CHAPTER 1

INTRODUCTION

The residential housing where the field test was performed is a one room set housing , planned for EWS class ,comprising of a total of 484 flats. The room size is 3.0mx3.6m with an attached kitchen and toilet area. The clear floor height of each housing is 2.9m.

1.1 Introduction of the project

The Residential building was constructed under low cost housing scheme within a limited period of time by Delhi State Industrial & Infrastructure Development Corporation at Bawana, New Delhi. To save time and to maintain the economy, the housing was designed and constructed using flexible shutterings. It is fast , simple and cost effective . It gives a good quality work which requires minimum maintenance and when durability is the prime consideration. The system is different from the normal beam column framing. In the system of flexible shuttering, the slab is cast monolithically with the RCC walls.

1.1.1 Background of Flexible Shuttering Method

This technology has been used extensively in other countries such as Europe, Gulf Countries, Asia and all other parts of the world. This is suitable for constructing large number of houses within short time using room size forms to construct walls and slabs in one continuous pour on concrete. Early removal of forms can be achieved by hot air curing curing compounds. This facilitates fast construction. All the activities are planned in assembly line manner and hence result into more accurate, well – controlled and high quality production at optimum cost and in shortest possible time.

In this system of formwork construction, cast – in – situ concrete wall and floor slabs cast monolithic and provides the structural system in one continuous pour. Large room sized forms for walls and floor slabs are erected at site. These forms are made strong and sturdy, fabricated with accuracy and easy to handle. They afford large number of repetitions (around 250 times).

The concrete is produced in RMC batching plants under strict quality control and convey it to site with transit mixers.

The frames for windows and doors as well as ducts for services are placed in the form before concreting. Staircase flights, façade panels, chajjas etc. and other pre-fabricated items are also integrated into the structure. This proves to be a major advantage as compared to other modern construction technique. The integrated use of this technology results in a durable structure.

1.1.2 Design Aspects of Flexible shuttering method

Earlier building design was based on two type of buildings –load bearing brick wall with RCC slab and beam column framed structure with RCC slab. The first one is suitable only for single storey construction but if more storeys are required then the thickness of the brick wall is to be increased which becomes highly economic. For multistoried structures,beam column framing system is suitable, but for higher levels the percentage of become very high resulting in congestion and if concrete is not properly poured to cover the steel it will result in weaker sections which may counter seismic failure. Now a days buildings can be of two types -

i)Conventional RCC columns, beams, and slab construction
(RCC moment resisting framed structure)
ii) RCC load-bearing walls and slabs.

In the case of RC moment-resisting framed structures, the horizontal forces due to wind or earthquake are resisted by the frames resulting in the bending moments in columns to resist bending moment and vertical loads would be more than that required to resist vertical loads without bending moment. Similarly, additional reinforcement will be required in beams at supports. Recent changes in the IS Codes, as well as recommended good practice demand provision of additional reinforcement comply with ductility requirements. The sizing and detailing of columns needed to be –that they are 20% stronger than beams they support.

In the case of RC load-bearing walls, monolithic casting of slab along with RC walls results in a box type structure, which is very strong in resisting horizontal forces due to wind or earthquake. In view of large depth of shear walls, the resulting stresses due to bending moment and vertical loads are smaller and in many cases, concrete alone is capable of resisting these force.

1.2 OBJECTIVE OF THE WORK

The building was constructed under low cost housing scheme with a common RCC wall between the two dwellings. The door opening was proposed in the common wall. The housing is a totally made up of reinforced cement concrete of M15 grade. As the M15 grade is now taken over by M20 grade, it had become necessary to check the strength of the building especially when the opening was created was created after full construction. To study the safety of the structure following objectives had been set.

1 .Load test on the existing building at Bawana
2 .Numerical Analysis using STAADPRO with and without opening of the common wall
3 .Comparision of observed deflections (field test) and predicted deflections.

CHAPTER 2

LITERATURE REVIEW

Deformation is the change in dimension or form under the action of applied forces. The process of deformation comprises of the following consecutive stages:

i) Elastic deformation
ii) Plastic deformation , and
iii) Fracture.

Elastic deformation is the deformation which disappears as soon as the load is removed. Plastic deformation is the permanent change in shape or size of the specimen even after removal of loads. The time dependent part of permanent deformation is called creep.

The tests on hardened concrete are carried out to assess the quality of concrete in the structure, the load carrying capacity and the strength of concrete at a particular location in the structure. The compressive strength test on cubes are required for statistical quality control and acceptance criteria as per code requirements. However, to assess the quality and strength of concrete in the structure following methods are generally used.

a) The concrete core test
b) The pull out test
c) The ultrasonic method
d) The Schmidt test hammer method.

All the test mentioned above are the tests for hardened concrete but they give information about the compressive strength of concrete. In a restrained slab, as the slab deflects under the application of load the edges of the slab try to move outwards. In the case of an isolated slab beam system the edge beams provide a lateral restraint to this outward movement and consequently certain in plane compressive forces are introduced across the slab cross section. The inplane forces in turn act on the edge beams and as a result the beams deflect laterally outwards. Thus the development of compressive membrane forces amongst other factors is

dependent on the flexural rigidities of the beams. A few studies have been reported on isolated slab–beam systems of rectangular panels and circular slabs with ring beams. Park [1964] presented an analysis for the prediction of ultimate loads of partially restrained slabs assuming the restraint at the edges to be of constant stiffness. Hayes and Taylor [1969] reported the results of tests of ten reinforced concrete slab beam panels. The variables included the relative strength of beams and slabs, the layout of the slab, reinforcement and details of their connections. They reported that the concentration of reinforcement in the central region has a desirable effect upon the behaviour and would affect the economy of reinforcement. They pointed out that a good corner connection controls the propagation of cracking in the slab and can increase the load carrying capacity considerably. Girolami et al. [1970] reported the tests on specimens subjected to both transverse and inplane loading. A number of point loads were applied over the panel surface to simulate uniform vertical loading. Equal horizontal loads were applied over the panel surface to simulate uniform membrane forces. The membrane loads were held constant during the test. Their tests indicated that the load capacity of a panel can be estimated accurately if the membrane forces acting on the panel are known. Hopkins and Park [1971] tested a one fourth scale concrete slab–beam floor system. Their study showed that lack of information concerning the long term behaviour of the slab may limit the applicability of compressive membrane action in design. Datta and Ramesh [1973,1975] have proposed a method to predict the ultimate load of slab–beam panels and checked the validity of the method with their tests on nineteen square isotropic slab–beam panels. In the analysis they assumed a suitable value of deflection at ultimate to calculate the ultimate load. They assumed a value of deflection equal to 0.8 times the slab thickness for specimens with low edge beam stiffness and the experimental deflection for the slab–beams with higherdegree of value of stiffness. Desayi and Kulkarni [1979] reported a method to predict the complete load deflection behavior force on load carrying capacity is more for slabs having a lower percentage of main reinforcement and low aspect ratio.

Although many investigations were reported on two way slabs, the studies on slab strips are quite numerous. Roberts [1969] reported the theoretical evaluation of ultimate load based on rigid plastic approach. He found that in many cases the experimental maximum load was greater than Johansen's yield line load. He reported that the deflection at maximum load is not a fixed

proportion of the thickness of the slab. Eyre and Kemp [1983] reported a simple graphical procedure for determining the effect of membrane action upon the load capacity of a rigid plastic reinforced concrete slab. They pointed out that the elastic deformations, shrinkage, creep and boundary flexibilities are difficult design parameters to be determined. Subsequently they studied the inplane stiffness of slabs [1994] under compressive membrane action. Their investigation pointed out that the assumptions of an elastic full depth of element which is frequently used for slab stiffness in theoretical predictions leads to over estimation of ultimate load. Guice et al. [1989] reported tests on restrained slab strips and developed a modified procedure to predict the peak flexural capacity of the slab. It was pointed out that no procedure has been developed that could accurately predict the load deflection relationships of the slabs throughout the entire range of loading.

Rankin and Long [1997] used deformation theory to develop a method for predicting the ultimate load capacity of the laterally restrained slab strips. In their method the degree of lateral restraint was taken into account by an equivalent strip approach, based on analogy between elastically and rigidly restrained "three hinged arches". An experimental study on the strength and deformation behaviour on one way slabs subjected to distributed loading has been reported by Nayak andMenon [2004]. Recently the authors have presented a method to predict the load deflection behaviour of restrained reinforced concrete slab strips by Muthu KU,Amarnath K,Ibrahim A, Mattarneh H[2006]. They have proposed a method to predict the load deflection behaviour of fully restrained slab strips subjected to distributed loading. An experimental programme of casting and testing of ten restrained slab strips has been planned to verify the results of the proposed analysis. The proposed analysis has been developed by modifying the rigid plastic analysis of Eyre and Kemp [1983]. The fixity of the restrained slab strips was achieved by clamping the slabs to the frame by bolting through the holes provided in the slab. In the present study the partial fixity was achieved by providing an edge beam on either ends. The size of the edge beam was varied to have different degrees of partial fixity.

It has long been recognised [1955] that membrane forces in concrete slabs can considerably enhance its load-carrying capacity, compared to estimates obtained from considering only flexural theory. The most beneficial results from the previous published research into membrane

action have come from the behaviour, at small displacements, of slabs with horizontal restraint around the edges. These types of concrete slabs are subjected to compressive membrane action, which greatly enhance the load-carrying capacity. A well-known practical use of compressive membrane action is in the assessment of bridge decks [1988] that have been subjected to reinforcement corrosion. In some special cases it can be shown [1996] that these slabs can withstand the applied load without the need for reinforcing steel bars.

Fibres considerably reduce brittleness of concrete and improve its mechanical properties. Fibre concrete can be used in structural slabs. These slabs can be used for example in structural ceilings, pedestrian bridges and industrial floors. In 1980, Ghalib [1980] proposed a design method based on ultimate strength criteria for small steel fibre reinforced concrete (SFRC) slabs. This method is based on test results of eight steel fibre reinforced two-way slabs. Since then, no new method has been proposed for designing fibre concrete slabs and ACI committee 544 [1988] has recommended the same method for design of slabs with small spans. However, the committee has recommended that this method should not be used for slabs with dimensions larger than those tested by Ghalib. In 1999, Marti et al. [1999] tested circular and square slabs under point loading with continuous simple supports along the perimeter. They also presented some formulations, which estimated the results of their slab tests with specific dimensions.

However, they stated that additional tests on slabs with other parameters are required to further verify their proposed formulations. The research study was done by Ali R. Khaloo, Majid Afshari(2002)to experimentally determine flexural strength, load–deflection curve and energy absorption of small concrete slabs using various percentages of steel fibres.

CHAPTER 3

METHODOLOGY

3.1 FIELD LOAD TEST

An experimental analysis was carried out at the Bawana site with the help of civil engineering deptt. DTU to find out how much does the roof deflect when the roof is subjected to a load equal to 1.25 times the live load for which it was designed.

The test was performed on an already constructed housing at Bawana. Two dwellings (41a and 42a) were selected for the test. The two dwellings had a common wall in between where a door opening (900 mmX1850mm) was created after full construction. The area to be tested was divided into grids of 1000mmX900mm (approx.) and similarly roof was also marked with the grids. Wires in the form of grid were tied across the walls at a level of 5 meters from the floor. Weights in the form of plumb bobs were hanged from the roof with the help of cotton threads. The threads were marked at the points they were crossing the wires at no load conditions. The live load was then transferred to the roof in the form of sand bags. The load was transferred in five stages 20% (approx.) at a time. After every 20% loading the deflection in the roof was measured at the thread and corresponding reading was taken from the floor as well. After the full loading the deflections were measured and the roof was left with the load for 24 hrs. The final reading was of the deflection was recorded and compared with the allowable deflection based on IS-456 : 2000 article 17.6.3. The unloading was done and the final recovery was recorded.

3.1.1 CALCULATIONS FOR THE APPLIED LOADS:

The area of one dwelling came out to be 16.08 sq.m (excluding toilet area).

As the housing was for a load of 200kg/sq.m , the total load to be applied is as:

16.08 x 200 x 1.25= 4020 kgs

The room was divided into grids of 1000mm x 900mm . The total no. grids on which the test is to be performed is as:

16.08/1000x900 =18 nos.

Calculation of loads

load per grid = 4020/18 = 225 kg

Assuming two bags per grid ,load per bag =22.5kg and

No. of bags in one layer for both the rooms = 18x2 x2=72 bags

Load /layer=72x22.5= 1620 kg (from layer 1 to 4)

Load in the last layer = 1560 kg (70 bags)

3.1.2 CALCULATION FOR ALLOWABLE DEFLECTION:

As per IS : 456 -2000 article 17.6 it is mentioned that the structure should be tested for full dead load and 1.25 times the live load. Dead weight includes weight of finishes and partition walls if any. The deflections are then compared with the maximum allowable deflection which is as given below.

Allowable deflection =40x L^2/ D
Where L = effective span in meters
And D=depth of slab in mm.

For shorter span
Where, L= 3.6m and D=100mm
Allowable deflection =40x L^2/ D
=40x 3.6^2/100=5.18mm

For longer span
Where, L= 4 m and D= 100mm
Allowable deflection =40x L^2/ D
=40x 4^2/100 =6.4mm

Adopting the least of the above two values i.e. 5.18mm

For the field load test ,the grids were marked over the floor and the roof with the help of colour.

Fig.1 Showing marking on roof slab

With the help of drilling machine hooks were inserted in the roof , walls , across the door opening and top of the door opening.

Fig.2 Door opening created in casted wall

The wires were tied at a height of five feets from the floor and plumb bobs were hanged from the roof crossing the wires.

Fig 3 Showing plumb bobs suspending from the roof

Fig. 4 Plumb bobs crossing the grid at five feet height

The readings were noted from the plumb bob to the floor with no live load and threads of the plumb bobs are marked at the point where it crosses the wires.

First layer of the load is applied and the reading is again recorded after one hour.

Fig.5 Applying first layer of loads

Second to fifth layer of bags are applied with recording the reading every time after one hour of loading.

Fig.6 Application of total applied load

The load was kept for 24 hrs. and final reading was noted down. Actual deflection is compared with allowable deflection .Unloading of the load was done and the readings for recovery was taken after 48 hrs.

3.2 NUMERICAL ANALYSIS

The Residential building was analysed by using the STAADPRO and stresses found in the wall and slabs were checked for the case without any opening. The choice of software is done based on the fact that it is user friendly, faster, compatible, results are reliable and is extensively used everywhere. The software is easy to learn and use and practically all type building can be modelled, analysed and designed with it.

3.2.1 CAPABILITY OF STAAD

STAAD.Pro V8i is one of the leading structural analysis and design software which supports more than 100 steel, concrete and timber design codes and has the largest worldwide user base. STAAD stands for Structural Analysis and Design. It is one of the software applications created to help structural engineers automate their tasks ,to remove the tedious and long procedures of the manual methods of structural analysis and design.STAAD.Pro allows structural engineers to analyse and design virtually any type of structure through its flexible modeling environment, advanced features and fluent data collaboration. Flexible modeling is provided by the graphical environment and the design supports over 70 international codes and over 20 U.S. codes in seven languages. STAADPro is a highly intuitive and easy to use, 2D/3D frame analysis program. It's strength is in the highly graphical nature of the user interface with it's point and edit interaction. This friendly approach to structural analysis software concept has been the basis of the far more powerful and extensive, STAAD/Pro-STAAD, and complements the suite by maintaining the principle of keep it simple.

Since, STAAD uses the Matrix Displacement Method of structural analysis, there are two Cartesian coordinate systems - the local and the global. The geometry of the structure as a whole is defined by the nodes at the ends of the various structural members, and each node has a unique number. Each member also has a unique number and the topology of the member is defined relative to the node numbers at its ends. The location of each node is defined relative to a global coordinate system. By default, the origin of the global coordinate system is at node number1.The location of points or sections within each structural member is defined relative to the local coordinate system with the origin at the left end node of the member viewed horizontally. Each member has its own local coordinate system.

3.2.2 FUNDAMENTAL ANALYSIS AND COMMANDS OF STAAD

Regardless of the structure being analysed, the following are fundamental steps and STAAD command keywords shown in the brackets:

1. Define whether the problem is 2D or 3D (STAAD PLANE or SPACE)
2. Define the length and force units (UNITS)

3. Define the nodes and their locations (JOINT COORDINATES)

4. Define the member and their nodes (MEMBER INCIDENCES)

5. Define the section properties of the members, Ix, etc (MEMBER PROPERTY)

6. Define the mechanical properties of the members such as the Young's modulus,

density, etc (CONSTANTS)

7. Define the support conditions (SUPPORTS)

8. Define the load cases (LOAD)

9. Define the loads of each load case as member loads, joint loads, (or code loads)

(MEMBER LOAD or JOINT LOAD)

10. Define the load combinations (LOAD COMB)

11. Analyse the structure (PERFORM ANALYSIS)

12. Define the output format (PRINT)

13. Finish the run (FINISH)

These commands are stored automatically in STAAD in a file with the extension .std. This file is formatted as an ASCII text file which means that it can be edited outside of STAAD with a word processor or any other text editor. Therefore, you can also write the input file independantly of STAAD and just refer to it when you enter STAAD to run the analysis.

