3) statistical uncertainty, and (4) those arising from inherent variables were addressed. Improved treatment of r_d in "simplified" assessment of in situ CSR results in triggering relationships that are unbiased with respect to use in conjunction with either (1) direct seismic response analyses for evaluation of in situ CSR, or (2) improved "simplified" assessment of in situ CSR. This is an important step forward, as these studies also show that all previous, widely used correlations are unconservatively biased when used in conjunction with direct response analyses for assessment of CSR, as a result of bias in previous "simplified" r_d recommendations. The new models provide a significantly improved basis for engineering assessment of the likelihood of liquefaction initiation, relative to previously available models. The new models presented and described in this paper deal explicitly with the issues of: (1) FC, (2) magnitude-correlated, and (3) effective overburden stress (K_{σ} effects), and they provide both: (1) an unbiased basis for evaluation of liquefaction initiation hazard and (2) significantly reduced overall model uncertainty. Indeed, model uncertainty will be reduced sufficiently that overall uncertainty in application of these new correlations to field problems is now driven strongly by the difficulties/uncertainties associated with project-specific engineering assessment of the necessary "loading" and "resistance" variables, rather than uncertainty associated with the correlations themselves. This represents a significant overall improvement in our ability to accurately and reliably assess liquefaction hazard.

J.H. Hwang*, C.W. Yang, D.S. Juang [2] presents a practical reliability-based method for liquefaction analysis. The proposed method is simple and clear. Based on the popular Seed'85 method, we use the empirical acceleration attenuation law to derive the probability density distribution function (PDF) and the statistics for the earthquake-induced cyclic shear stress ratio (CSR). He also collected liquefaction and non-liquefaction data from Chi-Chi earthquake and others around the world. The logistic model modified from Liao et al. is then used to derive the PDF and the statistics for cyclic resistance ratio (CRR). With these statistics, the first order and second moment method can be used to calculate the relation between the liquefaction probability with the safety factors and the reliability index. The whole proposed computation procedure is summarized in a flow chart, to facilitate its use by engineers. Finally, an analysis assessing the liquefaction potential at a real construction site is presented, to demonstrate the use and the feasibility of the method.. The probability may seem a little high at first glance, but it should be noted that the derived liquefaction probability does not include the probability that an earthquake of the given magnitude will occur. It only gives the liquefaction probability for one soil layer and for the given earthquake event. The real liquefaction probability would be the probability that liquefaction would occur during an earthquake considered jointly with the probability that an earthquake of such a magnitude will occur. Based on seismic hazard analysis, the probability that the specified earthquake will occur is about 0.002 annually, or, in other words, this size earthquake has a return period of 475 years. Thus, a comprehensive probabilistic liquefaction analysis method is considered the uncertainties in the CSR and the CRR, as well as the probability that an earthquake will occur. Based on the proposed method, the liquefaction probability related to a safety factor can be easily calculated. The influence of some of the soil parameters on the liquefaction probability can be quantitatively evaluated.

K. Onder Cetina, Armen Der Kiureghian, Raymond B. Seed [3] A Bayesian framework for probabilistic assessment of the initiation of seismic soil liquefaction is described. A database, consisting of post-earthquake field observations of soil performance, in conjunction with in situ ''index'' test results is used for the development of probabilistically-based seismic soil liquefaction initiation correlations. The proposed stochastic model allows full and consistent representation of all relevant uncertainties. including (a) measurement/estimation errors, (b) model imperfection, (c) statistical uncertainty, and (d) inherent variability. Different sets of probabilistic liquefaction boundary curves are developed for the seismic soil liquefaction initiation hazard problem, representing various sources of uncertainty that are intrinsic to the problem. The resulting correlations represented a significant improvement over prior efforts, producing predictive relationships with enhanced accuracy and greatly reduced overall model uncertainty.

K. Onder CETIN , Raymond B SEED And Armen DER KIUREGHIAN [4] presents the results to date of ongoing studies to develop improved, probabilisticallybased correlations for the use of SPT data for evaluation of resistance to "triggering" or initiation of cyclic liquefaction. Although these studies are ongoing, the relationships developed at this stage are considered to represent a sufficient advance over previously available, similar relationships as to merit their exposition at this time. The relationships presented herein have a number of significant advantages over previous probabilistic and "deterministic" relationships currently available. These include:

• Previously available field case history data have been re-evaluated, taking advantage of recent developments/insights regarding (a) factors affecting "correction" of SPT data for energy, equipment, procedure, and rod-length effects, and (b) factors affecting evaluation of in-situ equivalent uniform cyclic stress ratio including source mechanism effects, local site effects, etc.

• A large number of "new" field case history data were collected and similarly evaluated.

• With this greatly enhanced database, higher standards were set for acceptability of case history data, and data not meeting these standards were deleted. The result is an enlarged database of high quality.

• The Bayesian parameter estimation method was used to develop and evaluate correlations. This method allowed for separate treatment of different sources of aleatory and epistemic uncertainty, and allowed assessment of more contributing variables/parameters than prior studies.

The resulting correlations provide a significantly improved basis for evaluation of liquefaction resistance, and also resolve a number of previously difficult issues including (a) "corrections" for fines content and effective overburden stress, and (b) magnitude-correlated duration weighting factors (for magnitudes other than $MW = 7.5$)

Neelima Satyam D and K. S. Rao [5] state that determination of liquefaction potential due to an earthquake is a complex geotechnical engineering problem. Many factors, including soil parameters and seismic characteristics influence this phenomenon. To assess the liquefaction hazard in an area, it is important to examine initially the liquefaction susceptibility. Before proceeding to the rigorous investigation of the liquefaction potential for Delhi region, first qualitative assessment of liquefiable soils is carried out based on the geotechnical characteristics. The percentage of silt is high in the north and eastern side of Delhi indicating that there is a great chance of soil being subjected to liquefaction. Also, the value of plasticity index in these locations is also less compared to the soils in the western side of the area indicating the probability of liquefaction to occur. According to the geological criteria also, the Holocene soils present in the trans Yamuna region are highly susceptible than the Pleistocene soils in the western side of Delhi region and there is very less chance of liquefaction to occur in the central part of Delhi because of the rock outcrop and gravelly sands. This liquefaction hazard map which is developed in this paper will help in selecting a suitable ground improvement technique and a foundation system by the engineers for future constructions in the Delhi region.

