# ADDITION OF SCRAP TYRE CRUMBS <br> TO PAVEMENT CONCRETE 

A Dissertation Submitted in a Partial Fulfillment for the requirement for Award of Degree of

## MASTER OF ENGINEERING

In
STRUCTURAL ENGINEERING

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## CANDIDATE'S DECLARATION

## \&

CERTIFICATE

This is to certify that the Major Project Report on topic "ADDITION OF SCRAP TYRE CRUMBS TO PAVEMENT CONCRETE" is a bonafide record of work done by me (DEEPAK RAI) for the fulfillment of the degree of "Master of Engineering (Structures)" from DELHI COLLEGE OF ENGINEERING, Delhi. I have not submitted the matter embodied in the dissertation for the award of any other degree.

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## CERTIFICATE

This is to certify that above statement made by DEEPAK RAI is correct to the best of my knowledge.

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## INTRODUCTION

Scrap tyres are about to become the latest headache for the environment in view of disposal problems. Burning tyres in the open emits significant quantities of hazardous air pollutants including particles, dioxins, carbon monoxide, polyaromatic hydrocarbons and volatile organic compounds. Although a one off tyre fire is temporary but the quantities of contaminants released and their concentration can be significant such as dioxins may bioaccumulate and cause contamination problems over time. The remaining ash residue also contains hazardous contaminants potential creating a contaminated site that requires remediation.

The burning of the waste material such as tyres in the open is typically a prohibited or non-complying activity within air quality plans. As with any fire, tyre dump fire poses a threat to life (or at least of injury) for the general public and more particularly to Fire Fighters. In addition there is a risk of the fire spreading and causing damage to property or natural areas. Burning of tyres result in loss of amenity through visual impacts as a result of the heavy dark smoke and extremely unpleasant odors. There is also the potential for pollutants from tyre fires to enters waterways and contaminate land. The water used to extinguish fire can transport the combustion products (including hydrocarbon and carbon black) from the fire to the water lands or other water.

Smoke from the tyres fire is particularly dark and dense; it has the potential to cause short-term acute health effects if inhaled (eye, nose, and throat irritation, asthma attacks, and respiratory difficulties) and significant odour nuisance. Toxic fumes like Carbon Monoxide and oxides of Nitrogen on account of burning tyres create respiratory problems, dizziness, metabolic disorders and Cancer. Smoke fumes can also reduce visibility on roads leading to unsafe condition.

Suspended particulate matter forms Smog in humid conditions, which reduces visibility resulting in road accidents. The increased air pollution disrupts air traffic. The surveys show that the levels of Carbon Monoxide and suspended particulate matter (SPM) had increased more than 100 folds and 5 folds respectively in the locations where tyres are burnt.

In most of the countries, unwanted tyres are typically disposed off into Landfills or shredded for refuse. Significant quantities are also stockpiled in the hope that an alternative disposal or reuse option will be found.

Many elastomers (including Natural Rubber), both in their virgin form and as recycled material (mainly from Scrap Tyres) have been and are being added to Bitumen or Asphalt to enhance the properties of road surface dressing. There has been interest in Rubber as a paving material for a long time. Rubber has been admixed with bitumen to improve the durability of road since 1930 and there have been many programmes investigated mainly by elastomer producers. The elastomers have been evacuated in the form of lattices and as both vulcanized and unvulcanized powders (fine powder) and Crumbs (large particles). Rubber ranges from the very cheap (various off-grade materials) to the relatively expensive, such as Poly-chloroprene and thermoplastic elastomers. The viscoelastic properties of the bitumen are modified by the addition of elastomeric materials. The advantages are the following:

1. Longer life for the surface dressing.
2. Reduction in cracking and fatting up.
3. Improved resistance to flow or deformation.
4. Improved adhesion and resistance to stripping.
5. Reduction in Aquaplaning (the water drainage capacity may be increased and road noise).
6. The low temperature properties of surface dressing are also improved.
7. In many cases it is possible to achieve a reduction in the pavement thickness.
8. It is possible to lay the material under a wider range of climate conditions.
9. In countries like India, which lack in adequate supply of bitumen, the use of rubber Crumbs from Scrap tyres may be regarded as a means of limiting import.

Test showed that unvulcanized materials (lattices or powders) are about twice as effective as vulcanized virgin materials. Scrap tyres Crumbs showed a very marginal enhancement in the properties of the bitumen in comparison with these materials. Nevertheless, it is at least questionably whether it would be detrimental.

## CHAPTER 2

## PROPERTIES AND COMPOSITION OF TYRES

A wide range of chemical compounds such as Natural Rubber, SBR (Styrene Butadiene Rubber) and Butadiene Rubber can be found in the tyre rubber of road vehicles. Chemical analysis of tyre material has also revealed that metals such as Zinc, Iron and Calcium can be present in the different concentrations. A large variety of other chemical substances are also added to tyre rubber viz. Vulcanizing agents, Retardants, Pigments, Fillers, Reinforcing agents, Softeners, Antioxidants, Antiozonants and desiccants.

## Composing by weight in tyre rubber

1. Natural Rubber $-40 \%$
2. $\operatorname{SBR}-30 \%$
3. Butadiene Rubber-20\%
4. Butyl and Halogenated Butyl Rubber - 10\%

Tyres mainly consist of steel, rubber compounds and textiles (often in the form of cotton). The typical compounds of tyre are as follows:

| Tyre composition | Number of parts containing the <br> material | \% weight |
| :--- | :---: | :---: |
| Rubber Hydrocarbon | 100 | 51 |
| Carbon Black | 50 | 26 |
| Oil | 25 | 13 |
| Sulphur | 2 | 1 |
| Zinc Oxide | 4 | 2 |
| Other chemicals | 15 | 7 |

The rubber is made stiffer by the process of Vulcanization. Degree of hardness and stiffness depends upon the quantity of sulphur added. For example, tyre rubber may contain $3-5 \%$ sulphur while a battery case rubber contains as much as $30 \%$ sulphur.