By using the "File" menu STAAD reads the .std file as its input and automatically creates an output file with the extension .anl. This file is also a text file and is useful for including in calculation reports.

STAAD also creates certain other output files for its internal use. STAAD creates a database for your analysis, .dbs, and files for the bending moments .bmd, displacements, .dsp, reactions, .rea, among others.

The aforementioned STAAD commands are incomplete by themselves - they are the keywords of the commands. The complete commands follow a particular syntax to completely describe the problem. Though we can use a totally character-based approach with STAAD, the most effective use of STAAD is when we use the PRE-PROCESSOR of STAAD to write the .std for us. The PRE-PROCESSOR is a set of functions within STAAD that you select from the menus of STAAD's GUI. Each of the command keywords presented in brackets earlier has an icon or menu that you click on to create the commands in the .std file.

From within STAAD you can see the contents of the .std file at any time, and the .anl file as well after analysis. To do this you just click on the icon for each. This puts the file on the screen and you can edit the file from there if you wish. It can make use of various forms of analysis from the traditional 1st order static analysis, 2nd order p-delta analysis, geometric non linear analysis or a buckling analysis. It can also make use of various forms of dynamic analysis from modal extraction to time history and response spectrum analysis. It uses the Matrix Displacement Method of structural analysis.

Continuum structures (plates, slabs, walls, shells, tanks, etc) are modelled in STAAD by using finite elements. The following is with respect to the element of the STAAD library which can be quadrilateral or triangular.

3.2.3 ANALYSIS WITH STAAD

The STAAD Analysis Engine has 2D and 3D capabilities for solving problems containing Beams, plate elements and 8 noded bricks. The general nature of the solution engine allows beam models using the stiffness matrix method to be combined with finite elements. A wide range of support conditions, load types and various other member/element specifications are available for combination with these features

3.2.4 LOAD TYPES AND SUPPORTS

i. Loads for nodes,beams,elements including concentrated, uniform,linearly varying, trapezoidal, etc.
ii. Uniform ,linearly varying element pressure on all or part of element.
iii. Floor /area load converts force per area to member loads on one way or two way action.
iv. Support conditions includes fixed , pinned,linear/non linear springs,auto spring generator etc.
v. Special specifications like Member offsets, partial releases,tension/compression only, cable/truss only etc .

3.2.5 POST PROCESSING

The STAAD processor offers extensive capability for viewing and verification of models. Interactions between the graphical model and the tabular results makes assessment extremely rapid. The logical screen layout allows graphical results, such as bending moment diagrams, to be viewed along the side tables where summaries can be displayed showing maximum and minimum values.

3.3 STAAD MODELLING FOR THE PRESENT STUDY

The STAAD modeling of the housing was done with the graphical aid in the modeling mode. First of all joint coordinates were plotted graphically and then walls were created as elements using curser. Similarly, the slab was formed as plate elements. The above floors were generated using repeat command. The support condition was generated as fixed. Element properties were assigned to the walls and slab as 100mm thickness. The dead weight and live load were applied. Load was applied as self weight of the slab, dead weight of the finishes and the live load as per IS: 875(part 2). As per this code, live load for the housing unit is 200 kg/m^2.A combination of loads is also applied to find the maximum deflection due to live load and dead loads. The model is also analysed for seismic loads and combination of seismic loads with dead and live loads. As the actual building is casted with a concrete of M15 grade, the grade of concrete in the Staad model is also maintained the same. After analysing the model, the results were observed in the post processing mode. The deflections were observed in the graphical window and compared for different load cases.

CHAPTER 4

LOAD TEST RESULTS :

As shown below, the two dwellings are marked with the grids. The deflections in the slab at these grid points were observed and the results are present in tabular and graphical form below.

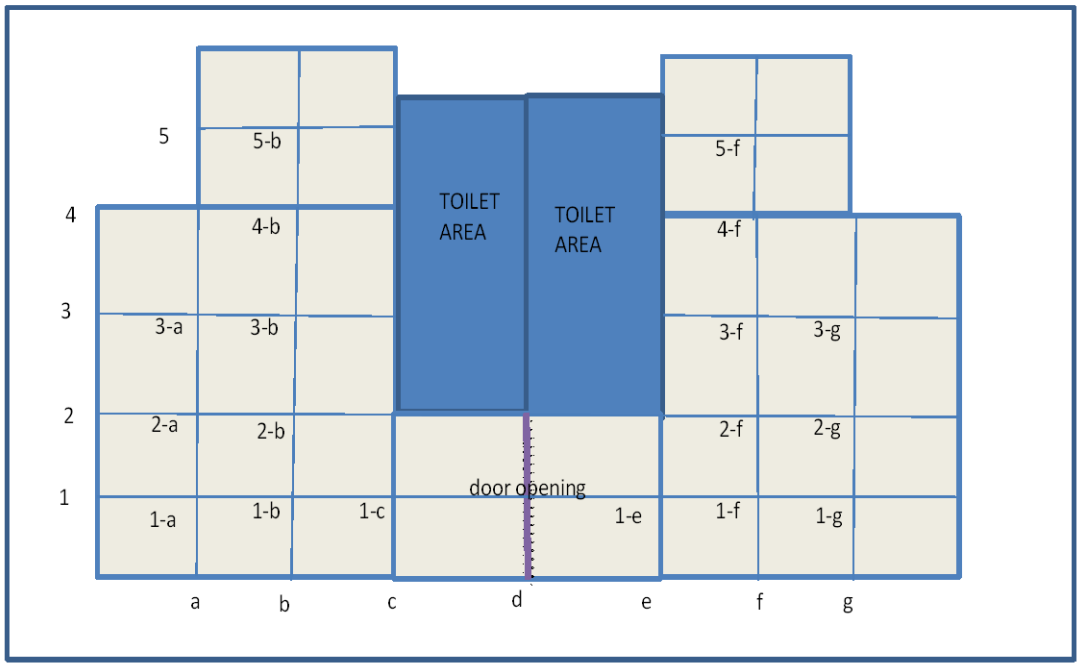

Fig.7 Showing plan of two flats where the load test was performed

4.1 Deflection at grid points

Table- 1 Deflections at point 1-a

Loading in %	Time in hrs.	Deflection in mm
0	0	0
20	1	2
40	2	3
60	3	5
80	4	5
100	5	6
100	6	6
100	29	5
0	53	1
0	72	

- Represents unloading points
- Represents loading points

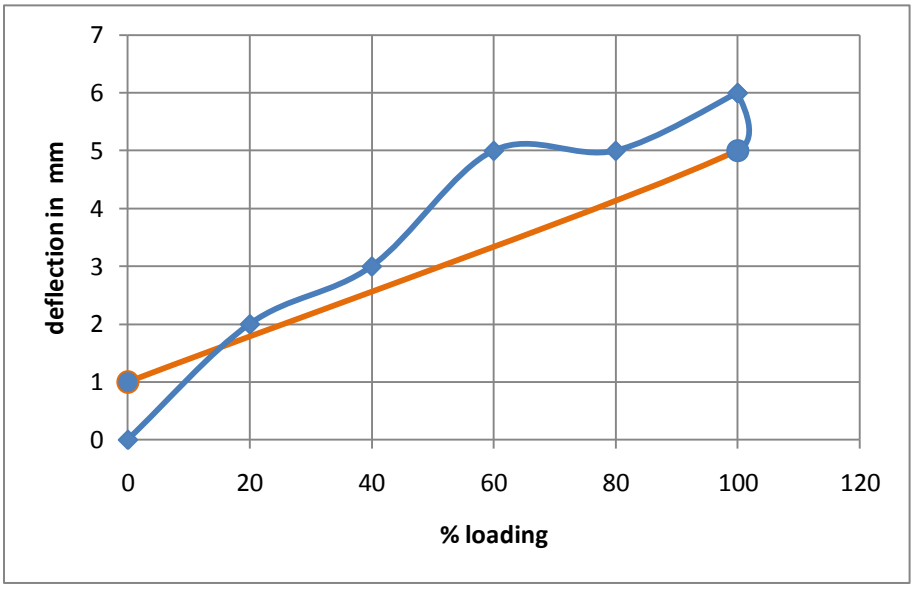

Fig.8 Deflection at a point (1-a) on slab on loading and its recovery after unloading

able- 2 Deflections at point 2-a

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	3
60%	3	5
80%	4	5
100%	5	6
100%	6	6
100%	29	5
0%	53	3
0%	72	

- **Represents unloading points**
- **Represents loading points**

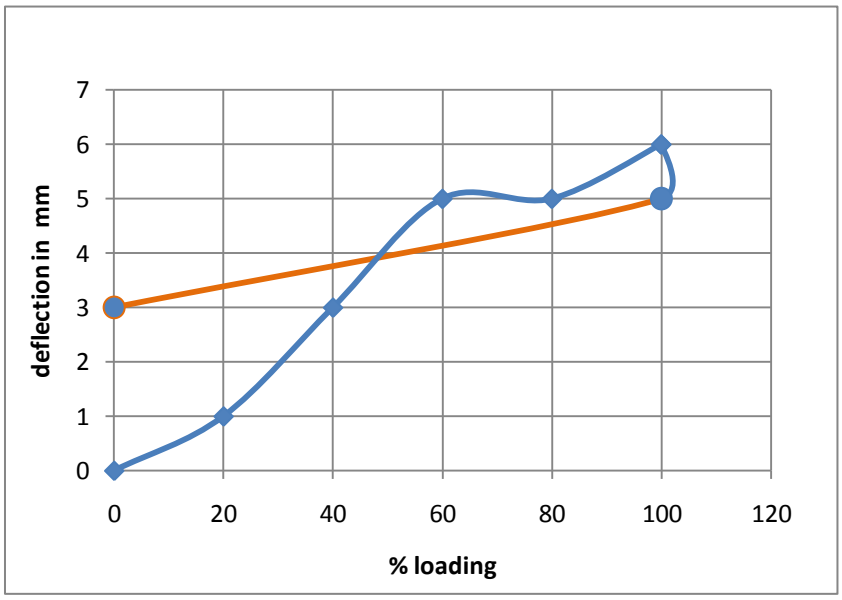

Fig.9 Deflection at a point (2-a) on slab on loading and its recovery after unloading

Table- 3 Deflections at point 3-a

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	3
60%	3	5
80%	4	7
100%	5	7
100%	6	6
100%	29	4
0%	53	3
0%	72	

- **Represents unloading points**
- **Represents loading points**

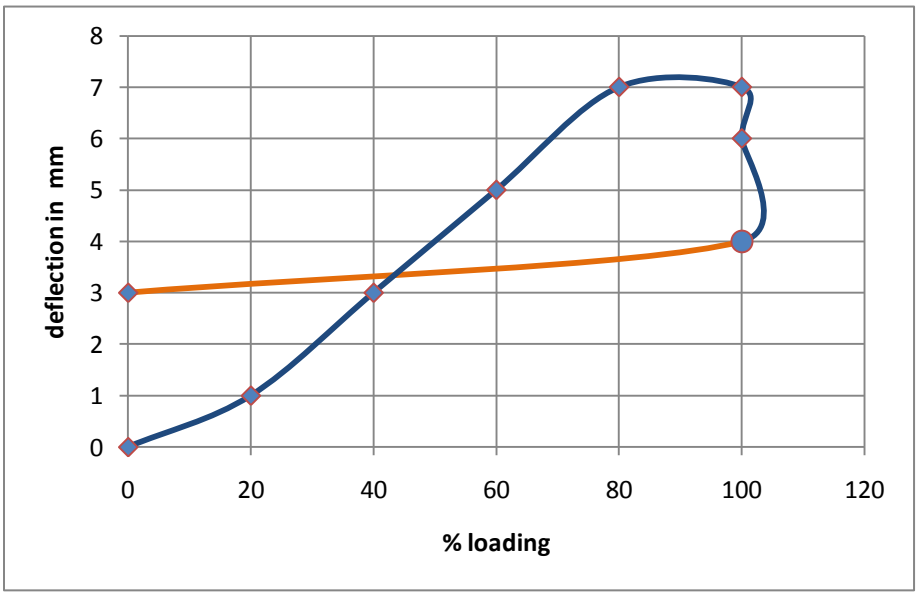

Fig.10 Deflection at a point (3-a) on slab on loading and its recovery after unloading

Table- 4 Deflections at point 1-b

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	3
60%	3	4
80%	4	5
100%	5	5
100%	6	6
100%	29	5
0%	53	2
0%	72	

- **Represents unloading points**
- **Represents loading points**

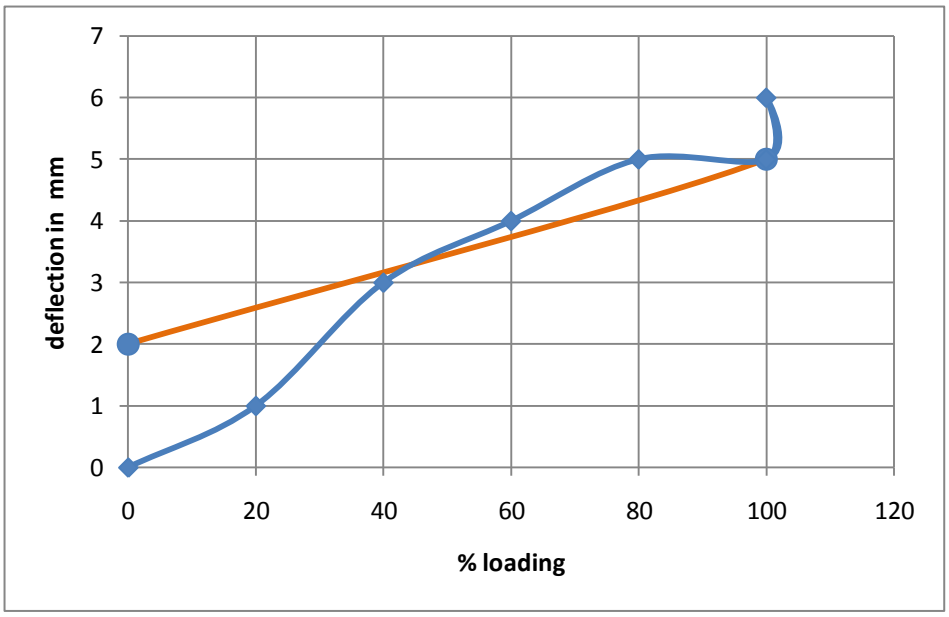

Fig.11 Deflection at a point (1-b) on slab on loading and its recovery after unloading

Table- 5 Deflections at point 2-b

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	1
60%	3	2
80%	4	3
100%	5	3
100%	6	4
100%	29	5
0%	53	3
0%	72	

- **Represents unloading points**
- **Represents loading points**

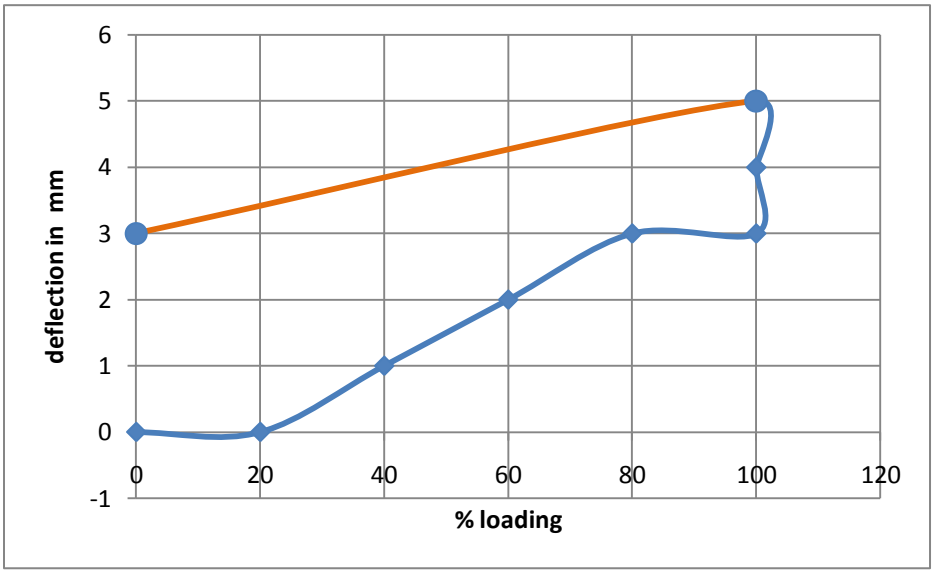

Fig.12 Deflection at a point (2-b) on slab on loading and its recovery after unloading

Table- 6 Deflections at point 3-b

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	0
60%	3	1
80%	4	1
100%	5	2
100%	6	2
100%	29	2
0%	53	
0%	72	