Hidenori MOGI and Hideji KAWAKAMI [6] In this study, spatial variation of peak ground accelerations (PGAs) is examined using strong motion records for a large number of events from dense accelerometer arrays at Chiba in Japan, SMART–1 in Lotung, Taiwan, and a realtime city gas network damage estimation system (SIGNAL) in Japan. We defined PGA ratios as spatial intra-event variations of PGAs and examined their probability density functions (PDFs), mean values, standard deviations and percentiles estimated using accelerometer arrays of the Chiba, SMART-1, and SIGNAL databases. Then, the relationship between these statistics and the station separation distances was analyzed. We found that there is a very large scatter of PGAs in the results. It was also revealed that the means and standard deviations have an almost linear relationship with the logarithm of the station separation distances ranging from several meters to one hundred kilometers, and this relationship can be attributed to the dependence of the correlation between intra-event PGAs on the separation distances.

Dr. Sudhir K Jain [7] Explanatory Examples are given on Indian Seismic Code IS 1893 (Part I). He measured SPT resistance and results of sieve analysis for a site in Zone IV. He determine the extent to which liquefaction is expected for 7.5 magnitude earthquake. Estimated the liquefaction potential and resulting settlement expected at Delhi.

Raghvendra Singh, Debasis Roy and Sudhir K. Jain [8] damaging effects of Bhuj Earthquake on embankment dams have been considered in this paper with particular reference to Chang Dam, Fatehgadh Dam and Kaswati Dam. Liquefaction to various extents of the foundation soils underneath these embankment dams during Bhuj Earthquake have been reported as one of the major causes of the distress within these dams. The data presented in this paper indicate that liquefaction within the shallow foundation soils would have been widespread underneath Chang Dam, while that underneath Fatehgadh Dam and Kaswati Dam were relatively localized. This assessment is in qualitative agreement with the facts that the damage to Chang Dam was near total, while those inflicted on the other two dams were relatively less pronounced. The sliding block method was then used to estimate the magnitude of observed deformations.

Shashank Burman and A. Murali Krishna [9] this study gives a better understanding of the liquefaction potential evaluation. This paper discussed the evaluation of seismically induced Liquefaction based on semi empirical field-based procedures using the Standard Penetration Test (SPT) profile of soil. This work has been carried out to initiate such studies in the region and to address the problem of seismically induced liquefaction based on deterministic as well as probabilistic approach. A case study for IIT Guwahati with 3 different borehole locations has been carried out for 3 different Earthquake Moment Magnitude of M=5, 6 and 7 respectively.. The results obtained in probabilistic approach are in good agreement with deterministic approach. This Study provides a good understanding to account for the liquefaction assessment based on two approaches.

IS 1893 (Part 1) Sixth Revision February 2016 the entire deterministic approach is performed using this code.

$CHAPTER = 3$

SOIL LIQUEFACTION AND LIQUEFACTION HAZARD ASSESSMENT

3.1 SOIL LIQUEFACTION : Liquefaction is the phenomena in which the strength and stiffness of a soil is reduced by earthquake shaking or other rapid loading. Loss of strength in saturated and cohesion-less soils occurs under undrained conditions, during dynamic loading, because of increased pore water pressures and hence significant reduction in effective stresses. Soil liquefaction induced by an earthquake can be a major cause of damage to structures and facilities. Damage to structure in liquefied sediments can occur as a result of bearing capacity failure of a foundation, lateral spreading or slope failure and differential settlement.

Sediments below water table temporarily lose shear strength and behave more as a viscous liquid than as a solid. The water in the soil voids exerts pressure upon the soil particles. If the pressure is low enough, the soil stays stable. However, once the water pressure exceeds a certain level, it forces the soil particles to move relative to each other, thus causing the strength of the soil to decrease and failure of the soil. The shear resistance of cohesion less soil is mainly proportional to the intergranular pressure and the co-efficient of friction between solid particles. The liquefaction phenomena that results from this process can be divided into two main groups e.g. flow liquefaction and cyclic mobility (Youd, T. L. and Perkins, 1978). Flow liquefaction can occur when the static shear stress is greater than the shear strength of the soil in its liquefied state. The deformations produced by flow liquefaction are induced by static shear stress. It occurs less frequently than cyclic mobility but can cause more severe damage. Cyclic mobility occurs when the static shear stress is less than the shear strength of the liquefied soil. It can occur in broad range of soils and site conditions. Various laboratory and field methods have been developed to assess soil resistance to liquefaction. The information regarding geomorphology, soil properties and its origin, water table depth, past seismic history is very essential in the liquefaction assessment of an area. Generally, the liquefaction process is associated with recent Holocene deposits and uncompacted fills. However, there have been a few observed cases of liquefaction of Pleistocene and even pre-Pleistocene deposits.