The following table gives the comparison between properties of raw rubber and Tyre rubber.

| S.No. | Property | Raw Rubber | Tyre Rubber |
| :--- | :--- | :--- | :--- |
| 1. | Tensile Strength | $200 \mathrm{~kg} / \mathrm{cm}^{2}$ | $2000 \mathrm{~kg} / \mathrm{cm}^{2}$ |
| 2. | Elongation at break (\%) | 1200 | 800 |
| 3. | Water Absorption | Large | Small |
| 4. | Swelling in Organic Solvents | Infinite | Large |
| 5. | Tackiness | Marked | Light |
| 6. | Useful Temperature Range | $10-60^{\circ} \mathrm{C}$ | $-40-100^{0} \mathrm{C}$ |
| 7. | Chemical Resistance | Very poor | Much better |
| 8. | Elasticity | Very high | Depends on degree |

The major parts or components of a tyre are the Bead, the Carcass, the Sidewall and the Tread. The composition of the sidewall and tread rubber is given below:

| Tread Rubber | Side Wall Rubber |
| :--- | :--- |
| Smoked sheet -50 parts | Smoked sheet -50 parts |
| SBR -70 parts | SBR -70 parts |
| Zinc oxide -5 parts | Zinc oxide -5 parts |
| Stearic acid -2 parts | Stearic Acid -2 Parts |
| ISAF Black -60 parts | ISAF Black -45 parts |
| Softener -2 parts | Softener -2 parts |
| Anti oxidant -1.5 parts | Anti oxidant -1.5 parts |
| Accelerator -1.0 part | Accelerator -1.0 part |
| Sulphur -2.2 parts | Sulphur -2.2 parts |

The physical properties of the rubber as compared to other polymers are given below:

| Material | Density <br> $\left(\mathbf{1 0}^{\mathbf{3}}\right.$ <br> $\left.\mathbf{k g} / \mathbf{m}^{\mathbf{3}}\right)$ | Thermal <br> Conductivity <br> $\left(\mathbf{J ~ m}^{-1} \mathbf{K}^{-1} \mathbf{S}^{\mathbf{- 1}}\right)$ | Thermal <br> Expansion <br> $\left(\mathbf{1 0}^{-6} \mathbf{K}^{-\mathbf{1}}\right)$ | Young's <br> Modulus <br> $\left(\mathbf{G N} / \mathbf{m}^{\mathbf{2}}\right)$ | Tensile <br> Strength <br> $\left(\mathbf{M N} / \mathbf{m}^{\mathbf{2}}\right)$ | \% <br> Elongation |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Tyre Rubber | 1.2 | 0.12 | 80 | $1-3$ | - | - |
| Polyethylene | 0.9 | 0.34 | 180 | 0.2 | $15-35$ | $100-500$ |
| Polystyrene | 1.05 | 0.08 | 60 | 3.0 | 55 | 3 |
| Nylon | 1.15 | 0.25 | 100 | 3.0 | 80 | $40-80$ |

Currently $20 \%$ of scrap tyres are retreaded, $5 \%$ used as boat fenders, silage, clamps etc. $10 \%$ crumbed for secondary raw materials, $15 \%$ incinerated and $50 \%$ land filled, dumped and stockpiled. In 199160,000 tones of rubber crumbs was produced in UK, and as much as 20,000 tones are used in sports and play surfaces, brake lining and in road asphalt. Rubberized road can increase road elasticity, temperature range and resistance to oxidation, which can results in fewer ruts, potholes and cracks in pavement surface.

The various uses of Scrap tyres are the following:

1. Boat and dock fenders.
2. Weights on silage sheeting on farms.
3. Crash barriers at motor racing circuits.
4. Children play surfaces and furniture.
5. Protection for young plants and trees.
6. Compost heap containers.
7. Structural support for earth walls.
8. Road surfaces.

## CHAPTER- 3

## Experimental Investigation

### 3.1 Proposed Experimental Work

In this experimental work, the possibility of using Scrap Tyre Crumbs in concrete for pavements is investigated.

Tyre Crumbs of three different sizes will be used to replace $5 \%$ of Fine Aggregate in concrete. The compressive strength of 150 mm size Cubes at 28 day will be investigated. The Crumb size giving the highest strength would be chosen.

After fixing the size of the Crumbs, the next step is to find out what percentage of Crumbs gives the higher strength. For that, different percentages of Fine Aggregate viz. $5 \%, 10 \%, 15 \%$ and $20 \%$ will be replaced by scrap tyre Crumbs. Compressive strength and Modulus of Elasticity (Both Static and Dynamic) of the concrete with different percentages of Crumbs will be studied by casting cubes of size 150 mm and cylinders of 150 mm diameter and 300 mm length.

In order to modify the workability properties of concrete, CEMWET SP 3000 super plasticizer admixture has been used with $5 \%$ and $10 \%$ of Crumbs. Concrete Cube, Cylinder and Beam specimens have been cast for calculating compressive strength, Modulus of Elasticity and Flexural strength of the modified concrete.

### 3.2 Materials Used in the Experimental Work

The following materials were used in the experimental work

1. ACC Cement (OPC) (43 Grade) having Specific Gravity of 3.05.
2. $\quad$ Shriram Cement (OPC) (53 Grade) having Specific Gravity of 3.10.
3. Fine aggregate conforming to grading zone II and having Specific Gravity of 2.45 .
4. Coarse Aggregate of 20 mm size having Specific Gravity of 2.69.
5. Coarse Aggregate of 10 mm size having Specific Gravity of 2.70.
6. Scrap tyres Crumbs having Specific Gravity of 0.97.
7. CEMWET SP 3000 admixture, having following properties.

The basic components of CEMWET SP3000 are Synthetic polymers with high molecular weight Naphthalene Formaldehyde Condensates that allows mixing water to be reduced considerably. It is a Chloride free product having prestigious mark of IS: 9103-99. Its typical properties are:

Colour : Brown free flowing liquid.
Specific Gravity: $1.22-0.020$
Chloride content: NIL to BS: 5075 to IS: 456-1978
Nitrate Content: NIL
Freezing Point: $0^{0} \mathrm{C}$ can be reconstituted if stirred after thawing.
Air entrained: Maximum 1.5\%

## Primary uses are:

- Production of plastic self compacting concrete
- Ready Mixed Concrete
- Low Water Cement ratio concrete.