- **Represents unloading points**
- **Represents loading points**

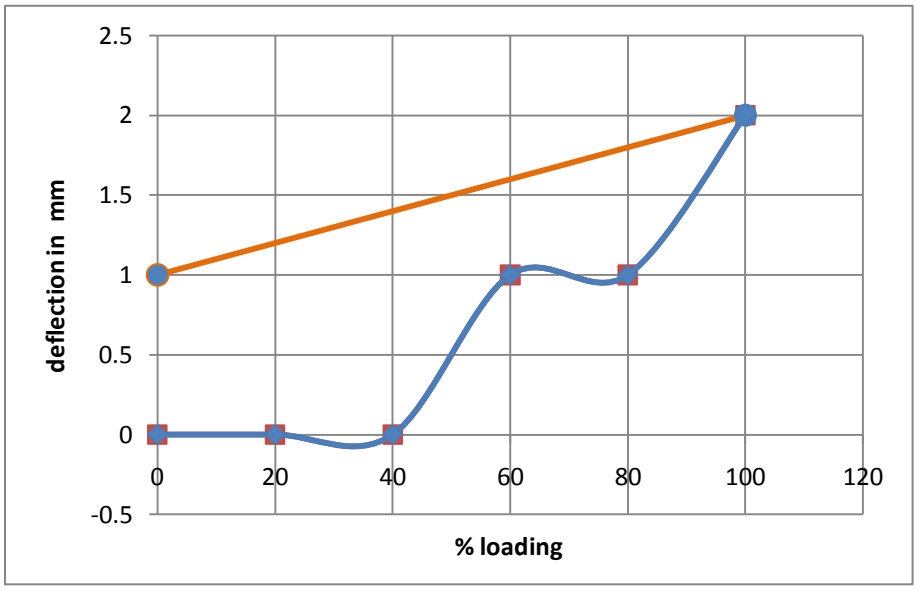

Fig.13 Deflection at a point (3-b) on slab on loading and its recovery after unloading

Table- 7 Deflections at point 4-b

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	0
60%	3	0
80%	4	1
100%	5	1
100%	6	1
100%	29	1
0%	53	
0%	72	

- **Represents unloading points**
- **Represents loading points**

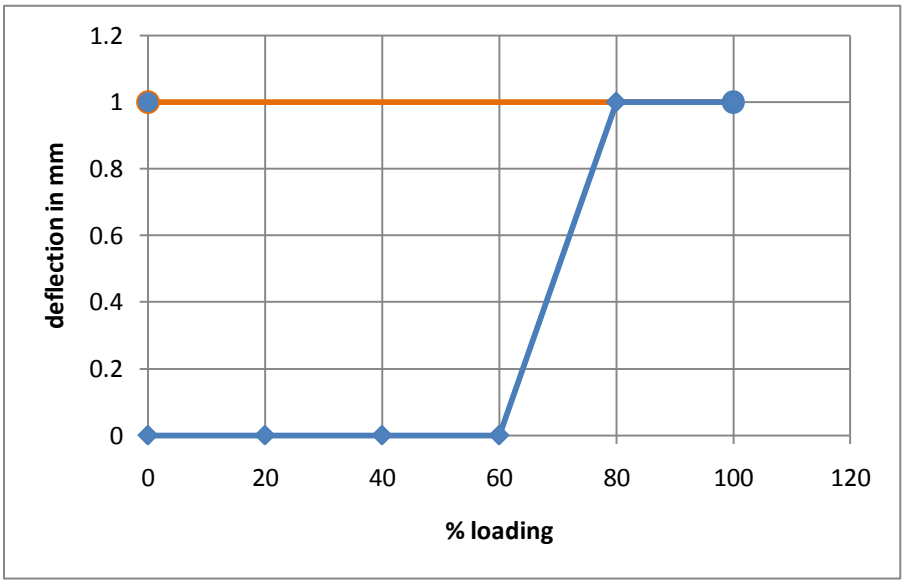

Fig.14 Deflection at a point (4-b) on slab on loading and its recovery after unloading

Table- 8 Deflections at point 5-b

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	0
60%	3	1
80%	4	2
100%	5	2
100%	6	2
100%	29	3
0%	53	1
0%	72	

- **Represents unloading points**
- **Represents loading points**

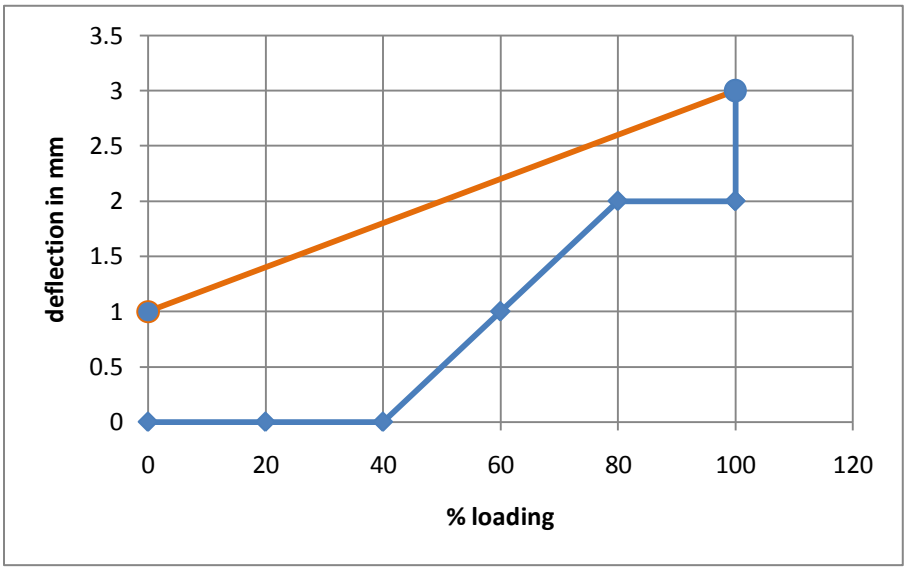

Fig.15 Deflection at a point (5-b) on slab on loading and its recovery after unloading

Table- 9 Deflections at point 1-c

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	0
60%	3	1
80%	4	1
100%	5	2
100%	6	2
100%	29	2
0%	53	1
0%	72	

- **Represents unloading points**
- **Represents loading points**

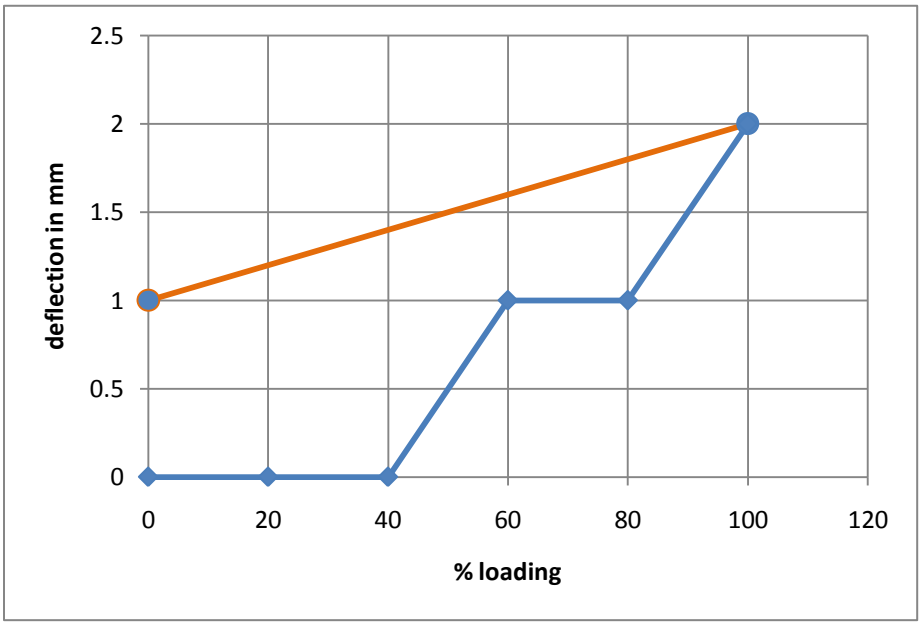

Fig.16 Deflection at a point (1-c) on slab on loading and its recovery after unloading

Table- 10 Deflections at point 1-e

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	2
40%	2	3
60%	3	4
80%	4	4
100%	5	4
100%	6	4
100%	29	4
0%	53	0.5
0%	72	

- **Represents unloading points**
- **Represents loading points**

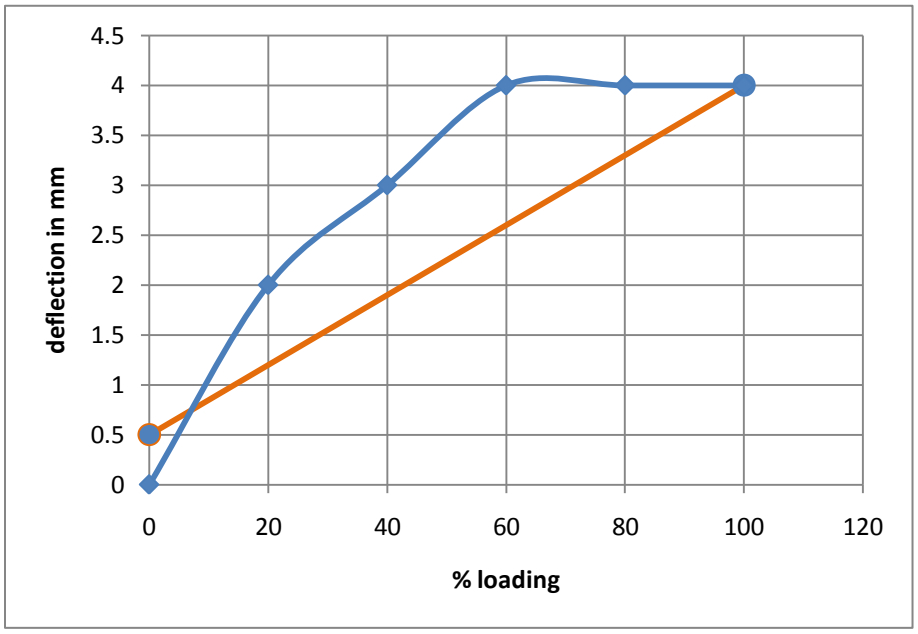

Fig.17 Deflection at a point (1-e) on slab on loading and its recovery after unloading

Table- 11 Deflections at point 1-f

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	0
60%	3	0
80%	4	2
100%	5	3
100%	6	3
100%	29	3
0%	53	1
0%	72	

- **Represents unloading points**
- **Represents loading points**

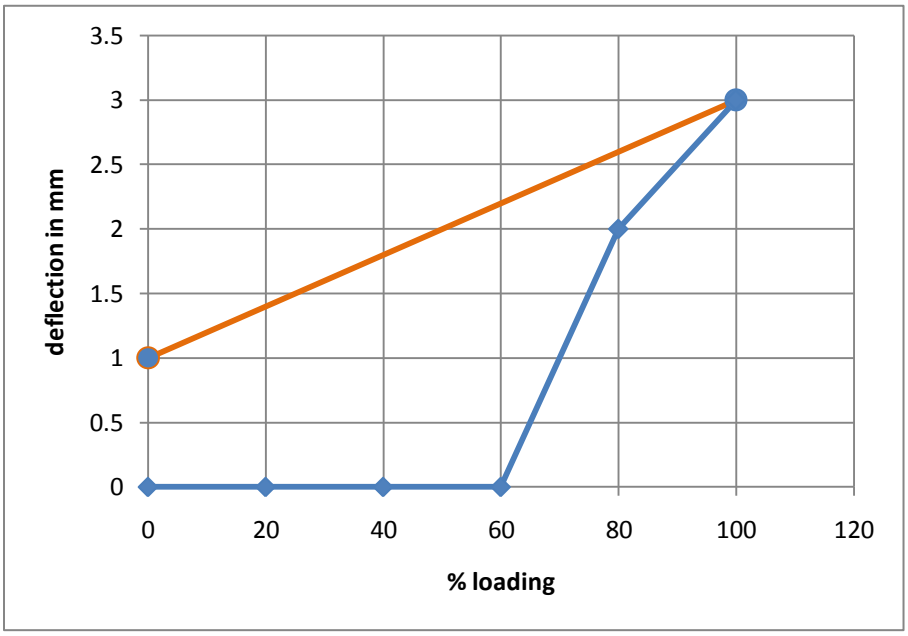

Fig.18 Deflection at a point (1-f) on slab on loading and its recovery after unloading

Table- 12 Deflections at point 2-f

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	4
60%	3	4
80%	4	6
100%	5	7
100%	6	7
100%	29	4
0%	53	2
0%	72	

- **Represents unloading points**
- **Represents loading points**

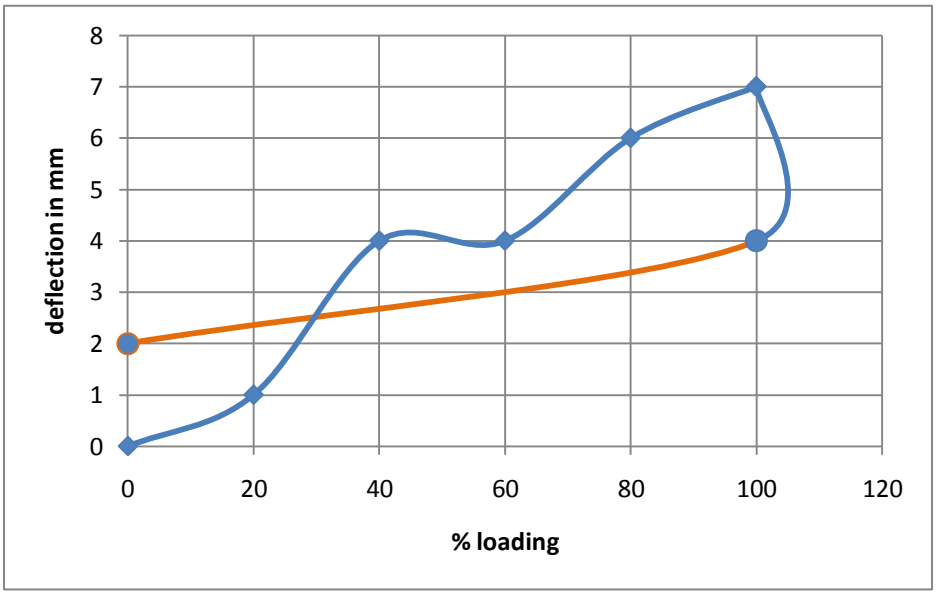

Fig.19 Deflection at a point (2-f) on slab on loading and its recovery after unloading

Table- 13 Deflections at point 3-f

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	1
60%	3	1
80%	4	3
100%	5	4
100%	6	4
100%	29	4
0%	53	2
0%	72	

- **Represents unloading points**
- **Represents loading points**

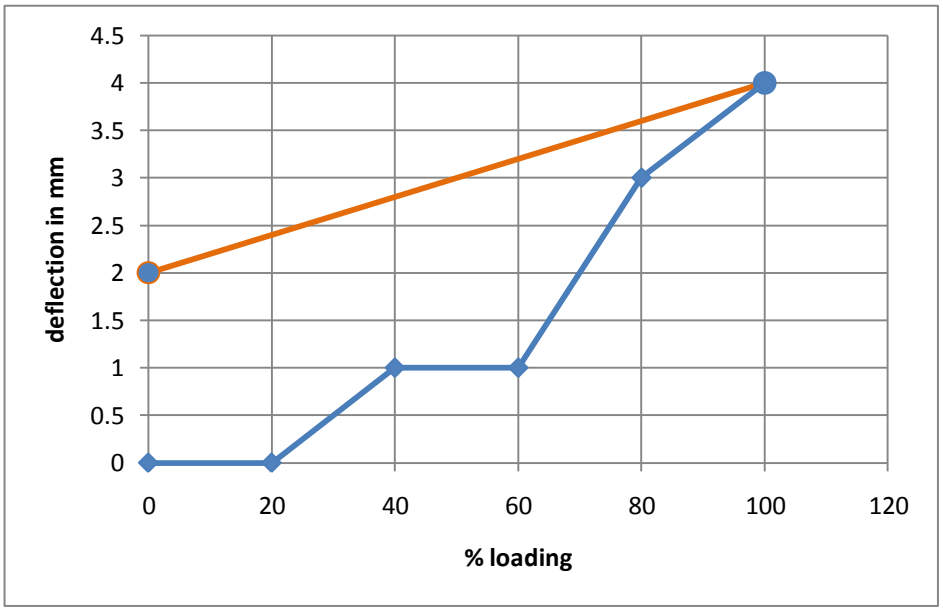

Fig.20 Deflection at a point (3-f) on slab on loading and its recovery after unloading

Table- 14 Deflections at point 4-f

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	1
60%	3	1
80%	4	2
100%	5	2
100%	6	3
100%	29	3
0%	53	2
0%	72	