3.2 LIQUEFACTION HAZARD ASSESSMENT

The first step in liquefaction hazard assessment is the evaluation of liquefaction susceptibility. Liquefaction susceptibility was first coined by Youd and Perkins (1978) as a measure of inherent resistance of soil to liquefaction, and can range from not susceptible, to highly susceptible. Susceptibility can be estimated by comparing the properties of a given deposit to other soil deposits where liquefaction has been observed in the past. The primary relevant soil properties include grain size, fines content (i.e., amount of silt and /or clay), density, degree of saturation, and age of the deposit. In order to assess the preliminary liquefaction potential assessment of soil deposits over a large area in a seismically active region. Liquefaction susceptibility maps are the most basic level of liquefaction hazard mapping.

There are three different ways to predict liquefaction susceptibility of a soil deposit in a particular region i.e., Historical criteria, Geological/ Geomorphologic criteria and Compositional criteria. According to the historical criteria soils that have liquefied in the past can liquefy in future also. With the help of past earthquake records the liquefaction in future can be predicted. The liquefaction susceptibility depends strongly on the type of the geological process that created the soil deposits. River deposits, deposits formed by lakes, winds etc are highly susceptible to the process of liquefaction. Liquefaction potential also depend on the type of the soil. That is, uniform graded soils are highly susceptible than well-graded soil deposits; soils with angular particles are less susceptible than soils with rounded particles. Liquefaction potential is represented as the ratio induced to stress ratio causing liquefaction. It also refers to the probability that soil will actually liquefy at a given site, and therefore depends not only on the liquefaction susceptibility of soil, but also the level of seismic activity in the region. For example, very loose clean sand may be highly susceptible to liquefaction, however if it exists in a region of negligible seismicity, then its liquefaction potential will be low. In contrast, a denser soil may have a lower susceptibility however, a higher liquefaction potential because it is situated in an area of very strong seismic activity.

$CHAPTER = 4$

METHODOLOGY

4.1 DETERMINISTIC APPROACH

Evaluations of soil liquefaction potential by empirical methods and semi-empirical methods have become popular among practicing engineers. These methods use deterministic relations implying the occurrence or triggering of liquefaction. A semi-empirical method originally developed by Seed and Idriss [10,11] was based on the evaluation of soil liquefaction resistance based on the results of the standard penetration test (SPT) values. Over the past decades, these methods have been modified successively and have become more attractive and a standard of practice for many engineers around the world. Recently Idriss and Boulanger [12] have proposed an updated and improved method for the evaluation of liquefaction potential of soil deposits, which has been used in the present analysis. Although Deterministic analysis is well accepted method to evaluate the soil liquefaction, there are uncertainties left with traditional deterministic approach where factors of safety are often difficult to interpret. To overcome these uncertainties, probabilistic and statistical approaches have been employed in the literature.

Steps to calculate factor of safety of liquefaction

Step 1: The subsurface data used to assess liquefaction susceptibility should include the location of the water table, either SPT blow count N, mean grain size D50 , unit weight, and fines content of the soil (percent by weight passing the IS Standard Sieve No. 75μ).

Step 2: Evaluate total vertical stress σ_{vo} and effective vertical stress σ'_{vo} for all potentially liquefiable layers within the deposit.

Step 3: Evaluate Stress Reduction Factor r_d using:

 $r_d = \}$ $\mathbf{1}$ $1.174 - 0$ where z is the depth below the ground surface in metre.

Step 4: In the deterministic approach, Cyclic Stress Ratio(CSR) induced by earthquake ground motions, at a given depth z below the ground surface is usually calculated using Seed and Idriss relation. Calculation Critical Stress Ratio *CSR*, the resistance of a soil layer against liquefaction, induced by the design earthquake using:

$$
CSR=0.65(\frac{a\ max}{g})(\frac{\sigma v \sigma}{\sigma' v \sigma})r_{d}
$$

Where

 $\sigma v \sigma$ = Vertical overburden stress at depth z,

 $\sigma' \nu o =$ Effective vertical overburden stress at depth z,

 a_{max} = Peak ground acceleration,

 $g =$ Acceleration due to gravity, and

 r_d = Stress Reduction Factor.

Step 5: Obtain Critical Resistance Ratio CRR by correcting standard Critical Resistance Ratio *CRR7.5* for earthquake magnitude, stress level and initial static shear using:

$$
CRR = CRR_{7.5} \text{ (MSF) } k_{\sigma} k_{\alpha}
$$

Where :

 $CRR_{7.5}$ = Standard Critical Resistance Ratio for a 7.5 magnitude earthquake obtained using values of SPT (as per Step 6),

MSF = Magnitude Scaling Factor given by following equation:

$$
MSF = 10^{2.24} / M_w^{2.56}
$$

This factor is required when the magnitude is different than 7.5.

 k_{σ} = Correction for high overburden pressure is required when overburden pressure is high(depth > 15 m) and can be found using equation:

$$
k_{\sigma} = (\sigma' \nu o / Pa)^{-(f-1)}
$$

*P*a = atmospheric pressure, and

 σ' vo = effective overburden pressure, measured in same units

 k_{α} = Correction for static shear stresses is required only for sloping ground and is not required in routine engineering practice. Therefore, in the sope of this standard, value of k_{α} shall be assumed unity. Hence

 $k_{\alpha} = 1$

 f is an exponent and its value depends on the relative density D_r

For
$$
D_r = 40\%
$$
 - 60%, $f = 0.8 - 0.7$, and

$$
D_{\rm r}\!=60\%\,\text{-}\,80\%,\,\,f=0.7-0.6
$$

Step 6: Obtain Critical Resistance Ratio CRR7.5

Using values of SPT: Evaluate the standardized SPT blow count N_{60} , which is the Standard Penetration Test blow count for a hammer with an efficiency of 60 percent. Specifications are given in Table F1 of the standardized equipment corresponding to an efficiency of 60 percent. If equipment used is of non-standard type, N60 shall be obtained using:

$$
N_{60}=NC_{60}\,,
$$

where $C60 = C_{HT}C_{HW}C_{SS}C_{RL}C_{BD}$.