### 3.3 Experimental Setup

The following instruments were used in the experimental work

## 1. Ultrasonic Instrument (As per IS-13311:1992)

The Ultrasonic instrument is used for the non-destructive testing of the concrete. The other data of the Instrument used in experiment work is given below:
(a) Display unit with non-volatile memory for up to 250 measured values, $128 \times 128$ graphic LCD.
(b) RS 232 C interface.
(c) Integrated software for transmission of the measured values to PC.
(d) Measuring range: Approx. 15 to $6550 \mu$ s.
(e) $\quad$ Resolution : $0.1 \mu \mathrm{~s}$
(f) Voltage pulse : 1KV
(g) Pulse rate : $3 / 2$
(h) Impedance at input: $1 \mathrm{M} \Omega$
(i) Temperature range : $-10^{\circ}$ to $+60^{\circ} \mathrm{C}$
(j) Battery operation with six LR 6 batteries, 1.5 V for 30 hour.
(k) Transducer 54 KHz .
(1) Cable BNC, $\mathrm{L}=1.5 \mathrm{~m}$
(m) Calibration rod.
(n) Coupling paste.


## 2. Vibrating Table

In this type of vibrator, the vibrator is clamped to the table or table is mounted on springs, which are vibrated, transferring the vibrations to the table. They are commonly used for vibrating concrete cubes. This is adopted mostly in the laboratories and in making small but precise prefabricated R.C.C. members.


## 3. Water Bath

This is used to find out the compressive strength of accelerated cured concrete test specimen. It has temperature control unit by which we can set the required temperature at which concrete specimen are cured.


## 4. Universal Testing Machine (UTM)

This is used to find out the strength of concrete specimen in compression.


## CHAPTER- 4

## MIX DESIGN

(As per IS: 10262-1982)

## 1. Design Stipulations

(a) Characteristic compressive strength required at 28 days $=20 \mathrm{~N} / \mathrm{mm}^{2}$
(b) Maximum size of aggregate $=20 \mathrm{~mm}$
(c) Degree of Workability $=0.70$ C.F.
(d) Degree of Quality Control = Good
(e) Type of exposure = Mild

## 2. Test Data for Materials

(a) $\quad$ Specific gravity of Cement $=3.05$
(b) Compressive Strength of Cement at 7 Days $=$ Satisfying IS: 269-1989
(c) Specific Gravity of Coarse aggregate $=2.80$
(d) Specific Gravity of fine aggregate $=2.44$
(e) Water absorption of coarse aggregate $=0.50 \%$
(f) Water absorption of fine aggregate $=1.0 \%$

| Sieve Size | Weight Retained (gm) | Cumulative Weight Retained (gm) | Percentage Weight Retained | Percentage Weight Passing | Remarks |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 10 mm | 0 | 0 | 0 | 100 | Conforming to Grading Zone-II of Table-4 IS : 383-1970 |
| 4.75 mm | 12 | 12 | 1.2 | 98.8 |  |
| 2.36 mm | 11 | 23 | 2.3 | 97.7 |  |
| 1.18 mm | 70 | 93 | 10.0 | 90.0 |  |
| $600 \mu$ | 353 | 446 | 44.6 | 55.4 |  |
| $300 \mu$ | 440 | 886 | 88.6 | 11.4 |  |
| $150 \mu$ | 91 | 977 | 97.7 | 2.3 |  |
| $75 \mu$ | 17 | 994 | 99.4 | 0.6 |  |
| Pan | 3 | 1000 | 100 | 0 |  |

## (i) Target Mean Strength

$$
\begin{aligned}
& \mathrm{f}_{\mathrm{t}}=\mathrm{f}_{\mathrm{ck}}+\mathrm{ks} \\
& =20+(1.65 \times 4)=26.6 \mathrm{~N} / \mathrm{mm}^{2}
\end{aligned}
$$

## (ii) Selection of Water-Cement Ratio

From Figure 46 of SP: 23-1982, the W/C ratio required for the target mean strength of $26.6 \mathrm{~N} / \mathrm{mm}^{2}$ is 0.48 . This is lower than the maximum value of 0.5 prescribed for the Moderate exposure (Refer table 5 of IS: 456-2000). Adopt W/C ratio of 0.48.

So,
Effective W/C ratio $=0.48$

## (iii) Selection of Water and Sand Content

From Table 42 of SP : 23-1982, for 20 mm maximum size aggregate, sand conforming to grading Zone II, Water content per Cubic meter of concrete is 186 $\mathrm{kg} / \mathrm{m}^{3}$ and sand content as percentage of total aggregate by absolute value is $35 \%$.

For change in value in W/C ratio, compacting factor for sand belonging to Zone II, the following adjustment is required as per Table 44 of SP: 23-1982.

| Changed Condition | Adjustment required in |  |
| :--- | :---: | :---: |
|  | Water <br> Content | Sand in Total <br> Aggregate |
| For sand conforming to Zone II of table 4 of the <br> IS:383-1970 | 0.0 | 0.0 |
| For decrease of C.F. $(0.80-0.70)=0.10$ | -3.0 | 0.0 |
| Decrease of W/C ratio $(0.60-0.48)=0.12$ | 0.0 | -2.4 |
| Total | -3.0 | -2.4 |

Therefore,
Required Sand content as percentage of total aggregate $=35-2.4=32.6 \%$
Required Water content $=186-5.58=180.42 \mathrm{~kg} / \mathrm{m}^{3}$

## 5. Determination of Cement Content

$\mathrm{W} / \mathrm{C}$ ratio $=0.48$
Water $=180.42 \mathrm{~kg}$
So
Cement content $=180.42 / 0.48=375.87 \mathrm{~kg} / \mathrm{m}^{3}$

## 6. Determination of Coarse aggregate and Fine aggregate content

From Table 41 of SP: 23-1982, for maximum size of aggregate of 20 mm , the amount of entrapped air in the wet concrete is $2 \%$, then by equations
$\mathbf{V}=\left(\mathbf{W}+\mathbf{C} / \mathbf{S}_{\mathbf{C}}+\mathbf{I} / \mathbf{P} \times \mathbf{f}_{\mathbf{a}} / \mathbf{s}_{\mathbf{f}_{\mathbf{a}}}\right) \times \mathbf{1} / \mathbf{1 0 0 0}$
$0.98=\left(180.42+375.87 / 3.04+1 / 0.326 \mathrm{xf}_{\mathrm{a}} / 2.44\right) \times 1 / 1000$
$980=\left(303.7+\mathrm{f}_{\mathrm{a}} / 0.795\right)$
$\mathrm{f}_{\mathrm{a}}=537.5 \mathrm{~kg} / \mathrm{m}^{3}$
Also,
$\mathbf{V}=\left(\mathbf{W}+\mathbf{C} / \mathbf{S}_{\mathbf{C}}+\mathbf{1} / \mathbf{1}-\mathbf{p x C}_{\mathbf{a}} / \mathbf{S}_{\mathbf{c a}}\right) \times \mathbf{1} / \mathbf{1 0 0 0}$
$980=\left(303.7 / \mathrm{C}_{\mathrm{a}} / 1.89\right)$
$\mathrm{C}_{\mathrm{a}}=1278.2 \mathrm{~kg} / \mathrm{m}_{3}$
Where
$\mathrm{C}=$ Cement content per cubic meter of concrete.
$\mathrm{S}_{\mathrm{C}}=$ Specific Gravity of Cement
$\mathrm{F}_{\mathrm{a}}=$ Content of Fine aggregate per cubic meter of Concrete
$\mathrm{S}_{\mathrm{fa}_{\mathrm{a}}}=$ Specific gravity of Fine aggregate
$\mathrm{C}_{\mathrm{a}}=$ Content of Coarse aggregate per cubic meter of concrete
$\mathrm{S}_{\mathrm{ca}}=$ Specific gravity of Coarse aggregate.