- **Represents unloading points**
- **Represents loading points**

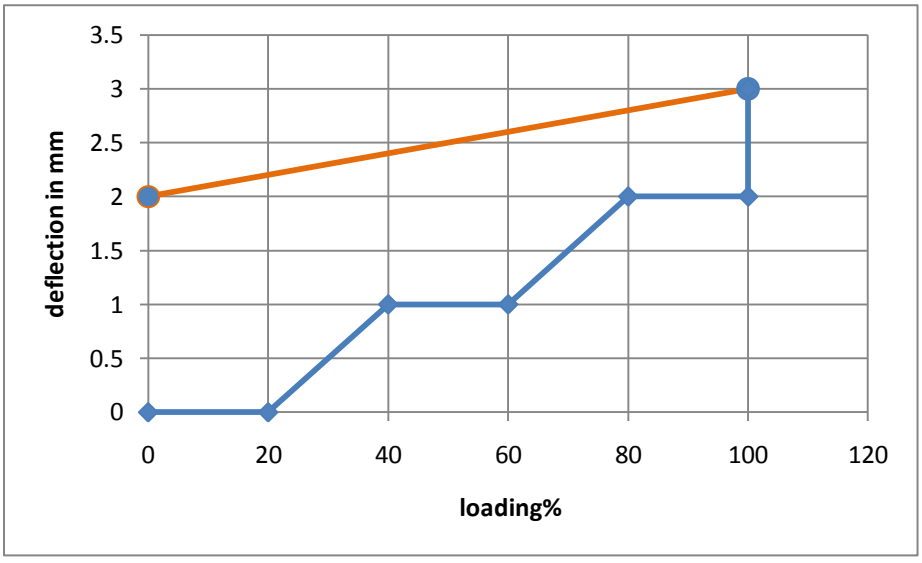

Fig.21 Deflection at a point (4-f) on slab on loading and its recovery after unloading

Table- 15 Deflections at point 5-f

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	2
60%	3	3
80%	4	5
100%	5	6
100%	6	6
100%	29	6
0%	53	4
0%	72	

- **Represents unloading points**
- **Represents loading points**

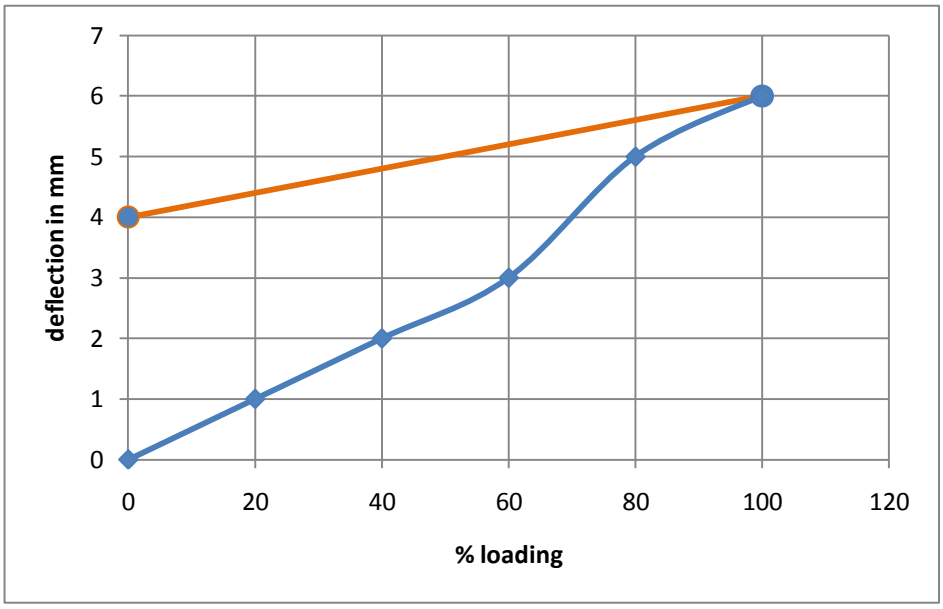

Fig.22 Deflection at a point (5-f) on slab on loading and its recovery after unloading

Table- 16 Deflections at point 1-g

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	1
60%	3	3
80%	4	4
100%	5	4
100%	6	4
100%	29	3
0%	53	2
0%	72	

- **Represents unloading points**
- **Represents loading points**

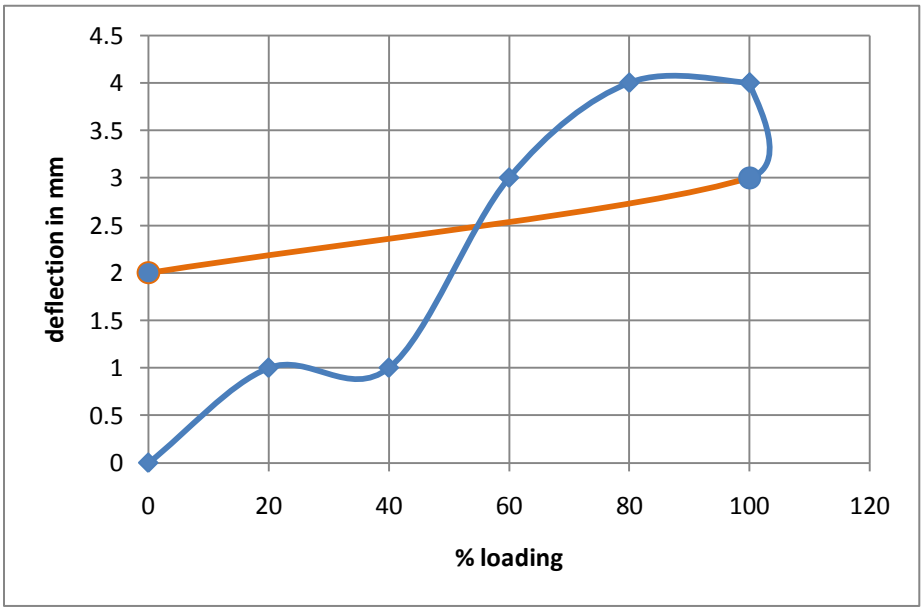

Fig.23 Deflection at a point (1-g) on slab on loading and its recovery after unloading

Table- 17 Deflections at point 2-g

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	1
40%	2	1
60%	3	2
80%	4	4
100%	5	4
100%	6	4
100%	29	5
0%	53	3
0%	72	

- **Represents unloading points**
- **Represents loading points**

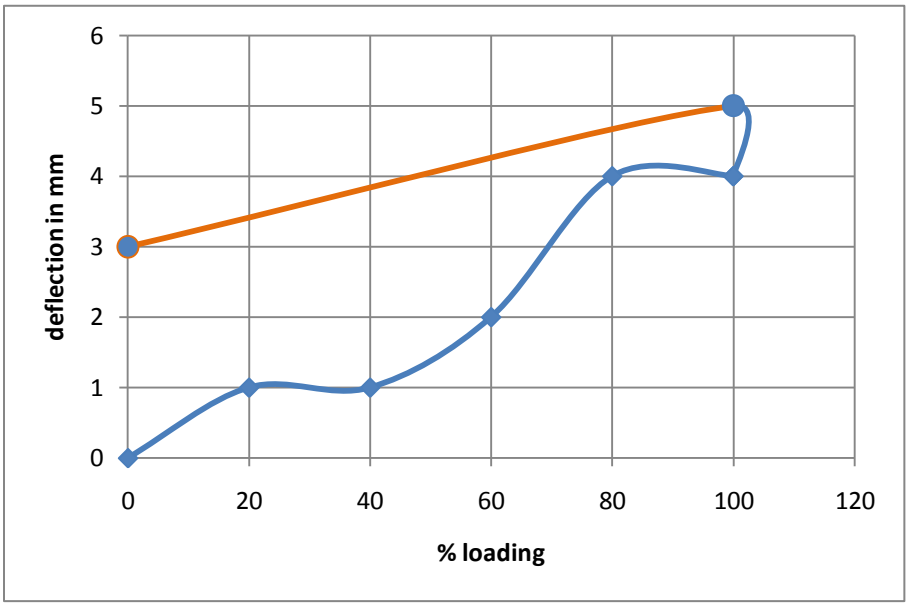

Fig.24 Deflection at a point (2-g) on slab on loading and its recovery after unloading

Table- 18 Deflections at point 3-g

loading	Time in hrs.	Deflection in mm
0	0	0
20%	1	0
40%	2	1
60%	3	1
80%	4	2
100%	5	2
100%	6	2
100%	29	3
0%	53	1
0%	72	

- **Represents unloading points**
- **Represents loading points**

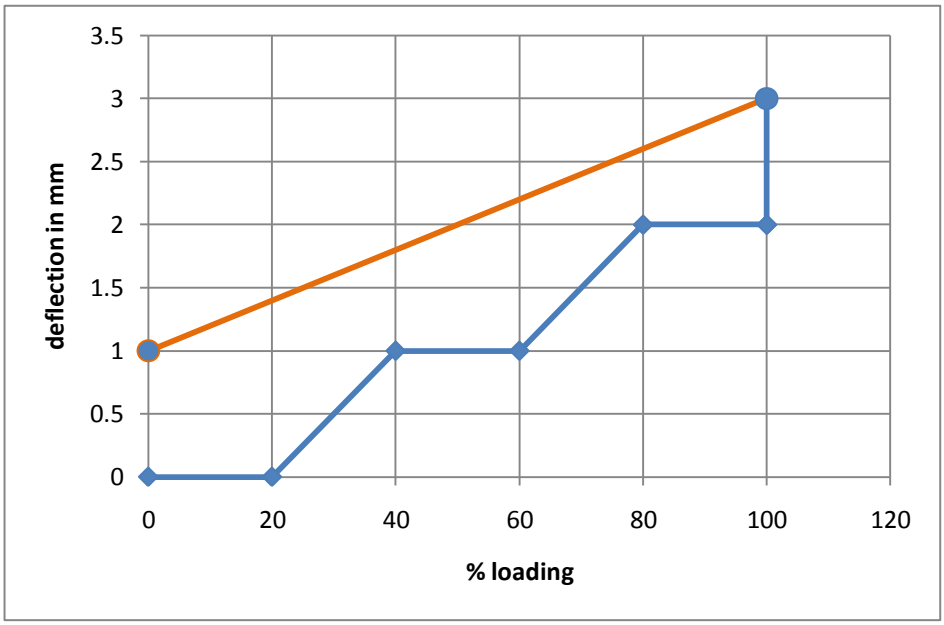

Fig.25 Deflection at a point (3-g) on slab on loading and its recovery after unloading

Table- 19 Consolidated data of total deflection for all grid points:

Grid no.	initial reading	final reading	total deflection in mm	allowable deflection in mm	Remarks
Grid 1-a	**1485**	**1480**	**5**	**5.18**	safe
Grid 2-a	**1460**	**1455**	**5**	**5.18**	safe
Grid 3-a	**1480**	**1476**	**4**	**5.18**	safe
Grid 1-b	**1460**	**1455**	**5**	**5.18**	safe
Grid 2-b	**1465**	**1462**	**3**	**5.18**	safe
Grid 3-b	**1463**	**1461**	**2**	**5.18**	safe
Grid 4-b	**1466**	**1465**	**1**	**5.18**	safe
Grid 5-b	**1471**	**1469**	**2**	**5.18**	safe
Grid 1-c	**109**	**107**	**2**	**5.18**	safe
Grid 1-e	**117**	**113**	**4**	**5.18**	safe
Grid 1-f	**1466**	**1465**	**1**	**5.18**	safe
Grid 2-f	**1464**	**1460**	**4**	**5.18**	safe
Grid 3-f	**1455**	**1453**	**2**	**5.18**	safe
Grid 4-f	**1460**	**1459**	**1**	**5.18**	safe
Grid 5-f	**1455**	**1450**	**5**	**5.18**	safe
Grid 1-g	**1480**	**1477**	**3**	**5.18**	safe
Grid 2-g	**1464**	**1460**	**4**	**5.18**	safe
Grid 3-g	**1447**	**1444**	**3**	**5.18**	safe

4.3 Deflection along the grid lines

Table- 20 Deflections along grid 1

Distance in met.	Deflection in mm,room1	Deflection in mm,room2
0	0	0
1	5	4
2	5	5
3	2	3
4	0	0

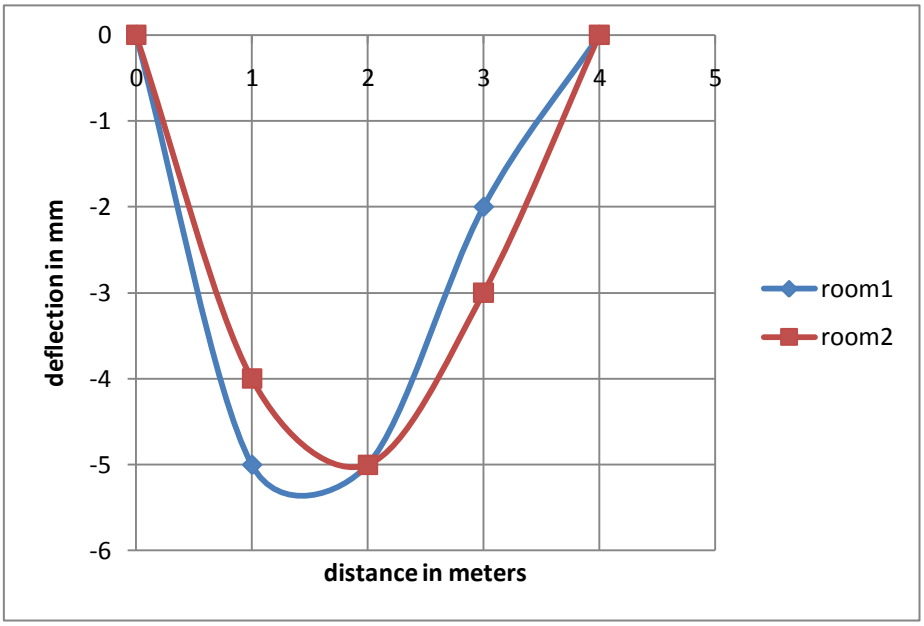

Fig.26 Deflection at different points along grid 1 for room 1 and room 2

Table- 21 Deflections along grid a and grid g

Distance in met.	Deflection in mm,grid a	Deflection in mm,grid g
0	0	0
0.9	5	3
1.8	5	4
2.7	4	3
3.6	0	0

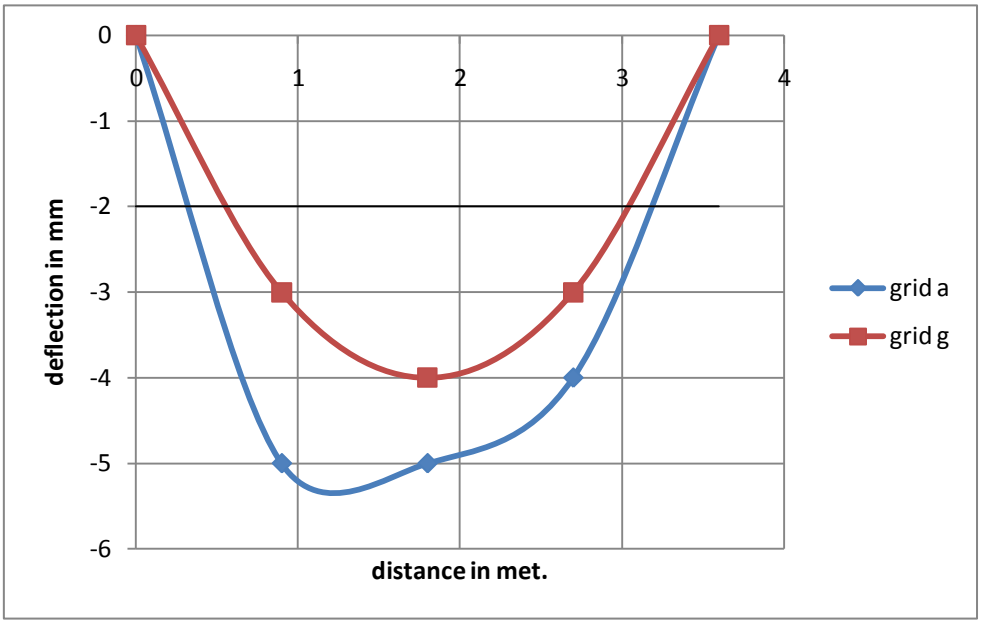

Fig.27 Deflection at different points along grid a (room1) and grid g (room 2)

Table- 22 Deflections along grid b and grid f

Distance in met.	Deflection in mm,grid b	Deflection in mm,grid f
0	0	0
0.9	5	2
1.8	3	3
2.7	2	2
3.6	1	1
4.5	2	2
5.4	0	0