Table F2 Correction Factors for Non-Standard SPT Procedures and Equipment

Factors C_{HT} , C_{HW} , C_{SS} , C_{RL} and C_{BD} , recommended by various investigators for some common non-standard SPT configurations are provided in Table F2. For SPT conducted as per IS 2131- 1981, the energy delivered to the drill rod is 60percent and hence C60 may be assumed as 1.

Calculate the normalized standardized SPT blow count $(N_1)_{60}$ is normalized to an effective overburden pressure of approximately 100kPa using Stress resistance ratio C_N using:

$$
(N_1)_{60} = C_N
$$
. N₆₀ where

$$
C_N = \sqrt{\tfrac{Pa}{\sigma' \nu o}} \le 1.7,
$$

The effect of fines content FC (in %) can be rationally accounted for by correcting $(N_1)_{60}$ and finding $(N_1)_{60CS}$ as follows:

$$
(N_1)_{60CS} = \alpha + \beta (N_1)_{60}
$$

Where

$$
\alpha = 0 \qquad \qquad \beta = 1 \qquad \qquad \text{for } FC \leq 5\%
$$

$$
\alpha = e^{[1.76 - \left(\frac{190}{FC^2}\right)]} \qquad \beta = 0.99 + \frac{(FC)^{1.5}}{1000} \qquad \text{for } 5\% < FC < 35\%
$$

$$
\alpha = 5 \qquad \qquad \beta = 1.2 \qquad \qquad \text{for } FC \geq 35\%
$$

Figure below can be used to estimate $CRR_{7.5}$, where $(N_1)_{60CS}$ shall be used instead of $(N_1)_{60}$ and only SPT clean sand based curve shall be used irrespective of fines contents.

 RELATION BETWEEN CRR AND (N1)⁶⁰ FOR SAND FOR Mw 7.5 EARTHQUAKES

The *CRR7.5* can be estimated using following equation, instead of figure.

$$
CRR_{7.5} = \frac{1}{34 - (N1)_{60CS}} + \frac{(N1)_{60CS}}{135} + \frac{50}{[10X(N1)_{60CS} + 45]^2} - \frac{1}{200}
$$

Step 7: Calculation of the Factor of Safety FS against initial liquefaction. Determination of earthquake induced loading expressed in terms of cyclic stress ratio (CSR), this loading is compared with the liquefaction resistance of the soil, which is expressed in terms of cyclic resistance ratio (CRR). The Factor of safety (FOS) is evaluated using CSR and CRR :

$$
FS = \frac{CRR}{CSR}
$$

where CSR is as estimated in Step 5 and CRR in Step 6. When the design ground motion is conservative, earthquake-related permanent ground deformation is generally small, if $1.2 \geq FS$

4.2 Deterministic calculations

Determination of the extent to which liquefaction is expected for 7.5 magnitude earthquake

The measured SPT resistance and results of sieve analysis for a site in New delhi (Zone IV) are indicated in Table below.

Site Characterization:

- 1. This site consists of loose to dense poorly graded sand to inorganic clay. The SPT values ranges from 6 at 1.5m depth to 197 at a depth of 35m.
- 2. The site is located in New Delhi (zone IV).
- 3. The peak horizontal ground acceleration value for the site will be taken as 0.24g corresponding to zone factor $Z = 0.24$
- 4. Actual water table depth $= 6.10$ m
- 5. Water table assumed for calculation $= 0.0$ m
- 6. Borehole diameter $= 150$ mm

Liquefaction Potential of Underlying Soil

Step by step calculation for the depth of 3m is given below. Detailed calculations for all the depths are given in Table 10.2. This table provides the factor of safety against liquefaction (FS) and maximum depth of liquefaction below the ground surface.

Step 1:

- 1. $\frac{a \max}{g} = 0.24$
- 2. $M_w = 7.5$
- 3. $\gamma_{\text{sat}} = 1.92 \text{ t/m}^3$
- 4. $\gamma_w = 1 \text{ t/m}^3$
- 5. Depth at which liquefaction potential is to be evaluated $= 3$ m

Step 2: Initial stresses:

$$
\sigma_{\rm vo} = (1.92 \times 1.5) + (1.92 \times 1.5) = 5.76 \text{ t/m}^2
$$

 $u_0 = 3 x 1 = 3 t/m^2$

 $\sigma_{\text{vo}}^{\prime} = (\sigma_{\text{vo}} - \mu_{\text{o}}) = 5.76 - 3 = 2.76 \text{ t/m}^2$

Step 3: Stress reduction factor:

 $r_d = \}$ $\mathbf{1}$ $1.174 - 0$

 $r_d = 1 - 0.00765x3 = 0.97705$

Step 4: Calculation Critical Stress Ratio *CSR*

$$
CSR = 0.65(\frac{a \max}{g})(\frac{\sigma v \sigma}{\sigma' v \sigma})r_{d}
$$

$$
CSR = 0.65 \times (0.24) \times (\frac{5.76}{2.76}) \times 0.97705
$$

CSR = 0.31809

Step 5: Obtain Critical Resistance Ratio CRR7.5

1. $C60 = C_{HT}C_{HW}C_{SS}C_{RL}C_{BD}$

 $[C_{HT} = 1, C_{HW} = 1, C_{SS} = 1]$ (assumed)

 $C_{RL} = 0.8$ (from Table F2)

 C_{BD} = 1.05 (from Table F2) (Borehole diameter = 150mm)

 $C60 = 1x1x1x0.8x1.05 = 0.84$

2.
$$
N_{60} = NC_{60}
$$

\n
$$
= 5 \times 0.84 = 4.2
$$
\n3. $C_N = \sqrt{\frac{P_a}{\sigma' \nu_0}} \le 1.7$
\n
$$
= \sqrt{\frac{10.33}{2.76}} = 1.934 \text{ (> } 1.7 \text{ therefore}
$$