## Final Mix Proportion is

| Water | Cement | Fine Aggregate | Coarse Aggregate |
| :---: | :---: | :---: | :---: |
| 180.42 | 375.87 | 537.5 | 1278.2 |
| 0.480 | 1.0 | 1.430 | 3.400 |

## CHAPTER -6

## CHOICE OF SCRAP TYRE CRUMBS CONTENT IN CONCRETE

6.1 Effect of percentage of Crumbs on the 28 Days Compressive Strength of Concrete.
6.2 Effect of percentage of Crumbs on the 7 Days Compressive Strength of Concrete.
6.3 Effect of percentage of Crumbs on the 24 Hours Compressive Strength of Concrete.
6.4 Effect of percentage of Crumbs on the Dynamic Modulus of Elasticity of Concrete.

Calculation for modified mix proportion with crumbs is presented in Appendix A2.

### 6.1 Effect of Percentage of Crumbs on the 28 Days Compressive

Strength of Concrete

| S. No. | Specimen | 28 Days Compressive Strength of Concrete $\left(\mathrm{N} / \mathrm{mm}^{2}\right)$ | Average 28 Days Compressive Strength of Concrete $\left(\mathrm{N} / \mathrm{mm}^{2}\right)$ |
| :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |
| 1. | Cube | 34.23 | 35.59 |
|  | Cube | 35.31 |  |
|  | Cube | 36.84 |  |
|  | Cube | 35.97 |  |
| CONCRETE WITH 5\% CRUMBS |  |  |  |
| 2. | Cube | 27.69 | 26.85 |
|  | Cube | 25.60 |  |
|  | Cube | 27.27 |  |
|  | Cube | 26.82 |  |
| CONCRETE WITH 10\% CRUMBS |  |  |  |
| 3. | Cube | 21.37 | 21.80 |
|  | Cube | 22.01 |  |
|  | Cube | 22.24 |  |
|  | Cube | 21.58 |  |
| CONCRETE WITH 15\% CRUMBS |  |  |  |
| 4. | Cube | 9.59 | 9.91 |
|  | Cube | 10.02 |  |
|  | Cube | 9.81 |  |
|  | Cube | 10.25 |  |
| CONCRETE WITH 20\%CRUMBS |  |  |  |
| 5. | Cube | 7.63 | 8.01 |
|  | Cube | 8.07 |  |
|  | Cube | 8.28 |  |
|  | Cube | 8.07 |  |

## Effect of percentage of Crumbs on the 28 Days Compressive strength of concrete



### 6.2 Effect of Percentage of Crumbs on the 7 Days Compressive

 Strength of Concrete| S. No. | Specimen | 7Days Compressive Strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ ) | Average 7DaysCompressive Strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ ) |
| :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |
| 1. | Cube | 27.0 | 26.26 |
|  | Cube | 25.28 |  |
|  | Cube | 25.94 |  |
|  | Cube | 26.82 |  |
| CONCRETE WITH 5\% CRUMBS |  |  |  |
| 2. | Cube | 17.66 | 19.29 |
|  | Cube | 19.84 |  |
|  | Cube | 17.44 |  |
|  | Cube | 18.75 |  |
| CONCRETE WITH 10\% CRUMBS |  |  |  |
| 3. | Cube | 12.21 | 13.13 |
|  | Cube | 12.86 |  |
|  | Cube | 13.29 |  |
|  | Cube | 14.17 |  |
| CONCRETE WITH 15\% CRUMBS |  |  |  |
| 4. | Cube | 8.94 | 9.89 |
|  | Cube | 10.47 |  |
|  | Cube | 9.38 |  |
|  | Cube | 9.81 |  |
| CONCRETE WITH 20\%CRUMBS |  |  |  |
| 5. | Cube | 8.07 | 8.39 |
|  | Cube | 8.07 |  |
|  | Cube | 8.51 |  |
|  | Cube | 8.28 |  |

## Effect of percentage of Crumbs on the 7 Days Compressive strength of concrete



A comparative study of compressive strength at 28 days and 7 days shows that:
a) For $\mathbf{0 \%}, \mathbf{5 \%}$ and $\mathbf{1 0 \%}$ crumbs, the 28 days strength is much higher than the $\mathbf{7}$ day strength.
b) For $\mathbf{1 5 \%}$ and $\mathbf{2 0 \%}$ crumbs, the $\mathbf{2 8}$ days strength is same as $\mathbf{7}$ days strength.

This shows that the use of $\mathbf{1 5 \%}$ or more crumbs is not going to give good results.

### 6.3 Compressive Strength By Accelerated Curing By Warm Water Method (IS : 9013-1978)

In this method small cubes of 7.05 cm side are prepared using coarse aggregate of 10 mm size only. After the specimens have been made, they are left to stand undisturbed in their moulds in a place free from vibration at a temperature of $27 \pm 2^{0} \mathrm{C}$ for at least one hour prior to immersion in the curing tank. The time between the addition of water to the ingredients and immersion of the specimen in the curing tank is maintained at least $11 / 2$ hours but shall not exceed $31 / 2$ hours.

The specimens in their moulds are gently lowered into the curing tank and remain totally immersed at $55 \pm 2^{0} \mathrm{C}$ for a period of not less than 19 hours 50 minutes. The specimen are then removed from the water, removed from the mould and immersed in the cooling tank at $27 \pm 2^{\circ} \mathrm{C}$ before the completion of 20 hours 10 minutes from the start of immersion in the curing tank. They remain in the cooling tank for a period of not less than one hour.