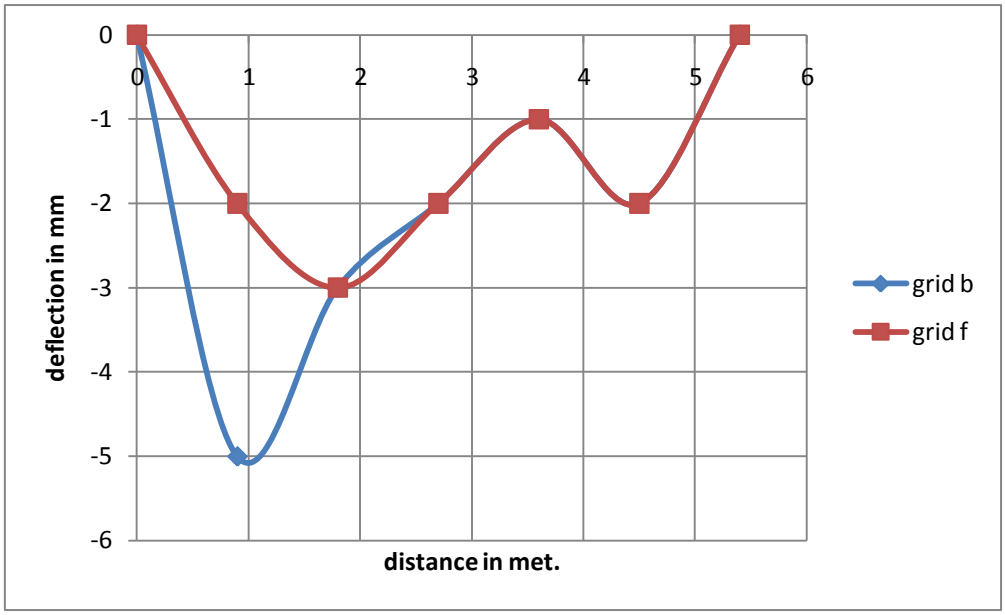

Fig.28 Deflection at different points along grid b (room 1) and grid f (room 2)

Table- 23 Deflection at proposed door opening

loading	Time Spent	Deflection at top		
		at first 100mm	at center	at last 100mm
0	0	0	0	0
20%	1	0	0	0
40%	2	0	1	0
60%	3	0	1	0
80%	4	0	1	0
100%	5	0	2	0
100%	6	0	2	0
100%	29	0	1	0
0%	53	0	1	0
0%	72	0		0

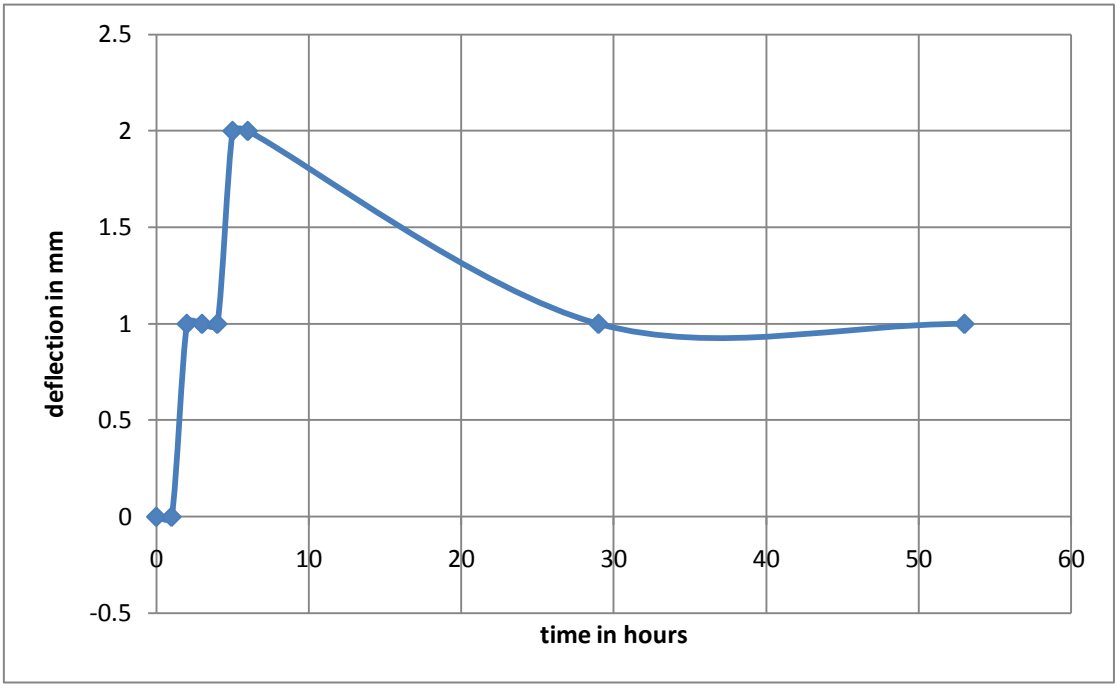

Fig.29 Deflection in the opening of common wall

4.5 DISSCUSSION

All the points of the roof slab deflected from their original position when full dead load and 1.25 x live load was applied. The deflection varies from minimum 1 mm to 5mm maximum. When the load was left for 24 hrs., they further deflected to their maximum upto 7mm. But when the slab was unloaded, the deflected slab tried to rebounce to its original shape but only.The part of deflection in the slab which was recovered immediately after the removal of loads was the elastic deformation. The part of deformation which was not recovered at all is the plastic deformation.

As can be seen from the graphs that in some of the points of the slab the plastic deformation is about 3mm,which is about 50 percent of the total deflection of the slab. Such things cannot happen, so it may be human error in recording the readings of deflections for those particular points.

CHAPTER 5

NUMERICAL ANALYSIS RESULTS

Two units of the housing were modeled using the STAADPRO and the results obtained were studied and discussed here . The results of the node displacements along the different grids are presented in the form of graph.

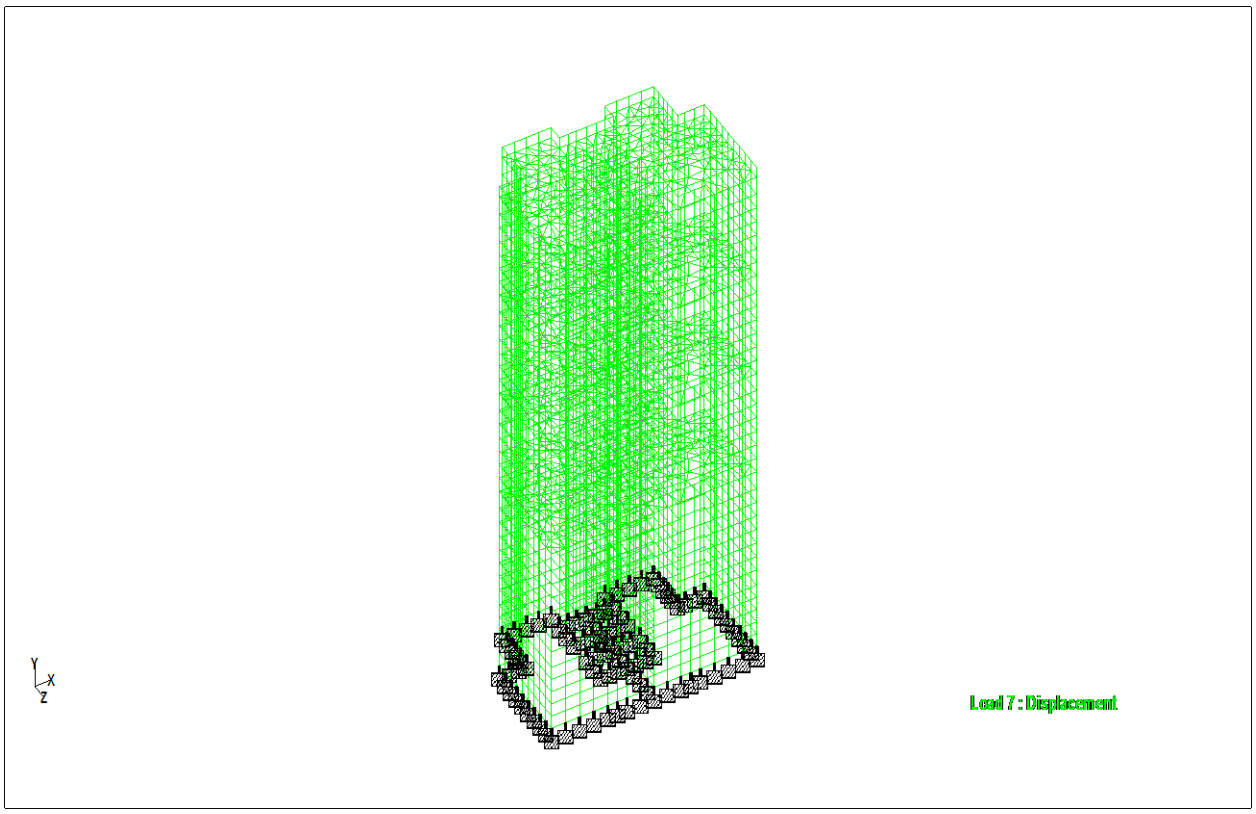

Fig.30 Showing STAAD model of the building

Deflections of slab (without opening)

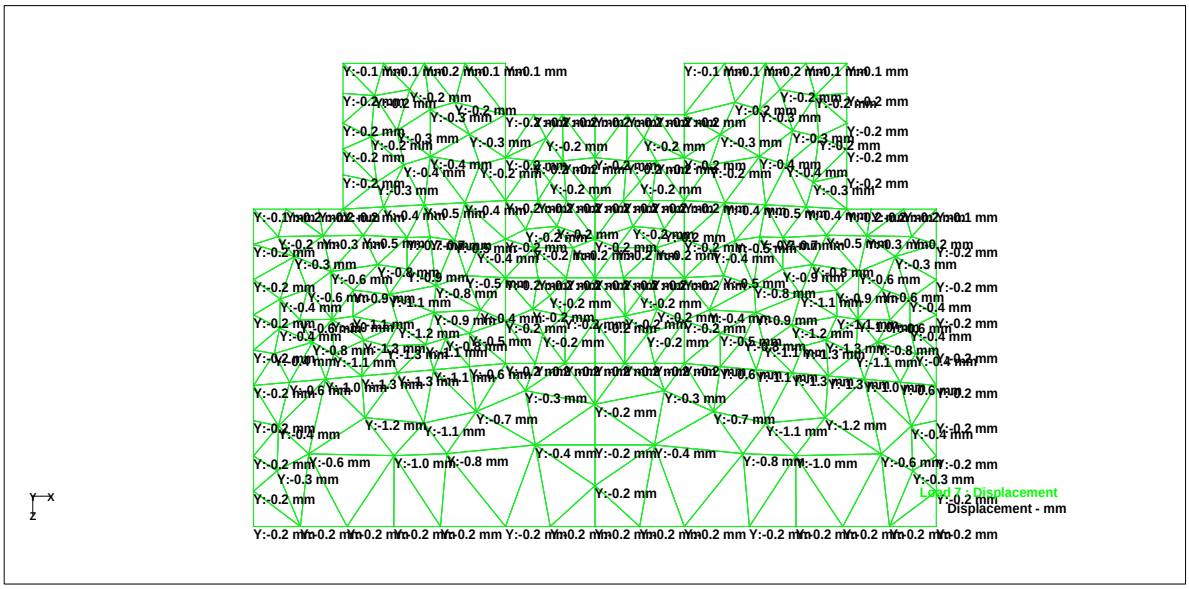

Fig.31 Deflection at different points of RCC slab(without opening)

Deflections of slab (with opening)

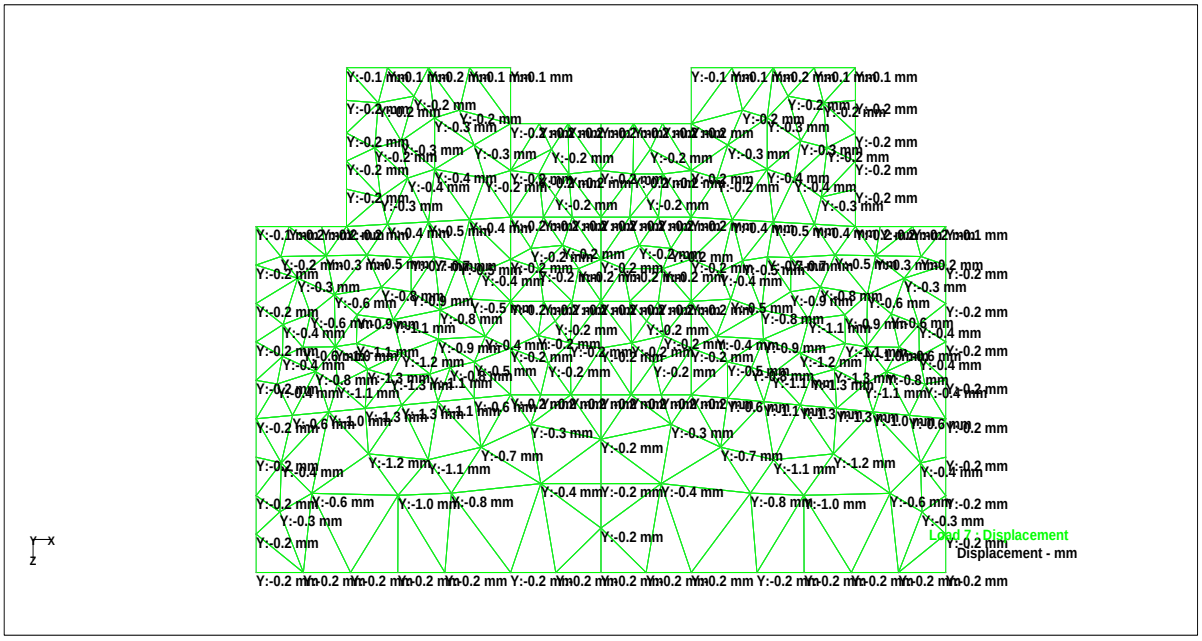

Fig.32 Deflection at different points of RCC slab(with opening)