\n
$$
C_N = 1.7
$$

4. Calculate the normalized standardized SPT blow count

 $(N_1)_{60} = C_N$. N₆₀

$$
= 1.7 \times 4.2 = 7.14
$$

5. The effect of fines content FC (in %) can be rationally accounted for by correcting $(N_1)_{60}$ and finding $(N_1)_{60CS}$

 $FC = 49\%$ at depth of 3 m

$$
\alpha = 5 \qquad \beta = 1.2 \qquad \text{for } FC \ge 35\%
$$
\n
$$
(N_1)_{60CS} = \alpha + \beta(N_1)_{60}
$$
\n
$$
= 5 + 1.2 \times 7.14 = 13.568
$$
\n
$$
CRR_{7.5} = \frac{1}{34 - (N_1)60CS} + \frac{(N_1)60CS}{135} + \frac{50}{[10X(N_1)60CS + 45]^2} - \frac{1}{200}
$$
\n
$$
= \frac{1}{34 - 13.568} + \frac{13.568}{135} + \frac{50}{[10X13.568 + 45]^2} - \frac{1}{200}
$$

CRR7.5 ⁼ 0.14598

Step 6 : Obtain Critical Resistance Ratio CRR by correcting standard Critical Resistance Ratio *CRR7.5* for earthquake magnitude, stress level and initial static shear using:

- 1 k_{σ} = 1 Correction for high overburden pressure is required when overburden pressure is high(depth > 15 m)
- 2 $k_{\alpha} = 1$ Assumed (not required in routine engineering practice)
- 3 MSF = 1 (This factor is required when the magnitude is different than 7.5)

 $CRR = CRR_{7.5}$ *(MSF)* k_a k_a

$$
CRR = 0.14598 \times 1 \times 1 \times 1 = 0.14598
$$

Step 7: Calculate the Factor of Safety FS against initial liquefaction using:

$$
FS = \frac{CRR}{CSR} = \frac{0.14598}{0.31809} = 0.45892
$$
 (< 1 Hence liquid the line)

Similarly, calculation is being performed for each layer of soil in MS excel. And results shown in table below.

4.3 PROBABILISTIC APPROACH

Civil engineers usually use a factor of safety (FS) to evaluate the safety of a structure. The safety factor is defined as the strength of a member divided by the load applied to it. Most design codes require that a member's calculated safety factor should be greater than a specified safety factor, a value at least larger than one, to ensure the safety of the designed structure. The specified safety factor is largely determined by experience, there has been no rational way to determine such a factor up to now. Because the safety factor-based design method does not consider the variability of the member strength or the applied loading, the probability that the structure will fail cannot be known. Engineering design methods based on reliability analysis have been born against this background.

The reliability method requires a detailed investigation of the member strength and the applied loading data, from which statistical indices, such as the mean value and the coefficient of variation, can be derived. Then, using the first order and second moment method, the relationships between the failure probability, the reliability index and the safety factor can be deduced. As science and technology progress, more data about member strength and loading are collected, making engineering reliability analysis more feasible. These developments have led to the gradual evolution of design codes in various countries, from safety factor-based methods to reliability-based ones. There has been some research on reliability analysis in liquefaction areas. They used the same linear first order and second moment method to assess the variability of the major parameters that influence soil liquefaction and to set up probability models for liquefaction evaluation. However, these models have adopted the early simplified methods for liquefaction evaluation; the soil parameters they used are rarely used now. Moreover, the rationality of the reliability analysis results largely depends on the amount and quality of the collected data used to deduce the statistics of earthquake-induced cyclic stress and cyclic soil strength. Liao et al.[13] collected data for 289 liquefaction and non liquefaction cases around the world, then employed the logistic regression model to establish probabilistic cyclic strength curves. Since that effort, this methodology has attracted much attention. Similar probabilistic cyclic strength curves, based on the SPT-N, CPT-qc, and Vs parameters have been proposed [14-16]. These models only consider the variability of the soil cyclic strength, but do not take into consideration the variability of the earthquake induced cyclic shear stress.

Juang et al. [17,18] proposed a limit state curve, which separates the states of liquefaction and non-liquefaction by using an artificial neural network. They developed a reliability-based method for assessing the liquefaction potential by introducing the Bayes' mapping theorem. Their work also contains a useful discussion on the relation of the safety factor and the liquefaction probability, which has led to a notable advancement in the state of the art for liquefaction evaluation. Nevertheless, the use of an artificial neural network, with its hidden variables, does not have a clear physical meaning, which may explain why practicing engineers have not much familiarized themselves with this technique. In this study, a practical reliability-based method is developed for assessing the soil liquefaction potential. The proposed approach, based on conventional probability theory, enables the earthquake-induced cyclic stress ratio (CSR) and the soil cyclic resistance ratio (CRR) statistics to be clearly derived.

On the basis of the simplified SPT-N method proposed by Seed et al. [19], the probability density function (PDF) can be deduced for the earthquake-induced cyclic stress ratio, by means of the empirical peak ground acceleration attenuation law and its statistics. We used a revised version of the logistic model proposed by Liao et al. [13] to regress the probabilistic cyclic strength curves with cases of liquefaction and non-liquefaction. The PDF of the soil cyclic resistance ratio is then derived from these curves. With the CSR and CRR statistics, it becomes very simple to calculate the relationship between the liquefaction probability, the reliability index and the safety factor by way of the first order and second moment method.