Let,
$R_{a}=$ Accelerated strength of specimen in $\mathrm{N} / \mathrm{mm}^{2}$ at 24 hours.
$\mathrm{R}_{28}=$ Strength of specimen after 28 days in $\mathrm{N} / \mathrm{mm}^{2}$.

Then,
From IS: 9013-1978 the 28 days compressive strength of specimen can be calculated from accelerated strength by the regression equation.

$$
\mathbf{R}_{28}=\mathbf{1 2 . 6 5}+\mathbf{R}_{\mathrm{a}}
$$

## Calculation of modified mix design are presented in Appendix A3

| S. No. | Specimen | Accelerated Curing Strength $\left(\mathbf{R}_{\mathrm{e}}\right)$ ( $\mathrm{N} / \mathrm{mm}^{2}$ ) | Estimated 28 Days Compressive Strength $\left(R_{28}\right)$ $\left(\mathrm{N} / \mathrm{mm}^{2}\right)$ |  |
| :---: | :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |  |
| 1. | Cube | 11.01 | 23.66 | 22.70 |
|  | Cube | 10.01 | 22.66 |  |
|  | Cube | 10.21 | 22.86 |  |
|  | Cube | 9.00 | 21.65 |  |
| CONCRETE WITH 5\% CRUMBS |  |  |  |  |
| 2. | Cube | 8.00 | 20.65 | 19.65 |
|  | Cube | 7.00 | 19.65 |  |
|  | Cube | 6.00 | 18.65 |  |
|  | Cube | 7.00 | 19.65 |  |
| CONCRETE WITH 10\% CRUMBS |  |  |  |  |
| 3. | Cube | 5.00 | 17.65 | 17.40 |
|  | Cube | 4.00 | 16.65 |  |
|  | Cube | 5.00 | 17.65 |  |
|  | Cube | 5.00 | 17.65 |  |
| CONCRETE WITH 15\% CRUMBS |  |  |  |  |
| 4. | Cube | 4.00 | 16.65 | 15.90 |
|  | Cube | 3.00 | 15.65 |  |
|  | Cube | 3.00 | 15.65 |  |
|  | Cube | 3.00 | 15.65 |  |
| CONCRETE WITH 20\% CRUMBS |  |  |  |  |
| 5. | Cube | 3.00 | 15.65 | 15.15 |
|  | Cube | 2.00 | 14.65 |  |
|  | Cube | 2.00 | 14.65 |  |
|  | Cube | 3.00 | 15.65 |  |



### 6.4 Ultrasonic Pulse Velocity Method (IS-13311:1992)

In Ultrasonic Pulse Velocity Method, the Ultrasonic pulse is generated by an electro acoustical transducer. When pulse is induced into the concrete from a transducer, it undergoes multiple reflections at the boundaries of the different material phases within the concrete. A complex system of stress wave is developed which includes longitudinal, shear and surface waves. The receiving transducer detects the onset of the longitudinal waves, which is the fastest.

The Ultrasonic Pulse Velocity Method could be used to establish:
(a) The homogeneity of concrete
(b) The presence of cracks, voids and other imperfections
(c) Changes in the structure of the concrete which may occur with time
(d) The quality of concrete in relation to standard requirements.
(e) The values of dynamic elastic modulus of the concrete.

The quality of concrete in terms of uniformity, incidence or absence of internal flaws, crack and segregation etc. can thus be assessed using the guidelines given in the following table:

| S.No. | Pulse Velocity by Cross Probing (km/sec.) | Concrete quality grading |
| :---: | :---: | :---: |
| 1. | Above 4.5 | Excellent |
| 2. | 3.5 to 4.5 | Good |
| 3. | 3.0 to 3.5 | Medium |
| 4. | Below 3.0 | Doubtful |

The Dynamic Modulus of Elasticity (E) of the concrete may be determined from pulse velocity and dynamic Poisson's ratio ( $\mu$ ) using the following relationship:

$$
\frac{E=\rho(1+\mu)(1-2 \mu) V^{2}}{1-\mu}
$$

Where,
$\mathrm{E}=$ Dynamic Modulus of Elasticity in MPa.
$\rho=$ Density of concrete in $\mathrm{kg} / \mathrm{m}^{3}$
$\mathrm{V}=$ Pulse velocity in $\mathrm{km} / \mathrm{sec}$.
$\mu=$ Poisson's Ratio (0.24)

Calculations of materials for cylinders are presented in Appendix A4

## OBSERVATIONS

| Specimen | $\text { Time }(\mu \mathrm{s})$ <br> (t) | Distance (m) (I) | Velocity (m/s) (V) | Weight (kg) |
| :---: | :---: | :---: | :---: | :---: |
| Control Mix |  |  |  |  |
| Cylinder | 34.2 | 0.150 | 4386 | 13.250 |
| Cylinder | 33.1 | 0.150 | 4532 | 13.400 |
| Cylinder | 33.9 | 0.150 | 4425 | 13.540 |
| Concrete with 5\% Crumbs |  |  |  |  |
| Cylinder | 35.9 | 0.150 | 4178 | 13.075 |
| Cylinder | 36.5 | 0.150 | 4109 | 13.025 |
| Cylinder | 33.3 | 0.150 | 4504 | 13.060 |
| Concrete with 10\% Crumbs |  |  |  |  |
| Cylinder | 38.6 | 0.150 | 3886 | 13.025 |
| Cylinder | 35.0 | 0.150 | 4285 | 13.010 |
| Cylinder | 36.2 | 0.150 | 4143 | 13.020 |
| Concrete with 15\% Crumbs |  |  |  |  |
| Cylinder | 38.6 | 0.150 | 3886 | 13.000 |
| Cylinder | 37.9 | 0.150 | 3958 | 12.900 |
| Cylinder | 38.9 | 0.150 | 3856 | 12.950 |
| Concrete with 20\% Crumbs |  |  |  |  |
| Cylinder | 38.5 | 0.150 | 3896 | 12.800 |
| Cylinder | 40.5 | 0.150 | 3703 | 12.750 |
| Cylinder | 37.5 | 0.150 | 4000 | 12.790 |