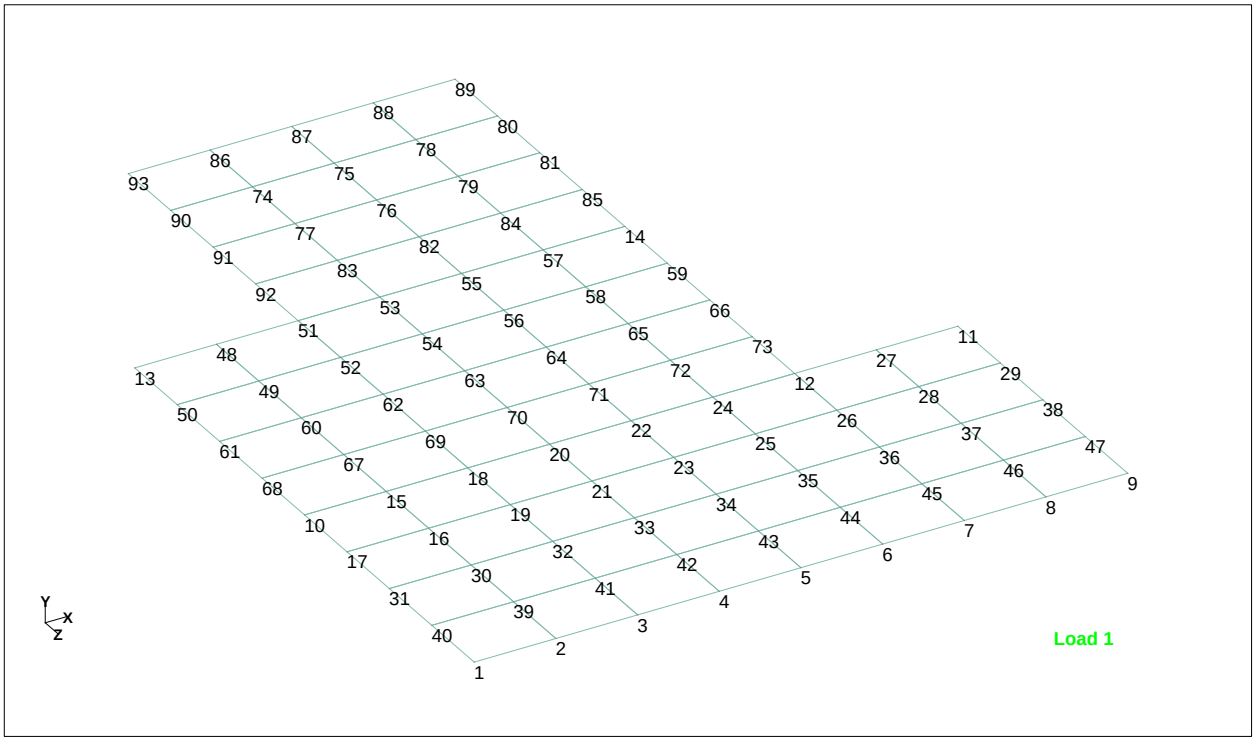

Fig.33 Showing node numbers at different points of RCC slab

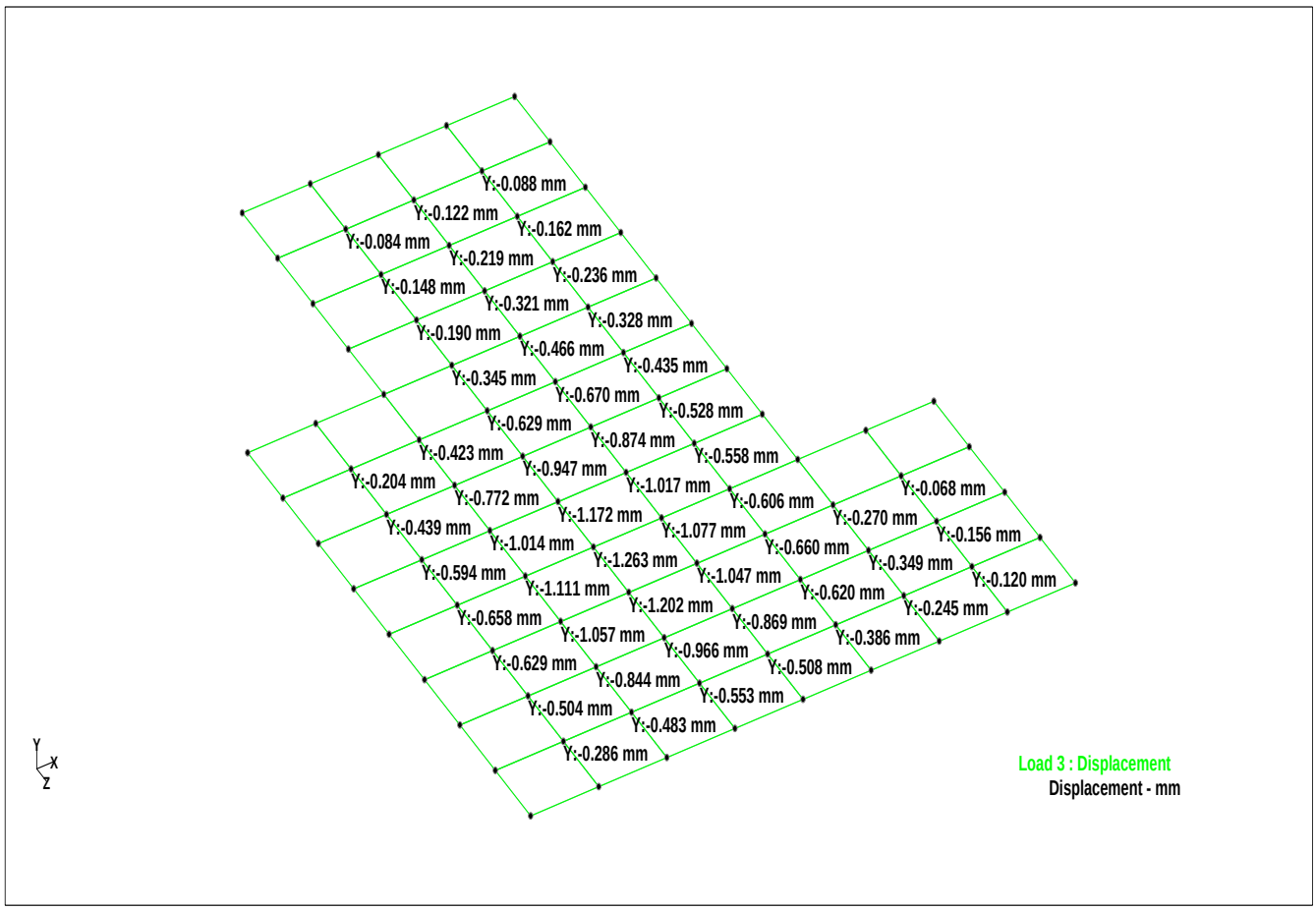

Fig.33 Showing deflection at different points of RCC slab

5.1 NODE DISPLACEMENTS ALONG THE GRIDS

Table- 24 Deflections along grid 1

Distance in met.	Deflection in mm,room1
0	0
0.5	-0.504
1	-0.844
1.5	-0.966
2	-0.869
2.5	-0.62
3	-0.35
3.5	-0.156
4	0

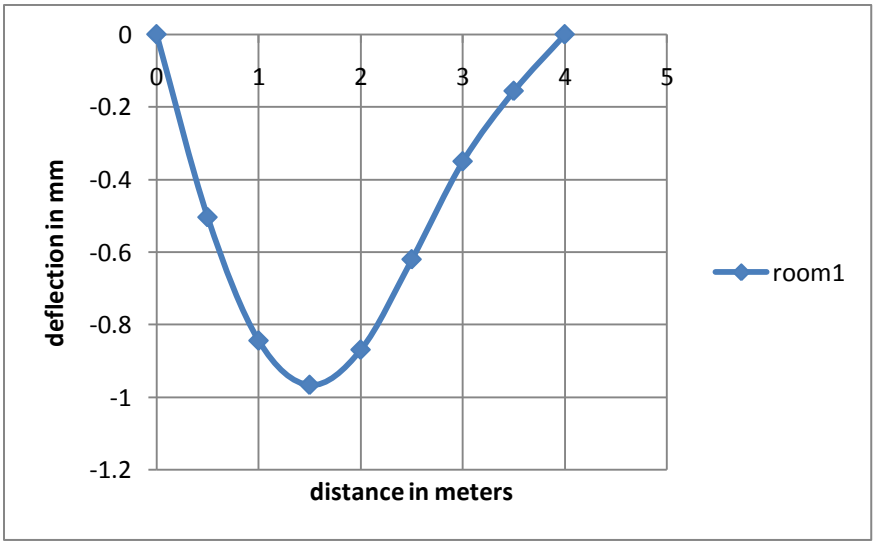

Fig.35 Deflection at different points along grid 1 for room 1

Table- 25 Deflections along grid a

Distance in met.	Deflection in mm,grid a
0	0
0.45	-0.483
0.9	-0.844
1.35	-1.057
1.8	-1.111
2.25	-1.014
2.7	-0.772
3.15	-0.423
3.6	0

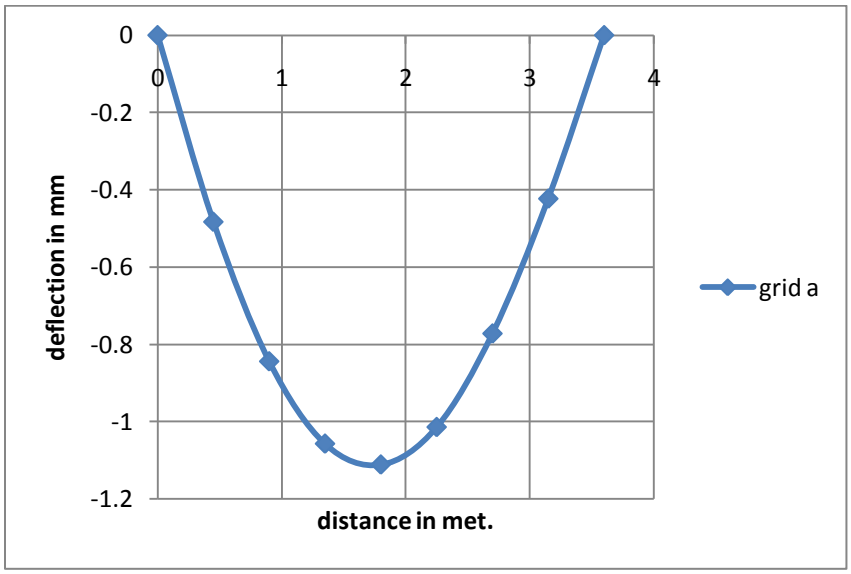

Fig.36 Deflection at different points along grid a (room1)

Table- 26 Deflections along grid b

Distance in met.	Deflection in mm,grid b
0	0
0.45	-0.508
0.9	-0.869
1.35	-1.047
1.8	-1.077
2.25	-1.017
2.7	-0.874
3.15	-0.67
3.6	-0.466
4.05	-0.321
4.5	-0.219
4.95	-0.122
5.4	0

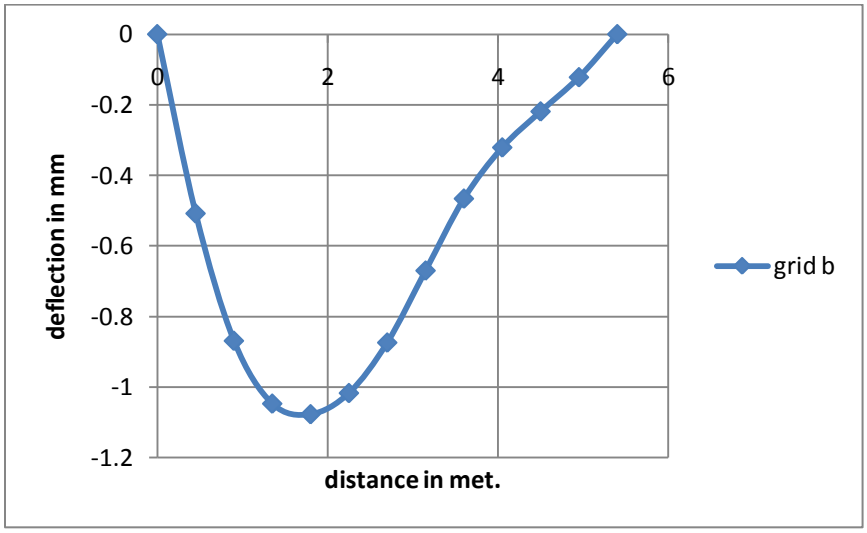

Fig.37 Deflection at different points along grid b (room 1)

5.3 DISSCUSSIONS

Both the type of models were analysed and found that the deflections of the two cases with opening and without opening does not vary much .and so it is concluded that the opening in the common wall does not affect the stability of the structure considerably.

As seen in the deflection diagrams, the deflections in both the slabs are approximately same. The deflection varies from 0.08 mm to 1.263 mm. The opening created in the common RCC wall is not able to make any remarkable effect in the results. The comparision is made for the combination of dead load and live load in both the cases.

The housing is casted with the M15 grade of concrete to maintain the economy which is against the codal provisions mentioned in IS: 456, according to which the minimum grade of concrete to be adopted is M20(table 5 ,pg no.20). As per code, the grade M15 can be used for temporary R.C.C. construction. With M15 grade it may have passed the basic requirements like limiting deflections, strength etc. but it can adversely affect the durability aspect of the building. A durable concrete is one that performs satisfactorily in the working environment during its anticipated exposure conditions during its service. One of the main characteristics influencing the durability of concrete is its permeability in to the ingress of water, oxygen, carbondioxide, chloride, sulphates, etc. impermeability is governed by the constituents and workmanship used in making the concrete. In this case, the permeability may be quite low as the aggregates used is of small size, and the during the casting the vibrators were used, so proper compaction may have achieved and thus the building may perfom better.

CHAPTER 6

COMPARISION OF THE EXPERIMENTAL AND NUMERICAL METHOD OF LOAD TEST.

The results of the two methods were studied, compared and discussed ahead. The comparative results are plotted graphically to study the difference, with deflections on vertical axis and distance from the origin on the horizontal axis. An equation between experimental results and numerical analysis results is also tried to obtained.

6.1 Comparision of the results of both the tests results

Table- 28 Deflections along grid 1

Distance in met.	Deflection as per field test,mm	Deflection as per Staad analysis,mm
0	0	0
1	-5	-0.844
2	-5	-0.869
3	-2	-0.35
4	0	0

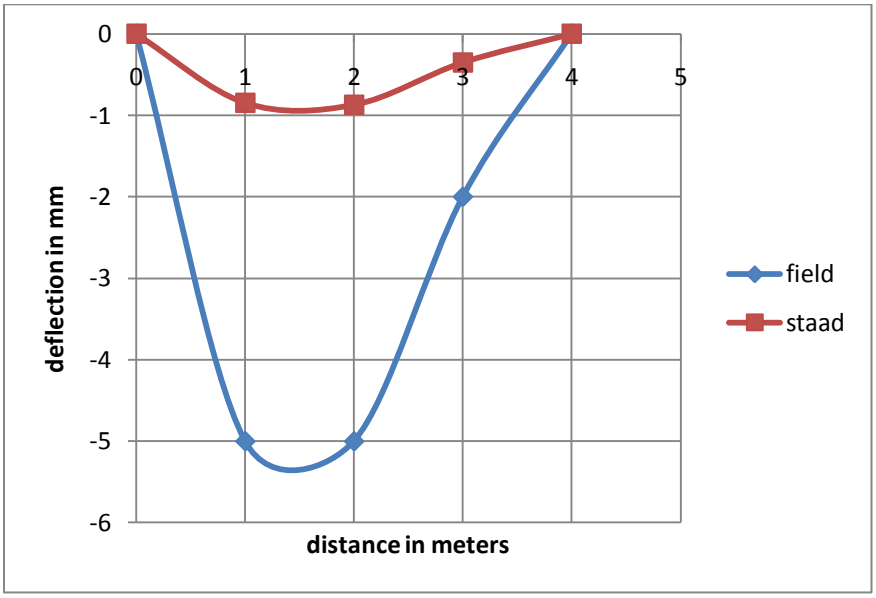

Fig.38 Deflection at different points along grid 1

Table- 29 Deflections along grid a

Distance in met.	Deflection as per field test,mm	Deflection as per Staad analysis,mm
0	0	0
0.9	-5	-0.844
1.8	-5	-1.111
2.7	-4	-0.772
3.6	0	0

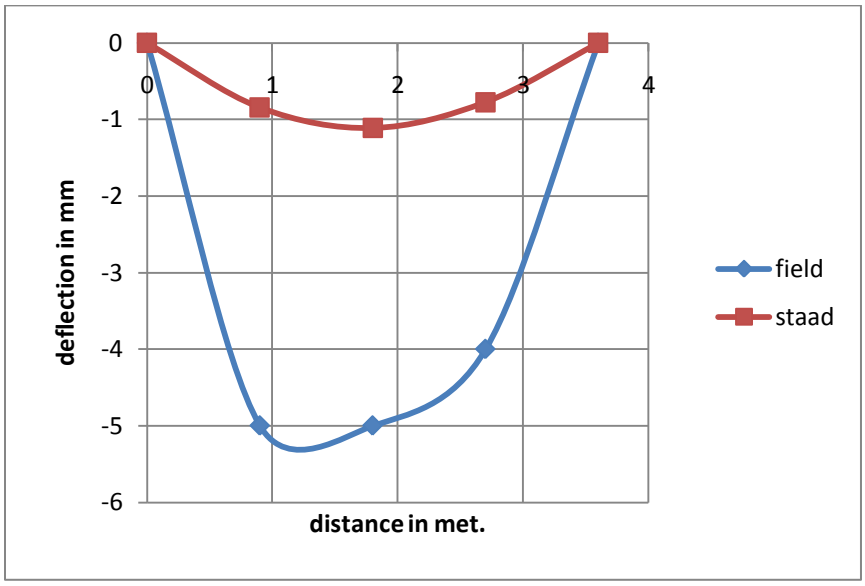

Fig.39 Deflection at different points along grid a

Table- 30 Deflections along grid b

Distance in met.	Deflection in mm,field test	Deflection in mm,staad results
0	0	0
0.9	-5	-0.869
1.8	-3	-1.077
2.7	-2	-0.874
3.6	-1	-0.466
4.5	-2	-0.219
5.4	0	0

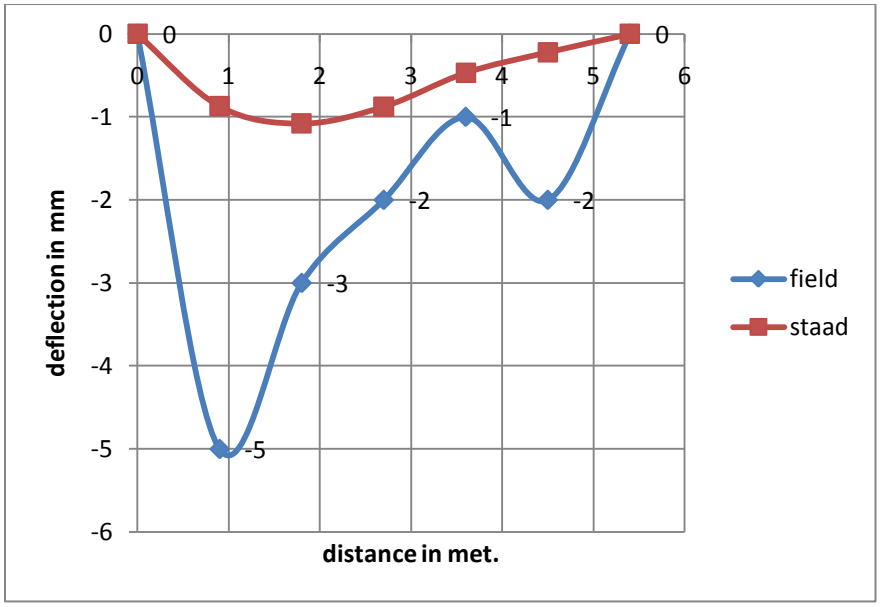

Fig.40 Deflection at different points along grid b

6.2 Finding a compatible equation between the two results

Table- 31 Consolidated Deflections along grids

Grid marking	Deflection in mm (field test)	Deflection in mm (staad results)
1	5	0.966
2	5	1.263
3	4	0.67
4	1	0.466
5	2	0.219
a	5	1.111
b	5	1.077
c	2	0.349

y=0.210x+0.003
here, y = deflection by staad results
and x = deflections by field test
$R^2 = 0.817$

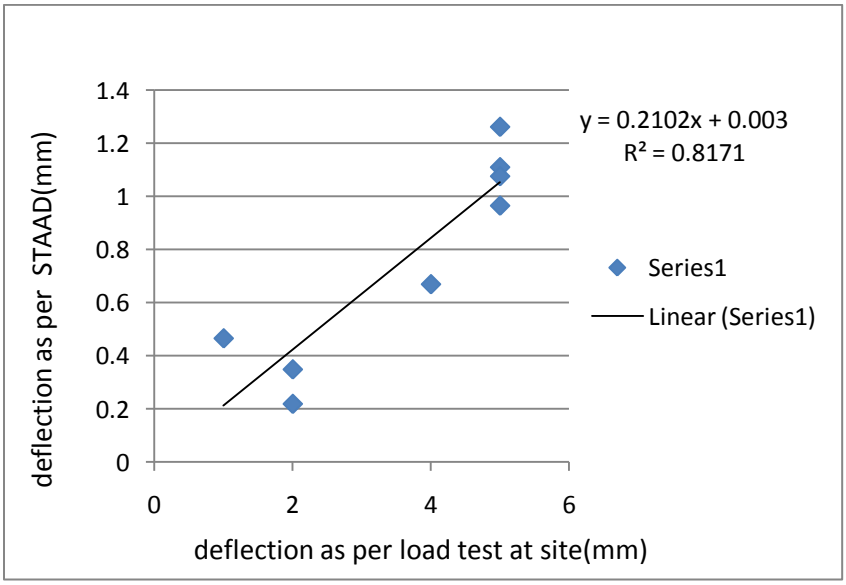

Fig.41 Deflection obtained by field test Vs STAAD results

CHAPTER-7

CONCLUSIONS

1) Load test has been performed on the slab at the site situated at Bawana with the application of 1.25 times the live load and it was found that the deflections in the slab were in the range of 1mm to 5 mm and were within the limits as mentioned in IS:456.the rebound of the slab was also noticed and it was upto 3 mm.