The first step in engineering reliability analysis is to define the performance function of a structure. If the performance function values of some parts of the whole structure exceed a specified value under a given load, it is thought that the structure will fail to satisfy the required function. This specified value (state) is called the limit state of the performance function of the structure. In the simplified liquefaction potential assessment methods, if the CSR is denoted as S; and the CRR is denoted as R; we can define the performance function for liquefaction as $Z = R - S$. If $Z = R - S < 0$; the performance state is designated as 'failed', i.e. liquefaction occurs. If $Z = R - S > 0$; the performance state is designated as 'safe', i.e. no liquefaction occurs. If $Z = R - S = 0$; the performance state is designated as a 'limit state', i.e. on the boundary between liquefaction and non-liquefaction states. Since there are some inherent uncertainties involved in the estimation of the CSR and the CRR, we can treat R and S as random variables, hence the liquefaction performance function will also be a random variable. Therefore, the above three performance states can only be assessed as having some probability of occurrence.

The liquefaction probability is defined as the probability that $Z = R - S \le 0$. However, an exact calculation of this probability is not easy. In reality, it is difficult to accurately find the PDFs of random variables, such as R and S. Moreover, the calculation of the probability of $Z = R - S \le 0$ needs multiple integration over the R and S domains, which is a complicated and tedious process. A simplified calculation method, the first order and second moment method, has been developed to meet this need. The method uses the statistics of the basic independent random variables, such as R and S; to calculate the approximate statistics of the performance function variable, in this case $Z = R - S$; so as to bypass the complicated integration process. According to the principle of statistics, the performance function $Z = R - S$ is also a normally distributed random variable, if both R and S are independent random variables under normal distribution. If the probability density function (PDF) and the cumulative probability function (CPF) of Z are denoted as $f_z(z)$ and $F_z(z)$ respectively, the liquefaction probability P_f then equals the probability of $Z = R - S \le 0$. Hence

$$
P_f = P(z \le 0) = \int_{-\infty}^{0} f(z) dz = F_z(z)
$$
 (1)

If the mean values and standard deviations of R and S are μ_r , μ_s and σ_r , σ_s according to the first order and second moment method, the mean value μ_z the standard deviation σ_z and the coefficient of variation δ_z of Z; can be derived as follows :

$$
\mu_z = \mu_r - \mu_s \tag{2}
$$

$$
\sigma_{z} = \sqrt{\sigma_{R}^{2} + \sigma_{S}^{2}}
$$
 (3)

$$
\delta_z = \frac{\sigma_z}{\mu_z} = \frac{\sqrt{\sigma_R^2 + \sigma_S^2}}{\mu_R - \mu_S} \tag{4}
$$

The statistics for the performance function Z can be simply calculated by Eqs. (2) – (4) , using statistics for the basic variables R and S. This shows the advantage of the first order and second moment method. **The reliability index** β is defined as the inverse of the coefficient of variation δ_z and is used to measure the reliability of the liquefaction evaluation results. β is expressed as:

$$
\beta = \frac{1}{\delta z} = \frac{\mu_R - \mu_S}{\sqrt{\sigma_R^2 + \sigma_S^2}}
$$
 (5)

$$
\mu_z = \beta. \sigma_z \tag{6}
$$

In Fig. 1 the liquefaction probability is indicated by the shaded tail areas of the PDF $f_z(z)$ of the performance function Z, Since $\mu_z = \beta$. σ_z , the larger the β ; the greater the mean value μ_z , and the smaller the shaded area and the liquefaction probability P_f , This means that β has a unique relation with P_f and can be used as an index to measure the reliability of the liquefaction evaluation. Since the normal distribution is the most important and the simplest probability distribution, we first assume that R and S are independent variables with a normal distribution to demonstrate the process of the reliability analysis. Based on this assumption, the performance function $Z = R - S$ is also in a normal distribution of Z $\tilde{C}(\mu_z, \sigma_z^2)$ By placing the PDF of Z into Eq. (1), we obtain the following liquefaction probability P_f :

$$
P_{f} = \int_{-\infty}^{0} fz(z) dz = \int_{-\infty}^{0} \frac{1}{\sqrt{2\pi} \sigma_{z}} e^{-1/2(z - \mu_{z}/\sigma_{z})^{2}}
$$

The above equation can be rewritten as

$$
P_f = \frac{1}{\sqrt{2H}} \int_{-\infty}^{-\mu_Z/\sigma_Z} e^{-t^2/2} dz = \Phi(-\frac{\mu_Z}{\sigma_Z})
$$

Where, $t = (\frac{z - \mu_Z}{\sigma_Z})$

where Φ is the cumulative probability function for a standard normal distribution. Since

$$
\beta = \mu_z/\sigma_z \text{ then }
$$

$$
P_f = \Phi(-\beta) = 1 - \Phi(\beta)
$$

The probability distribution of the basic engineering variables are usually slightly skewed, so they cannot be reasonably modeled by a normal distribution function. It has been found that most of the basic variables in engineering areas can be described more accurately by a log-normal distribution model, such as that proposed by Rosenblueth and Estra [20]. In this research, we also found that the CRR and the CSR data are more close to log-normal distributions, therefore, assumed that R (CRR) and S (CSR) are log– normal distributions. Based on this assumption, the liquefaction performance function is defined as $Z = ln(R/S) = ln(R) - ln(S)$ since the state of $ln(R/S) = ln(1) = 0$ is equivalent to the state of $(R/S) = 1$ or $(R - S) = 0$, the limit state of liquefaction. Then, the reliability index β and the liquefaction probability P_f can be expressed as

$$
\beta = \frac{1}{\delta z} = \frac{\mu_R - \mu_S}{\sqrt{\sigma_R^2 + \sigma_S^2}} = \frac{\ln\left[\frac{\mu_R}{\mu_S} \left(\frac{\delta_S^2 + 1}{\delta_R^2 + 1}\right)^{1/2}\right]}{\left[\ln(\delta_S^2 + 1)(\delta_R^2 + 1)\right]^{1/2}}
$$

$$
P_f = \Phi(-\beta) = 1 - \Phi(\beta)
$$

According to the safety factor-based design method, the safety factor FS for liquefaction is defined as the ratio of the mean values of R and S. Hence

$$
FS = \frac{\mu_R}{\mu_S}
$$

4.4 PROBABILISTIC CALCULATIONS

Step by step calculation for the depth of 3m is given below. Detailed calculations for all the depths are given in Table 10.2. This table provides the Reliabilty function (β) and probability of liquefaction (P_f) for different depths.