## RESULT

| Specimen | Mass <br> (kg) | $\begin{aligned} & \text { Volume } \\ & \left(\mathrm{m}^{3}\right) \end{aligned}$ | $\begin{aligned} & \text { Density } \\ & \left(\mathrm{kg} / \mathrm{m}^{3}\right) \end{aligned}$ | Velocity (m/sec) | Dynamic <br> Modulus of <br> Elasticity <br> (GPa) | Average <br> Modulus <br> of <br> Elasticity <br> (GPa) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |  |  |  |
| Cylinder | 13.250 | 0.00530 | 2500.00 | 4386 | 40.78 | 42.25 |
| Cylinder | 13.400 | 0.00530 | 2528.30 | 4532 | 44.03 |  |
| Cylinder | 13.540 | 0.00530 | 2554.72 | 4425 | 41.94 |  |
| CONCRETE WITH 5\% CRUMBS |  |  |  |  |  |  |
| Cylinder | 13.075 | 0.00530 | 2466.98 | 4178 | 36.16 | 37.50 |
| Cylinder | 13.025 | 0.00530 | 2457.54 | 4109 | 35.03 |  |
| Cylinder | 13.060 | 0.00530 | 2464.15 | 4504 | 42.32 |  |
| CONCRETE WITH 10\% CRUMBS |  |  |  |  |  |  |
| Cylinder | 13.025 | 0.00530 | 2457.55 | 3886 | 31.47 | 35.15 |
| Cylinder | 13.010 | 0.00530 | 2454.72 | 4285 | 38.22 |  |
| Cylinder | 13.020 | 0.00530 | 2456.60 | 4143 | 35.76 |  |
| CONCRETE WITH 15\% CRUMBS |  |  |  |  |  |  |
| Cylinder | 13.000 | 0.00530 | 2452.83 | 3886 | 30.03 | 30.17 |
| Cylinder | 12.900 | 0.00530 | 2433.96 | 3958 | 31.39 |  |
| Cylinder | 12.950 | 0.00530 | 2443.39 | 3856 | 30.72 |  |
| CONCRETE WITH 20\% CRUMBS |  |  |  |  |  |  |
| Cylinder | 12.800 | 0.00530 | 2415.09 | 3896 | 30.99 | 30.56 |
| Cylinder | 12.750 | 0.00530 | 2405.66 | 3703 | 27.97 |  |
| Cylinder | 12.790 | 0.00530 | 2413.20 | 4000 | 32.74 |  | Elasticity



## CHAPTER- 7

## Effect of Admixture

7.1 Effect of Admixture \& Crumbs on the 28 Days Compressive Strength of Concrete.
7.2 Effect of Admixture \& Crumbs on the 1 year Compressive Strength of Concrete.
7.3 Effect of Admixture \& Crumbs on the Dynamic Modulus of Elasticity of Concrete.
7.4 Effect of Admixture \& Crumbs on the Flexural Strength of Concrete.

### 7.1 Effect of Admixture \& Crumbs on the 28 Days Compressive Strength of Concrete

Admixture CEMWET SP 3000 was added to the concrete with crumbs to study its affect on the compressive strength. The percentage of crumbs was kept as $\mathbf{5 \%}$ and $10 \%$.

The dosage of admixture was varied as $\mathbf{0 . 5 \%}, \mathbf{1 \%}$ and $\mathbf{1 . 5 \%}$.
The calculations for mix proportion are presented in Appendix A5.

### 7.1 Effect of Admixture \& Crumbs on the 28 Days Compressive Strength of Concrete

## 28 Days Compressive Strength of Concrete

| S.No. | Specimen | 28 Days Compressive Strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ ) | Average 28 Days Compressive Strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ ) |
| :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |
| 1. | Cube | 35.35 | 34.07 |
|  | Cube | 33.57 |  |
|  | Cube | 33.79 |  |
|  | Cube | 33.57 |  |
| CONCRETE WITHOUT CRUMBS \& 0.5\% ADMIXTURE |  |  |  |
| 2. | Cube | 35.32 | 35.65 |
|  | Cube | 35.97 |  |
|  | Cube | 35.54 |  |
|  | Cube | 35.75 |  |
| CONCRETE WITHOUT CRUMBS \& 1.0 \% ADMIXTURE |  |  |  |
| 3. | Cube | 37.06 | 36.95 |
|  | Cube | 36.62 |  |
|  | Cube | 36.84 |  |
|  | Cube | 37.28 |  |
| CONCRETE WITHOUT CRUMBS \& 1.5\% ADMIXTURE |  |  |  |
| 4. | Cube | 34.44 | 34.33 |
|  | Cube | 34.23 |  |
|  | Cube | 34.66 |  |
|  | Cube | 34.00 |  |
| CONCRETE WITH 5\% CRUMBS \& 0.5\% ADMIXTURE |  |  |  |
| 5. | Cube | 34.40 | 33.07 |
|  | Cube | 31.83 |  |
|  | Cube | 33.57 |  |
|  | Cube | 32.48 |  |
| CONCRETE WITH 5\% CRUMBS \& 1.0 \% ADMIXTURE |  |  |  |
| 6. | Cube | 34.66 | 34.22 |
|  | Cube | 34.00 |  |
|  | Cube | 34.23 |  |
|  | Cube | 34.00 |  |
| CONCRETE WITH 5\% CRUMBS \& 1.5\% ADMIXTURE |  |  |  |
| 7. | Cube | 30.08 | 29.88 |
|  | Cube | 29.87 |  |
|  | Cube | 29.69 |  |
|  | Cube | 29.87 |  |
| CONCRETE WITH 10\% CRUMBS \& 0.5\% ADMIXTURE |  |  |  |
| 8. | Cube | 34.44 | 34.39 |
|  | Cube | 34.66 |  |
|  | Cube | 34.23 |  |
|  | Cube | 34.23 |  |
| CONCRETE WITH 10\% CRUMBS \& 1.0 \% ADMIXTURE |  |  |  |
| 9. | Cube | 34.44 | 34.39 |
|  | Cube | 34.66 |  |
|  | Cube | 34.88 |  |
|  | Cube | 34.66 |  |
| CONCRETE WITH 10\% CRUMBS \& 1.5\% ADMIXTURE |  |  |  |
| 10. | Cube | 30.74 | 30.90 |
|  | Cube | 30.96 |  |
|  | Cube | 31.18 |  |
|  | Cube | 30.74 |  |

Effect of Percentage of Admixture on the Concrete without

## Crumbs


$\square 28$ Days compressive strength of concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ )

## Effect of Admixture on the 28 Days Compressive Strength of Concrete with 5\% Crumbs


$\square 28$ Days compressive strength of concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ )

Effect of Admixture on the 28 Days Compressive Strength of Concrete with 10\% Crumbs

$\square 28$ Days compressive strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ )

Effect of Percentage of Crumbs on the 28 Days Compressive Strength of Concrete with $0.5 \%$ Admixture

$\square 28$ Days compressive strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ )

Effect of Percentage of Crumbs on the 28 Days Compressive Strength of Concrete with $\mathbf{1 . 0 \%}$ Admixture

$\square 28$ Days compressive strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ )

Effect of Percentage of Crumbs on the 28 Days Compressive Strength of Concrete with 1.5\% Admixture

$\square 28$ Days compressive strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ )

It is found that the use of $\mathbf{1 0 \%}$ Crumbs with $\mathbf{1 . 0 \%}$ admixture results in a 28 Days compressive strength $34.6 \mathrm{~N} / \mathrm{mm}^{2}$ as compared to $34.07 \mathrm{~N} / \mathrm{mm}^{2}$ strength of the control mix. The compressive strength has increased.

### 7.2 Effect of Admixture \& Crumbs on the One Year Compressive Strength of Concrete

One Year Compressive Strength of Concrete

| S.No. | Specimen | 28 Days Compressive Strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ ) | Average 28 Days Compressive Strength of Concrete ( $\mathrm{N} / \mathrm{mm}^{2}$ ) |
| :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |
| 1. | Cube | 54.40 | 52.86 |
|  | Cube | 52.32 |  |
|  | Cube | 50.14 |  |
|  | Cube | 54.40 |  |
| CONCRETE WITHOUT CRUMBS \& 0.5\% ADMIXTURE |  |  |  |
| 2. | Cube | 56.68 | 58.31 |
|  | Cube | 58.86 |  |
|  | Cube | 58.86 |  |
|  | Cube | 58.86 |  |
| CONCRETE WITHOUT CRUMBS \& 1.0 \% ADMIXTURE |  |  |  |
| 3. | Cube | 58.86 | 59.40 |
|  | Cube | 56.68 |  |
|  | Cube | 61.04 |  |
|  | Cube | 61.04 |  |
| CONCRETE WITHOUT CRUMBS \& 1.5\% ADMIXTURE |  |  |  |
| 4. | Cube | 56.68 | 55.54 |
|  | Cube | 56.68 |  |
|  | Cube | 54.40 |  |
|  | Cube | 54.40 |  |
| CONCRETE WITH 5\% CRUMBS \& 0.5\% ADMIXTURE |  |  |  |
| 5. | Cube | 52.32 | 52.21 |
|  | Cube | 52.10 |  |
|  | Cube | 51.89 |  |
|  | Cube | 52.54 |  |
| CONCRETE WITH 5\% CRUMBS \& 1.0 \% ADMIXTURE |  |  |  |
| 6. | Cube | 54.06 | 54.39 |
|  | Cube | 54.50 |  |
|  | Cube | 54.72 |  |
|  | Cube | 54.28 |  |
| CONCRETE WITH 5\% CRUMBS \& 1.5\% ADMIXTURE |  |  |  |
| 7. | Cube | 45.78 | 46.38 |
|  | Cube | 45.99 |  |
|  | Cube | 45.78 |  |
|  | Cube | 47.96 |  |
| CONCRETE WITH 10\% CRUMBS \& 0.5\% ADMIXTURE |  |  |  |
| 8. | Cube | 54.50 | 54.39 |
|  | Cube | 54.28 |  |
|  | Cube | 54.06 |  |
|  | Cube | 54.72 |  |
| CONCRETE WITH 10\% CRUMBS \& 1.0 \% ADMIXTURE |  |  |  |
| 9. | Cube | 56.68 | 55.65 |
|  | Cube | 54.72 |  |
|  | Cube | 54.50 |  |
|  | Cube | 56.68 |  |
| CONCRETE WITH 10\% CRUMBS \& 1.5\% ADMIXTURE |  |  |  |
| 10. | Cube | 47.96 | 46.98 |
|  | Cube | 48.18 |  |
|  | Cube | 45.78 |  |
|  | Cube | 45.99 |  |

## Crumbs



## of Concrete with 5\% Crumbs



## Effect of Admixture on the One Year Compressive Strength of Concrete with 10\% Crumbs



## Effect of Percentage of Crumbs on the One Year Compressive Strength of Concrete with $0.5 \%$ Admixture



## Effect of Percentage of Crumbs on the One Year Compressive Strength of Concrete with $1.0 \%$ Admixture



## Effect of Percentage of Crumbs on the One Year Compressive Strength of Concrete with $1.5 \%$ Admixture



It is noted that the use of $\mathbf{1 0 \%}$ of crumbs with $\mathbf{1 \%}$ admixture has resulted in a one year compressive strength $55.65 \mathrm{~N} / \mathrm{mm}^{2}$ as compared to $52.86 \mathrm{~N} / \mathrm{mm}^{2}$ one year strength of control mix. The one year strength with the use of $\mathbf{1 0 \%}$ crumbs is also more than control mix.

### 7.3 Modulus of Elasticity of Concrete By Ultrasonic Pulse Velocity Method (Dynamic Modulus of Elasticity)

Calculations for mix proportion for cylinders with admixture are presented in Appendix A6.