2) The two types of models has been considered for numerical analysis using STAAD. The two models were with opening and without opening in the common RCC wall of two units of the housing society. The deflections in the slab ranges from 1 mm to 1.3mm in both the cases. This implies that the opening in the wall is not affecting the slab as the size of the opening is very small compared to the overall stiffness of the unit.

3) The results of the testings, experimental and numerical were compared and it was found that there was a considerable difference in the deflections of the slab obtained. The maximum deflection found experimentally is 5mm while it is 1.2mm in the case of numerical analysis.

The reason may be the difference in the quality of material used in the field with that of numerical analysis. In numerical case, the software is adopting the ideal case of the quality of material and the workmanship. But in actual the quality and quantity of material used may vary from the ideal conditions. The other factor may be poor workmanship in the field which leads to higher deflections in the slab. Although the slab of the housing unit is casted monolithically with the R.C.C. walls of the unit, still it can may happen that during laying of the reinforcement, proper length of anchorage and development length is not taken care. As a result of which there may be partial fixitiy between the joint of slab and wall resulting in higher deflections.

4) A compatibility equation is also generated between the deflections obtained from experimental test and numerical analysis.

$y=0.210x+0.003$
here, y = deflection by staad results
and x = deflections by field test
$R^2 = 0.817$

This is a linear equation with a reliability of 81 percent as the correlation factor (R^2) is 0.817. The positive correlation factor above 0.8 is considered as strong. Now, if the result of one test is

present then the other test results can be obtained by using this equation. This value depends upon the data analysed. The validity of this equation is subject to the conditions adopted during the load test. If the site conditions are maintained approximately the same then the above relation is valid. Diversions from the actual conditions may limit the use of the compatibility equation.

REFERENCES

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25. STAADPRO V8i

ANNEXURE A

STAAD INPUT FILE- WITHOUT OPENING

STAAD SPACE
START JOB INFORMATION
ENGINEER DATE 30-Oct-10
END JOB INFORMATION
INPUT WIDTH 79
UNIT MMS NEWTON
JOINT COORDINATES
813 575 2900 3900; 814 0 2900 3900; 815 1150 2900 3900; 816 1725 2900 3900;
817 2300 2900 3900; 818 3650 2900 3900; 819 3100 2900 3900; 820 4200 2900 3900;
1495 7825 2900 3900; 1496 8400 2900 3900; 1497 7250 2900 3900;
1498 6675 2900 3900; 1499 6100 2900 3900; 1500 4750 2900 3900;
1501 5300 2900 3900; 1516 575 400 3900; 1517 0 400 3900; 1518 1150 400 3900;
1519 1725 400 3900; 1520 2300 400 3900; 1521 3100 400 3900; 1522 3650 400 3900;
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1599 0 800 1433.33; 1600 0 800 1844.44; 1601 0 800 2255.56; 1602 0 800 2666.67;
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1614 1150 1200 3900; 1615 1725 1200 3900; 1616 2300 1200 3900;
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1745 0 2000 2255.56; 1746 0 2000 2666.67; 1747 0 2000 3077.78;
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2001 1100 -1530 -500; 2002 1100 -1530 -200; 2003 1100 -1530 200;
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2011 0 -1530 2255.56; 2012 0 -1530 2666.67; 2013 0 -1530 3077.78;
2014 0 -1530 3488.89; 2015 3100 -1530 -400; 2016 3100 -1530 100;
2017 3100 -1530 2000; 2018 3466.67 -1530 2000; 2019 3833.33 -1530 2000;
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9850 1719 1718 2151 2152;
DEFINE MATERIAL START
ISOTROPIC CONCRETE
E 19364.9
POISSON 0.17
DENSITY 2.35616e-005
ALPHA 1e-005
DAMP 0.05
END DEFINE MATERIAL
CONSTANTS
MATERIAL CONCRETE ALL
ELEMENT PROPERTY
1697 TO 1703 1706 TO 1749 1752 TO 1784 1789 TO 1795 1798 TO 1836 1839 TO 1870 -
1883 TO 1889 1892 TO 1928 2269 TO 2275 2277 TO 2309 2314 TO 2354 -
2589 TO 2656 2661 TO 2722 2735 TO 2770 3051 TO 3081 3086 TO 3116 -
4441 TO 4447 4450 TO 4493 4496 TO 4528 4533 TO 4539 4542 TO 4580 -
4583 TO 4614 4627 TO 4633 4636 TO 4672 4677 TO 4717 4722 TO 4762 -

4997 TO 5064 5069 TO 5130 5143 TO 5178 5183 TO 5213 5218 TO 5248 -
6573 TO 6579 6582 TO 6625 6628 TO 6660 6665 TO 6671 6674 TO 6712 -
6715 TO 6746 6759 TO 6765 6768 TO 6804 6809 TO 6849 6854 TO 6894 -
7129 TO 7196 7201 TO 7262 7275 TO 7310 7315 TO 7345 7350 TO 7380 -
8705 TO 8711 8714 TO 8757 8760 TO 8792 8797 TO 8803 8806 TO 8844 -
8847 TO 8878 8891 TO 8897 8900 TO 8936 8941 TO 8981 8986 TO 9026 -
9261 TO 9328 9333 TO 9394 9407 TO 9442 9447 TO 9477 9482 TO 9512 -
9800 TO 9822 9825 TO 9847 9850 THICKNESS 100
1785 TO 1788 1871 TO 1874 1929 1930 2310 2311 2355 2356 2657 TO 2660 2723 -
2724 TO 2726 2771 2772 3082 3083 3117 3118 4529 TO 4532 4615 TO 4618 4673 -
4674 4718 4719 4763 4764 5065 TO 5068 5131 TO 5134 5179 5180 5214 5215 5249 -
5250 6661 TO 6664 6747 TO 6750 6805 6806 6850 6851 6895 6896 7197 TO 7200 -
7263 TO 7266 7311 7312 7346 7347 7381 7382 8793 TO 8796 8879 TO 8882 8937 -
8938 8982 8983 9027 9028 9329 TO 9332 9395 TO 9398 9443 9444 9478 9479 9513 -
9514 THICKNESS 100
1933 TO 2244 2775 TO 3026 THICKNESS 150
2245 TO 2254 2257 TO 2266 3027 TO 3036 3039 TO 3048 THICKNESS 150
2255 2256 2267 2268 3037 3038 3049 3050 THICKNESS 150
1875 TO 1882 1931 1932 2312 2313 2357 2358 2727 TO 2734 2773 2774 3084 3085 -
3119 3120 4619 TO 4626 4675 4676 4720 4721 4765 4766 5135 TO 5142 5181 5182 -
5216 5217 5251 5252 6751 TO 6758 6807 6808 6852 6853 6897 6898 7267 TO 7274 -
7313 7314 7348 7349 7383 7384 8883 TO 8890 8939 8940 8984 8985 9029 9030 -
9399 TO 9406 9445 9446 9480 9481 9515 9516 9823 9824 9848 -
9849 THICKNESS 100
2359 TO 2366 3121 TO 3128 4767 TO 4774 5253 TO 5260 6899 TO 6906 7385 TO 7392 -
9031 TO 9038 9517 TO 9524 THICKNESS 100
2369 TO 2372 3131 TO 3134 4777 TO 4780 5263 TO 5266 6909 TO 6912 7395 TO 7398 -
9041 TO 9044 9527 TO 9530 THICKNESS 100
2373 TO 2376 3135 TO 3138 4781 TO 4784 5267 TO 5270 6913 TO 6916 7399 TO 7402 -
9045 TO 9048 9531 TO 9534 THICKNESS 100
2367 2368 3129 3130 4775 4776 5261 5262 6907 6908 7393 7394 9039 9040 9525 -
9526 THICKNESS 100
2377 TO 2588 3139 TO 3350 4785 TO 4996 5271 TO 5482 6917 TO 7128 7403 TO 7614 -
9049 TO 9260 9535 TO 9746 THICKNESS 100
UNIT METER KN
SUPPORTS
2077 TO 2129 2136 2760 TO 2801 2808 FIXED
DEFINE 1893 LOAD
ZONE 0.24 RF 5 I 1 SS 2 ST 1 DM 0.05 DT 2.5
SELFWEIGHT
ELEMENT WEIGHT
2377 TO 2588 5414 TO 5420 5435 TO 5443 5445 TO 5482 7034 TO 7040 7055 TO 7063 -
7065 TO 7111 7113 TO 7128 7403 TO 7614 9049 TO 9260 9535 TO 9745 -
9746 PRESSURE 1
***** LIVE LOAD 25%
2377 TO 2588 5414 TO 5420 5435 TO 5443 5445 TO 5482 7034 TO 7040 7055 TO 7063 -

```
7065 TO 7111 7113 TO 7128 7403 TO 7614 9049 TO 9260 9535 TO 9745 -
9746 PRESSURE 0.5
LOAD 1
1893 LOAD X
LOAD 2
1893 LOAD z
LOAD 5 DEAD LOAD
SELFWEIGHT Y -1
ELEMENT LOAD
2377 TO 2588 3139 TO 3350 4785 TO 4996 5271 TO 5482 6917 TO 7128 7403 TO 7614 -
9049 TO 9260 9535 TO 9746 PR GY -1
LOAD 6 LIVE LOAD
ELEMENT LOAD
2377 TO 2588 3139 TO 3350 4785 TO 4996 5271 TO 5482 6917 TO 7128 7403 TO 7614 -
9049 TO 9260 9535 TO 9746 PR GY -2
LOAD COMB 7 LIMIT LOADS
5 1.5 6 1.5
LOAD COMB 8 1.2 DL + 1.2 LL + 1.2EQX
5 1.2 1 1.2 6 1.2
LOAD COMB 9 1.2 DL + 1.2 LL - 1.2EQX
5 1.2 1 -1.2 6 1.2
LOAD COMB 10 1.2 DL + 1.2 LL + 1.2EQZ
5 1.2 2 1.2 6 1.2
LOAD COMB 11 1.2 DL + 1.2 LL - 1.2EQZ
5 1.2 2 -1.2 6 1.2
LOAD COMB 12 1.5 DL + 1.5 EQX
5 1.5 1 1.5
LOAD COMB 13 1.5 DL - 1.5 EQX
5 1.5 1 -1.5
LOAD COMB 14 1.5 DL + 1.5 EQZ
5 1.5 2 1.5
LOAD COMB 15 1.5 DL - 1.5 EQZ
5 1.5 2 -1.5
*****
LOAD COMB 16 0.9 DL + 1.5 EQX
5 0.9 1 1.5
LOAD COMB 17 0.9 DL - 1.5 EQX
5 0.9 1 -1.5
LOAD COMB 18 0.9 DL + 1.5 EQZ
5 0.9 2 1.5
LOAD COMB 19 0.9 DL - 1.5 EQZ
5 0.9 2 -1.5
PERFORM ANALYSIS
FINISH
```

ANNEXURE-B

STAAD OUTPUT

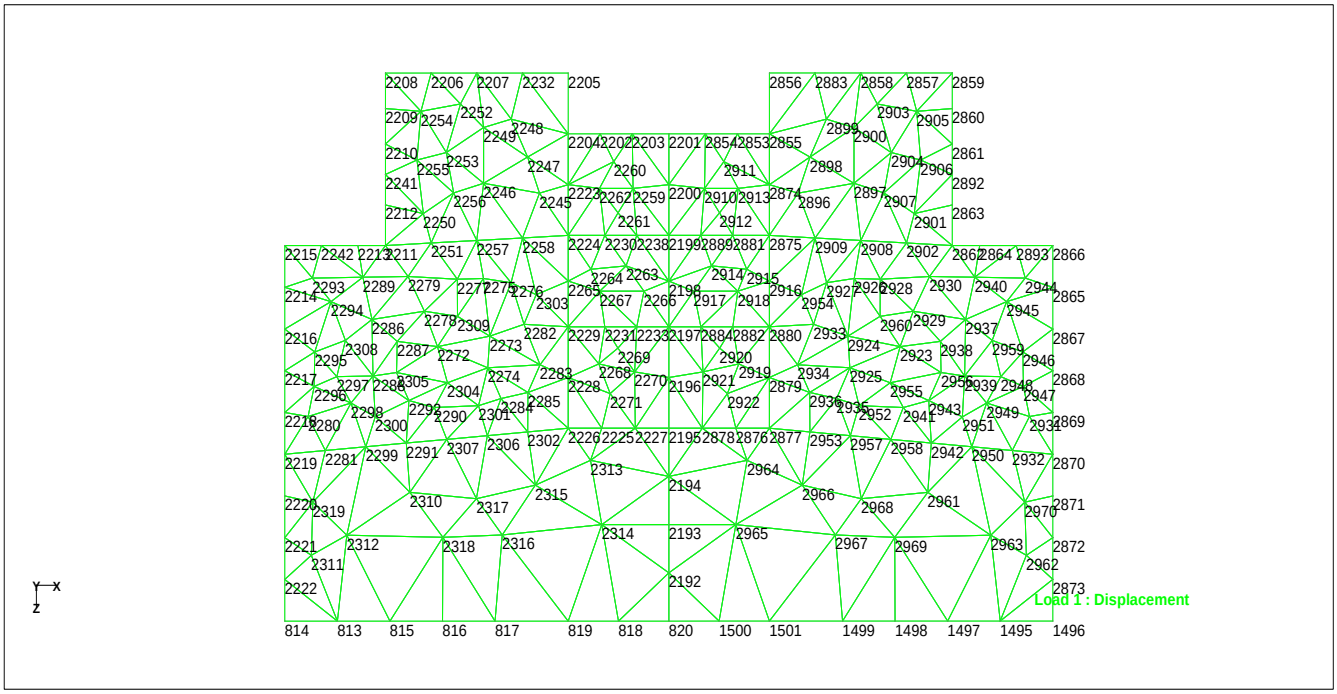

Fig.42 Showing node numbers of a single floor

NODE DISPLACEMENTS-with opening in the RCC wall

Node	L/C	X-Trans mm	Y-Trans mm	Z-Trans mm	Absolute mm	X-Rotan rad	Y-Rotan rad	Z-Rotan rad
813	7	-0.008	-0.175	0.025	0.177	0	0	0
814	7	-0.01	-0.171	0.029	0.174	0	0	0
815	7	-0.006	-0.177	0.024	0.179	0	0	0
816	7	-0.004	-0.179	0.022	0.18	0	0	0
817	7	-0.002	-0.183	0.021	0.184	-0.001	0	0
818	7	-0.003	-0.172	0.022	0.174	0	0	0
819	7	-0.005	-0.175	0.021	0.176	0	0	0
820	7	0	-0.172	0.023	0.173	0	0	0
1495	7	0.008	-0.175	0.025	0.177	0	0	0
1496	7	0.01	-0.171	0.029	0.174	0	0	0
1497	7	0.006	-0.177	0.024	0.179	0	0	0
1498	7	0.004	-0.179	0.022	0.18	0	0	0