$$
\frac{a \max g}{g} = 0.24
$$

$$
M_w = 7.5
$$

$$
\gamma_{\text{sat}} = 1.92 \text{ t/m}^3
$$

$$
\gamma_w = 1 \text{ t/m}^3
$$

Depth at which liquefaction potential is to be evaluated $= 3$ m

$$
CSR = 0.31809
$$

$$
CRR_{7.5} = 0.14598
$$

$$
CRR = 0.14598
$$

FS = $\frac{CRR}{CSR} = 0.4589$
Deterministic result = liquid
E

 $\mu_R = 0.31809$

 $\mu_s = 0.14598$

 $\sigma_R = 0.44101$

 σ _S = 0.08194

Coefficient of variation δ^R of R :

 $\delta_{\mathbf{R}} = \frac{\sigma}{\sigma}$ $\frac{\sigma_{\rm R}}{\mu_{\rm R}}$ = $\frac{0}{0}$ $\frac{0.44101}{0.31809} = 1.3864$

Coefficient of variation $δ$ **_S of S :**

$$
\delta_{\rm S} = \frac{\sigma_{\rm S}}{\mu_{\rm S}} = \frac{0.08194}{0.14598} = 0.5613
$$

The reliability index β :

$$
\beta = \frac{\ln \left[\frac{\mu_R}{\mu_S} \left(\frac{\delta_{S+1}^2}{\delta_R^2 + 1} \right)^{1/2} \right]}{\left[\ln \left(\delta_{S+1}^2 \right) \left(\delta_R^2 + 1 \right) \right]^{1/2}}
$$

on putting the values mentioned above, we get

$$
\beta = -1.23456
$$

mean value μ **z** of **Z**:

$$
\mu_z = \mu_r - \mu_s
$$

 $= 0.14598 - 0.31809 = -0.17212$

Standard deviation σ^z of Z :

$$
\sigma_z = \sqrt{\sigma_R^2 + {\sigma_S}^2}
$$

= $\sqrt{0.44101^2 + 0.08194^2} = 0.44856$

Probability function for a standard normal distribution:

 $\Phi(\beta)$ = this value can be obtained using MS Excel software directly, by applying Normal Distribution function.

 $Φ(β) = 0.00893$

Liquefaction probability P^f :

 $P_f = \Phi(-\beta) = 1 - \Phi(\beta)$ $= 1 - 0.00893 = 0.99107$ $P_f = 0.99107$

Hence, this layer has a high probability of liquefaction.

And the results of Probabilistic approach is similar to Deterministic approach.

CHAPTER 5

Results and Conclusions:

Determination of liquefaction potential due to an earthquake is a complex geotechnical engineering problem. Many factors, including soil parameters and seismic characteristics influence this phenomenon. Several methods developed for the assessment of the liquefaction potential. This study we used IS code method for deterministic approach and reliability method for probabilistic approach and have been discussed in detail. To assess the liquefaction hazard in an area, it is important to examine initially the liquefaction susceptibility. Before proceeding to the rigorous investigation of the liquefaction potential for Delhi region, first qualitative assessment of liquefiable soils is carried out based on the geotechnical characteristics. The percentage of silt is high in the north and eastern side of Delhi indicating that there is a great chance of soil being subjected to liquefaction.

Liquefaction potential over the study area was evaluated using the simplified procedure based on the available SPT profile of soil. The analysis involved two approaches, Deterministic and Probabilistic respectively.

SPT results of 10 bore hole were obtained, each at a interval of 200m. And using the soil characteristics like fine content, saturated unit weight, etc, each layer of soil of 1.5m depth is checked whether liquefiable or not. SPT test gave the no. of blow count (N) at each interval of 1.5m up to the depth of 35m below ground level. The analysis is performed on the software called MS EXCEL. The data is analyzed in tabular form. A number of corrections were applied to obtain the most precise results.

- Both the approaches used obtained almost same results as shown in Table 1.
- Table 2 to Table 11 shows the results of deterministic approach and
- Table 12 to Table 21 shows the results of probabilistic approach.
- Under probabilistic approach probability of liquefaction is obtained between 0 to 1. As we can see the values near to 1 are liquefiable and values near to 0 are non liquefiable.
- The results obtained shows that the soil is susceptible to liquefaction maximum up to the depth of 9m.
- The present analysis highlights the advantage of probabilistic analysis for liquefaction evaluation and also the need to consider various soil profiles in same area at different depths..

 TABLE 1: probability of liquefaction and factor of safety (comparative) results.

- \bullet Pink = liquefiable
- \bullet white = Non-liquefiable
- \bullet yellow = cannot be determined

- \bullet Pink = liquefiable
- \bullet white = Non-liquefiable
- \bullet yellow = cannot be determined

CONCLUSIONS

The comparative results shown in the above table 1 describe the almost same results for liquefaction.

- Factor of safety less than 1 describe the soil layer is liquefiable
- Probability of liquefaction close to 1 describe the soil is liquefiable
- Probability of liquefaction close to 0 describe the soil is non-liquefiable

Hence both the studies (deterministic and probabilistic)shows the similar results.

RESULTS

TABLE 2: Deterministic analysis of bore hole no.1

TABLE 4: Deterministic analysis of bore hole no.3

TABLE 5: Deterministic analysis of bore hole no.4

TABLE 6: Deterministic analysis of bore hole no.5

TABLE 7: Deterministic analysis of bore hole no.6

TABLE 8: Deterministic analysis of bore hole no.7

TABLE 9: Deterministic analysis of bore hole no.8

TABLE 10: Deterministic analysis of bore hole no.9

TABLE 11: Deterministic analysis of bore hole no.10

TABLE 12: Probabilistic calculation for bore hole no.1**.**

TABLE 13: Probabilistic calculation for bore hole no.2.

TABLE 14: Probabilistic calculation for bore hole no.3.

TABLE 15: Probabilistic calculation for bore hole no.4

.

TABLE 16: Probabilistic calculation for bore hole no.5

TABLE 17: Probabilistic calculation for bore hole no.6

TABLE 18: Probabilistic calculation for bore hole no.7

TABLE 19: Probabilistic calculation for bore hole no.8

TABLE 20: Probabilistic calculation for bore hole no.9

TABLE 21: Probabilistic calculation for bore hole no.10

References

- 1. Cetin KO, Seed RB, Moss RES, Der Kiureghian A, Tokimatsu K, Harder Jr. LF., Kayen RE. (2000). Field case K.O. Cetin et al. / Structural Safety 24 (2002) 67–82 81 histories for SPT-based in-situ liquefaction potential evaluation. Report No: UCB/GT-2000/09. University of California at Berkeley, August 2000.
- 2. J.H. Hwang*, C.W. Yang, D.S. Juang A practical reliability-based method for assessing soil liquefaction .Department of Civil Engineering, National Central University, Chung-li 32054, Taiwan, ROC,2004.
- 3. K. Onder Cetina, Armen Der Kiureghian, Raymond B. Seed. Probabilistic models for the initiation of seismic soil liquefaction a Department of Civil Engineering, Middle East Technical University, 06531, Ankara, Turkey bDepartment of Civil and Environmental Engineering, University of California, Berkeley, CA 94720, USA,2002.
- 4. K. Onder CETIN , Raymond B SEED And Armen Der Kiureghian, Probabilistic assessment of liquefaction initiation hazard, 12WCEE 2000.
- 5. Neelima Satyam D and K. S. Rao Assessment of Liquefaction Hazard using Shear Wave Velocities in Delhi, India. International Journal of Geotechnics and Environment (IJGE) 3(2) July-December 2011
- 6. Hidenori MOGI and Hideji KAWAKAMI Probability distribution of peak ground acceleration ratios, 13th World Conference on Earthquake Engineering Vancouver, B.C., Canada August 1-6, 2004 Paper No. 1345.
- 7. Dr. Sudhir K Jain, Explanatory Examples on Indian Seismic Code IS 1893 (Part I), Document No.:: IITK-GSDMA-EQ21-V2.0 Final Report :: A - Earthquake Codes IITK-GSDMA Project on Building Codes.
- 8. Raghvendra Singh, Debasis Roy and Sudhir K. Jain, Investigation of Liquefaction Failure in Earthen Dams during Bhuj Earthquake
- 9. Shashank Burman and A. Murali Krishna Soil Liquefaction Evaluation using Deterministic and Probabilistic Approaches: A case study, National Conference on Recent Advances in Civil Engineering (NCRACE-2013) During 15-16 November, 2013.
- 10. Seed, H. B., and Idriss I. M., (1971), Simplified procedure for evaluating soil liquefaction potential, Journal of Geotechnical Engineering, ASCE, Vol. 97, pp.1249-1273, 1971.
- 11. Seed H B and Idriss I M., (1982), Ground motions and soil liquefaction during earthquakes, Berkeley, CA: Earthquake Engineering Research Institute
- 12. Idriss IM., Boulanger RW., (2006), Semi-empirical procedures for evaluating liquefaction potential during earthquakes, Soil DynEarthqEng
- 13. Liao SSC, Veneziano D, Whitman RV. Regression models for evaluating liquefaction probability. J Geotech Eng ASCE 1988; 114(4):389–411
- 14. Youd TL, Noble SK. Liquefaction criteria based on statistical and probabilistic analyses. Proceedings of the NCEER Workshop on Evaluation of Liquefaction Resistance of Soils, Technical Report NCEER-97-0022; 1997. p. 201–16.
- 15. Toprak S, Holzer TL, Bennett MJ, Tinsley JC. CPT- and SPT-based probabilistic assessment of liquefaction. Proceedings of Seventh US–Japan Workshop on Earthquake Resistant Design of Lifeline Facilities and Counter-measures Against Liquefaction. Seattle; 1999: p. 69–86.
- 16. Andrus RD, Stokoe KH, Chung RM, Juang CH. Guidelines for evaluation liquefaction resistance using shear wave velocity measurements and simplified procedures. Gaithersburg, MD: National Institute of Standards and Technology; 2001
- 17. Juang CH, Chen CJ, Rosowsky DV, Tang WH. CPT-based liquefaction analysis Part 1: determination of limit state function. Geotechnique 2000;50(5):583–92. [13]
- 18. Juang CH, Chen CJ, Rosowsky DV, Tang WH. CPT-based liquefaction analysis Part 2: reliability for design. Geotechnique 2000;50(5):593–9
- 19. Seed HB, Tokimatsu K, Harder LF, Chung RM. The influence of SPT procedures in soil liquefaction resistance evaluation. J Geotech Eng ASCE 1985;111(12):1425–45.
- 20. Rosenblueth E, Estra L. Probabilistic design of reinforced concrete buildings. ACI Special Publication 31; 1972