1499	7	0.002	-0.183	0.021	0.184	-0.001	0	0
1500	7	0.003	-0.172	0.022	0.174	0	0	0
1501	7	0.005	-0.175	0.021	0.176	0	0	0
2192	7	0	-0.172	0.022	0.174	0	0	0
2193	7	0	-0.175	0.022	0.176	0	0	0
2194	7	0	-0.187	0.022	0.188	0	0	0
2195	7	0	-0.188	0.022	0.189	0	0	0
2196	7	0	-0.183	0.02	0.184	0	0	0
2197	7	0	-0.173	0.019	0.174	0	0	0
2198	7	0	-0.172	0.018	0.173	0	0	0
2199	7	0	-0.162	0.016	0.163	0	0	0
2200	7	0	-0.162	0.014	0.162	0	0	0
2201	7	0	-0.157	0.013	0.157	0	0	0
2202	7	-0.002	-0.156	0.016	0.157	0	0	0
2203	7	-0.001	-0.156	0.015	0.157	0	0	0
2204	7	-0.002	-0.156	0.015	0.157	0	0	0
2205	7	0.001	-0.148	0.014	0.149	0	0	0
2206	7	-0.003	-0.15	0.015	0.15	0	0	0
2207	7	-0.002	-0.15	0.015	0.151	0	0	0
2208	7	-0.004	-0.147	0.014	0.148	0	0	0
2209	7	-0.003	-0.155	0.015	0.155	0	0	0
2210	7	-0.003	-0.161	0.015	0.162	0	0	0
2211	7	-0.004	-0.195	0.017	0.196	0	0	0
2212	7	-0.003	-0.176	0.015	0.177	0	0	0
2213	7	-0.005	-0.177	0.018	0.178	0	0	0
2214	7	-0.005	-0.151	0.016	0.152	0	0	0
2215	7	-0.004	-0.147	0.015	0.148	0	0	0
2216	7	-0.005	-0.155	0.018	0.157	0	0	0
2217	7	-0.006	-0.159	0.02	0.16	0	0	0
2218	7	-0.006	-0.163	0.022	0.164	0	0	-0.001
2219	7	-0.006	-0.165	0.024	0.167	0	0	-0.001
2220	7	-0.006	-0.168	0.025	0.17	0	0	0
2221	7	-0.007	-0.169	0.026	0.171	0	0	0
2222	7	-0.008	-0.171	0.027	0.173	0	0	0
2223	7	-0.002	-0.163	0.017	0.164	0	0	0
2224	7	-0.003	-0.172	0.018	0.173	0	0	0
2225	7	-0.003	-0.192	0.021	0.193	0	0	0
2226	7	-0.004	-0.204	0.021	0.206	0	0	0
2227	7	-0.001	-0.189	0.021	0.19	0	0	0

2228	7	-0.003	-0.193	0.02	0.194	0	0	0
2229	7	-0.004	-0.191	0.018	0.192	0	0	0
2230	7	-0.002	-0.161	0.017	0.162	0	0	0
2231	7	-0.003	-0.176	0.019	0.177	0	0	0
2232	7	-0.001	-0.15	0.015	0.15	0	0	0
2233	7	-0.001	-0.167	0.019	0.169	0	0	0
2238	7	-0.001	-0.156	0.017	0.157	0	0	0
2241	7	-0.003	-0.168	0.015	0.168	0	0	0
2242	7	-0.004	-0.162	0.018	0.163	0	0	0
2245	7	-0.002	-0.243	0.017	0.244	0	0	0
2246	7	-0.003	-0.4	0.016	0.401	0	0	0
2247	7	-0.002	-0.253	0.016	0.253	0	0	0
2248	7	-0.002	-0.231	0.015	0.231	0	0	0
2249	7	-0.002	-0.275	0.016	0.275	0	0	0
2250	7	-0.003	-0.335	0.016	0.335	0	0	0
2251	7	-0.004	-0.445	0.017	0.445	0	0	0
2252	7	-0.002	-0.216	0.015	0.216	0	0	0
2253	7	-0.003	-0.31	0.016	0.311	0	0	0
2254	7	-0.003	-0.197	0.015	0.198	0	0	0
2255	7	-0.003	-0.246	0.015	0.247	0	0	0
2256	7	-0.003	-0.404	0.016	0.404	0	0	0
2257	7	-0.003	-0.541	0.017	0.542	0	0	0
2258	7	-0.003	-0.382	0.018	0.382	0	0	0
2259	7	-0.001	-0.165	0.016	0.166	0	0	0
2260	7	-0.001	-0.163	0.016	0.164	0	0	0
2261	7	-0.001	-0.16	0.017	0.161	0	0	0
2262	7	-0.002	-0.163	0.017	0.163	0	0	0
2263	7	-0.001	-0.168	0.018	0.169	0	0	0
2264	7	-0.002	-0.184	0.018	0.185	0	0	0
2265	7	-0.003	-0.227	0.018	0.228	0	0	0
2266	7	-0.001	-0.172	0.018	0.172	0	0	0
2267	7	-0.002	-0.182	0.018	0.183	0	0	0
2268	7	-0.002	-0.178	0.019	0.179	0	0	0
2269	7	-0.002	-0.175	0.019	0.177	0	0	0
2270	7	-0.001	-0.184	0.02	0.185	0	0	0
2271	7	-0.002	-0.184	0.02	0.185	0	0	0
2272	7	-0.005	-1.07	0.019	1.07	0.001	0	0
2273	7	-0.004	-0.805	0.019	0.805	0	0	0.001
2274	7	-0.004	-0.914	0.019	0.914	0	0	0.001

2275	7	-0.004	-0.65	0.018	0.65	0	0	0
2276	7	-0.004	-0.532	0.018	0.532	0	0	0.001
2277	7	-0.004	-0.689	0.018	0.689	0	0	0
2278	7	-0.005	-0.827	0.019	0.827	0.001	0	0
2279	7	-0.004	-0.495	0.018	0.495	0.001	0	0
2280	7	-0.006	-0.362	0.022	0.363	0	0	-0.001
2281	7	-0.006	-0.557	0.023	0.558	0	0	-0.001
2282	7	-0.004	-0.491	0.019	0.491	0	0	0.001
2283	7	-0.004	-0.374	0.02	0.374	0	0	0.001
2284	7	-0.004	-0.848	0.02	0.848	0	0	0.001
2285	7	-0.004	-0.528	0.02	0.528	0	0	0.001
2286	7	-0.005	-0.648	0.019	0.648	0.001	0	-0.001
2287	7	-0.005	-0.938	0.019	0.939	0.001	0	0
2288	7	-0.005	-0.969	0.02	0.969	0	0	-0.001
2289	7	-0.005	-0.313	0.018	0.313	0.001	0	0
2290	7	-0.005	-1.31	0.02	1.31	0	0	0
2291	7	-0.005	-1.301	0.021	1.302	0	0	0
2292	7	-0.005	-1.263	0.02	1.263	0	0	0
2293	7	-0.005	-0.203	0.018	0.204	0	0	0
2294	7	-0.005	-0.333	0.018	0.333	0	0	0
2295	7	-0.006	-0.364	0.02	0.364	0	0	-0.001
2296	7	-0.006	-0.407	0.021	0.407	0	0	-0.001
2297	7	-0.006	-0.626	0.02	0.627	0	0	-0.001
2298	7	-0.006	-0.834	0.021	0.835	0	0	-0.001
2299	7	-0.006	-1.012	0.022	1.013	0	0	-0.001
2300	7	-0.006	-1.093	0.021	1.093	0	0	-0.001
2301	7	-0.004	-1.084	0.02	1.084	0	0	0.001
2302	7	-0.004	-0.591	0.02	0.592	0	0	0.001
2303	7	-0.003	-0.384	0.018	0.384	0	0	0.001
2304	7	-0.005	-1.21	0.02	1.211	0	0	0
2305	7	-0.005	-1.108	0.02	1.108	0	0	0
2306	7	-0.004	-1.052	0.02	1.052	0	0	0.001
2307	7	-0.005	-1.319	0.021	1.32	0	0	0
2308	7	-0.005	-0.586	0.019	0.586	0	0	-0.001
2309	7	-0.004	-0.88	0.019	0.88	0.001	0	0
2310	7	-0.006	-1.187	0.022	1.187	0	0	0
2311	7	-0.007	-0.282	0.026	0.284	0	0	0
2312	7	-0.007	-0.581	0.024	0.581	0	0	-0.001
2313	7	-0.002	-0.262	0.021	0.263	0	0	0

2314	7	-0.002	-0.369	0.021	0.369	0	0	0
2315	7	-0.003	-0.673	0.021	0.673	0	0	0.001
2316	7	-0.004	-0.834	0.021	0.834	-0.001	0	0
2317	7	-0.005	-1.105	0.021	1.105	0	0	0
2318	7	-0.005	-0.956	0.022	0.957	-0.001	0	0
2319	7	-0.006	-0.38	0.024	0.381	0	0	-0.001
2853	7	0.002	-0.156	0.016	0.157	0	0	0
2854	7	0.001	-0.156	0.015	0.157	0	0	0
2855	7	0.002	-0.156	0.015	0.157	0	0	0
2856	7	-0.001	-0.148	0.014	0.149	0	0	0
2857	7	0.003	-0.15	0.015	0.15	0	0	0
2858	7	0.002	-0.15	0.015	0.151	0	0	0
2859	7	0.004	-0.147	0.014	0.148	0	0	0
2860	7	0.003	-0.155	0.015	0.155	0	0	0
2861	7	0.003	-0.161	0.015	0.162	0	0	0
2862	7	0.004	-0.195	0.017	0.196	0	0	0
2863	7	0.003	-0.176	0.015	0.177	0	0	0
2864	7	0.005	-0.177	0.018	0.178	0	0	0
2865	7	0.005	-0.151	0.016	0.152	0	0	0
2866	7	0.004	-0.147	0.015	0.148	0	0	0
2867	7	0.005	-0.155	0.018	0.157	0	0	0
2868	7	0.006	-0.159	0.02	0.16	0	0	0
2869	7	0.006	-0.163	0.022	0.164	0	0	0.001
2870	7	0.006	-0.165	0.024	0.167	0	0	0.001
2871	7	0.006	-0.168	0.025	0.17	0	0	0
2872	7	0.007	-0.169	0.026	0.171	0	0	0
2873	7	0.008	-0.171	0.027	0.173	0	0	0
2874	7	0.002	-0.163	0.017	0.164	0	0	0
2875	7	0.003	-0.172	0.018	0.173	0	0	0
2876	7	0.003	-0.192	0.021	0.193	0	0	0
2877	7	0.004	-0.204	0.021	0.206	0	0	0
2878	7	0.001	-0.189	0.021	0.19	0	0	0
2879	7	0.003	-0.193	0.02	0.194	0	0	0
2880	7	0.004	-0.191	0.018	0.192	0	0	0
2881	7	0.002	-0.161	0.017	0.162	0	0	0
2882	7	0.003	-0.176	0.019	0.177	0	0	0
2883	7	0.001	-0.15	0.015	0.15	0	0	0
2884	7	0.001	-0.167	0.019	0.169	0	0	0
2889	7	0.001	-0.156	0.017	0.157	0	0	0

2892	7	0.003	-0.168	0.015	0.168	0	0	0
2893	7	0.004	-0.162	0.018	0.163	0	0	0
2896	7	0.002	-0.243	0.017	0.244	0	0	0
2897	7	0.003	-0.4	0.016	0.401	0	0	0
2898	7	0.002	-0.253	0.016	0.253	0	0	0
2899	7	0.002	-0.231	0.015	0.231	0	0	0
2900	7	0.002	-0.275	0.016	0.275	0	0	0
2901	7	0.003	-0.335	0.016	0.335	0	0	0
2902	7	0.004	-0.445	0.017	0.445	0	0	0
2903	7	0.002	-0.216	0.015	0.216	0	0	0
2904	7	0.003	-0.31	0.016	0.311	0	0	0
2905	7	0.003	-0.197	0.015	0.198	0	0	0
2906	7	0.003	-0.246	0.015	0.247	0	0	0
2907	7	0.003	-0.404	0.016	0.404	0	0	0
2908	7	0.003	-0.541	0.017	0.542	0	0	0
2909	7	0.003	-0.382	0.018	0.382	0	0	0
2910	7	0.001	-0.165	0.016	0.166	0	0	0
2911	7	0.001	-0.163	0.016	0.164	0	0	0
2912	7	0.001	-0.16	0.017	0.161	0	0	0
2913	7	0.002	-0.163	0.017	0.163	0	0	0
2914	7	0.001	-0.168	0.018	0.169	0	0	0
2915	7	0.002	-0.184	0.018	0.185	0	0	0
2916	7	0.003	-0.227	0.018	0.228	0	0	0
2917	7	0.001	-0.172	0.018	0.172	0	0	0
2918	7	0.002	-0.182	0.018	0.183	0	0	0
2919	7	0.002	-0.178	0.019	0.179	0	0	0
2920	7	0.002	-0.175	0.019	0.177	0	0	0
2921	7	0.001	-0.184	0.02	0.185	0	0	0
2922	7	0.002	-0.184	0.02	0.185	0	0	0
2923	7	0.005	-1.07	0.019	1.07	0.001	0	0
2924	7	0.004	-0.805	0.019	0.805	0	0	-0.001
2925	7	0.004	-0.914	0.019	0.914	0	0	-0.001
2926	7	0.004	-0.65	0.018	0.65	0	0	0
2927	7	0.004	-0.532	0.018	0.532	0	0	-0.001
2928	7	0.004	-0.689	0.018	0.689	0	0	0
2929	7	0.005	-0.827	0.019	0.827	0.001	0	0
2930	7	0.004	-0.495	0.018	0.495	0.001	0	0
2931	7	0.006	-0.362	0.022	0.363	0	0	0.001
2932	7	0.006	-0.557	0.023	0.558	0	0	0.001

2933	7	0.004	-0.491	0.019	0.491	0	0	-0.001
2934	7	0.004	-0.374	0.02	0.374	0	0	-0.001
2935	7	0.004	-0.848	0.02	0.848	0	0	-0.001
2936	7	0.004	-0.528	0.02	0.528	0	0	-0.001
2937	7	0.005	-0.648	0.019	0.648	0.001	0	0.001
2938	7	0.005	-0.938	0.019	0.939	0.001	0	0
2939	7	0.005	-0.969	0.02	0.969	0	0	0.001
2940	7	0.005	-0.313	0.018	0.313	0.001	0	0
2941	7	0.005	-1.31	0.02	1.31	0	0	0
2942	7	0.005	-1.301	0.021	1.302	0	0	0
2943	7	0.005	-1.263	0.02	1.263	0	0	0
2944	7	0.005	-0.203	0.018	0.204	0	0	0
2945	7	0.005	-0.333	0.018	0.333	0	0	0
2946	7	0.006	-0.364	0.02	0.364	0	0	0.001
2947	7	0.006	-0.407	0.021	0.407	0	0	0.001
2948	7	0.006	-0.626	0.02	0.627	0	0	0.001
2949	7	0.006	-0.834	0.021	0.835	0	0	0.001
2950	7	0.006	-1.012	0.022	1.013	0	0	0.001
2951	7	0.006	-1.093	0.021	1.093	0	0	0.001
2952	7	0.004	-1.084	0.02	1.084	0	0	-0.001
2953	7	0.004	-0.591	0.02	0.592	0	0	-0.001
2954	7	0.003	-0.384	0.018	0.384	0	0	-0.001
2955	7	0.005	-1.21	0.02	1.211	0	0	0
2956	7	0.005	-1.108	0.02	1.108	0	0	0
2957	7	0.004	-1.052	0.02	1.052	0	0	-0.001
2958	7	0.005	-1.319	0.021	1.32	0	0	0
2959	7	0.005	-0.586	0.019	0.586	0	0	0.001
2960	7	0.004	-0.88	0.019	0.88	0.001	0	0
2961	7	0.006	-1.187	0.022	1.187	0	0	0
2962	7	0.007	-0.282	0.026	0.284	0	0	0
2963	7	0.007	-0.581	0.024	0.581	0	0	0.001
2964	7	0.002	-0.262	0.021	0.263	0	0	0
2965	7	0.002	-0.369	0.021	0.369	0	0	0
2966	7	0.003	-0.673	0.021	0.673	0	0	-0.001
2967	7	0.004	-0.834	0.021	0.834	-0.001	0	0
2968	7	0.005	-1.105	0.021	1.105	0	0	0
2969	7	0.005	-0.956	0.022	0.957	-0.001	0	0
2970	7	0.006	-0.38	0.024	0.381	0	0	0.001