OBSERVATIONS

| Specimen | Time ( $\mu \mathrm{s}$ ) <br> (t) | Distance (m) <br> (I) | Velocity (m/s) <br> (V) | Weight (kg) |
| :---: | :---: | :---: | :---: | :---: |
| Control Mix |  |  |  |  |
| Cylinder | 36.5 | 0.150 | 4109 | 13.095 |
| Cylinder | 36.0 | 0.150 | 4166 | 13.075 |
| Concrete Mix with 0.5\% Admixture \& 0\% Crumbs |  |  |  |  |
| Cylinder | 35.5 | 0.150 | 4225 | 13.100 |
| Cylinder | 35.2 | 0.150 | 4214 | 13.095 |
| Concrete Mix with 1.0\% Admixture \& 0\% Crumbs |  |  |  |  |
| Cylinder | 33.6 | 0.150 | 4464 | 13.125 |
| Cylinder | 35.0 | 0.150 | 4286 | 13.100 |
| Concrete Mix with 1.5\% Admixture \& 0\% Crumbs |  |  |  |  |
| Cylinder | 36.0 | 0.150 | 4167 | 13.075 |
| Cylinder | 35.8 | 0.150 | 4186 | 13.100 |
| Concrete Mix with 0.5\% Admixture \& 5.0\% Crumbs |  |  |  |  |
| Cylinder | 36.3 | 0.150 | 4132 | 13.010 |
| Cylinder | 36.1 | 0.150 | 4155 | 13.020 |
| Concrete Mix with 1.0\% Admixture \& 5.0\% Crumbs |  |  |  |  |
| Cylinder | 36.5 | 0.150 | 4109 | 13.025 |
| Cylinder | 36.0 | 0.150 | 4167 | 13.020 |
| Concrete Mix with $\mathbf{1 . 5 \%}$ Admixture \& 5.0\% Crumbs |  |  |  |  |
| Cylinder | 37.5 | 0.150 | 4000 | 13.015 |
| Cylinder | 37.3 | 0.150 | 4021 | 13.030 |
| Concrete Mix with 0.5\% Admixture \& 10\% Crumbs |  |  |  |  |
| Cylinder | 36.2 | 0.150 | 4143 | 13.00 |
| Cylinder | 36.5 | 0.150 | 4109 | 13.010 |
| Concrete Mix with 1.0\% Admixture \& 10\% Crumbs |  |  |  |  |
| Cylinder | 36.3 | 0.150 | 4132 | 13.005 |
| Cylinder | 36.6 | 0.150 | 4098 | 13.000 |
| Concrete Mix with 1.5\% Admixture \& 10\% Crumbs |  |  |  |  |
| Cylinder | 37.2 | 0.150 | 4032 | 13.015 |
| Cylinder | 37.1 | 0.150 | 4043 | 13.010 |

## RESULTS

| Specimen | $\begin{gathered} \text { Mass } \\ \text { (kg) } \end{gathered}$ | $\begin{gathered} \text { Volume } \\ \left(\mathrm{m}^{3}\right) \end{gathered}$ | $\begin{gathered} \text { Density } \\ \left(\mathrm{kg} / \mathrm{m}^{3}\right) \end{gathered}$ | Velocity (m/sec) | Dynamic Modulus of Elasticity (GPa) | Average Modulus of Elasticity (GPa) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CONTROL MIX |  |  |  |  |  |  |
| Cylinder | 13.095 | 0.00530 | 2470.76 | 4109 | 35.39 | 35.86 |
| Cylinder | 13.075 | 0.00530 | 2466.98 | 4167 | 36.33 |  |
| CONCRETE WITHOUT CRUMBS \& 0.5\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.100 | 0.00530 | 2471.69 | 4225 | 37.43 | 37.32 |
| Cylinder | 13.095 | 0.00530 | 2470.75 | 4214 | 37.22 |  |
| CONCRETE WITHOUT CRUMBS \& 1.0\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.125 | 0.00530 | 2476.41 | 4464 | 41.86 | 40.19 |
| Cylinder | 13.100 | 0.00530 | 2471.69 | 4286 | 38.52 |  |
| CONCRETE WITHOUT CRUMBS \& 1.5\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.075 | 0.00530 | 2466.98 | 4166 | 36.32 | 36.55 |
| Cylinder | 13.100 | 0.00530 | 2471.69 | 4189 | 36.79 |  |
| CONCRETE WITH 5\% CRUMBS \& 0.5\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.010 | 0.00530 | 2454.72 | 4132 | 35.56 | 35.77 |
| Cylinder | 13.020 | 0.00530 | 2456.60 | 4155 | 35.98 |  |
| CONCRETE WITH 5\% CRUMBS \& 1.0\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.025 | 0.00530 | 2457.55 | 4109 | 35.20 | 35.68 |
| Cylinder | 13.020 | 0.00530 | 2456.60 | 4167 | 36.17 |  |
| CONCRETE WITH 5\% CRUMBS \& 1.5\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.015 | 0.00530 | 2455.66 | 4000 | 33.34 | 33.53 |
| Cylinder | 13.030 | 0.00530 | 2458.49 | 4021 | 33.72 |  |
| CONCRETE WITH 10\% CRUMBS \& 0.5\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.000 | 0.00530 | 2452.83 | 4143 | 35.72 | 35.44 |
| Cylinder | 13.010 | 0.00530 | 2454.72 | 4109 | 35.16 |  |
| CONCRETE WITH 10\% CRUMBS \& 1.0\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.005 | 0.00530 | 2453.77 | 4132 | 35.54 | 35.24 |
| Cylinder | 13.000 | 0.00530 | 2452.83 | 4098 | 34.94 |  |
| CONCRETE WITH 10\% CRUMBS \& 1.5\% ADMIXTURE |  |  |  |  |  |  |
| Cylinder | 13.015 | 0.00530 | 2455.66 | 4032 | 33.87 | 33.95 |
| Cylinder | 13.010 | 0.00530 | 2454.72 | 4043 | 34.04 |  |

### 7.4 Effect of Percentage of Admixture \& Crumbs on the Flexural Strength of Concrete

## Flexural Strength of Concrete

The theoretical maximum tensile stress reached at the bottom fibers of a test Beam is known as the Modulus of Rupture, which is relevant to the design of highways and air field pavements. A concrete road slab is called upon to resist tensile stresses from two principal sources-Wheel load and Volume change in concrete. Wheel load may cause high tensile stresses due to bending, when there is inadequate sub grade support. Volume changes, resulting from changes in temperature and moisture may produce tensile stresses due to warping and due to movement of slab along sub grade.

Flexural Strength of concrete in terms of Modulus of Rupture can be calculated by using the relation:-

$$
\sigma=\mathbf{P l} / \mathrm{bd}^{3}
$$

Where,
P= Load in Newton
L= Supported Span
b= Width of Beam
$\mathrm{d}=$ Depth of Beam

Calculations of mix proportions for beam specimens are presented in Appendix-A7

Type of Loading - Two Point Loading (At third point of span)
Size of Beam specimen $=\mathbf{1 0 0} \mathbf{m m} \times \mathbf{1 0 0} \mathbf{m m} \times 500 \mathrm{~mm}$
Supported span of the Beam Specimen $=\mathbf{4 5 0} \mathbf{m m}$

| S.No | Specimen Specification | Load (Newton) | Modulus of Rupture | Average value of Modulus of Rupture |
| :---: | :---: | :---: | :---: | :---: |
| 1. | Control Mix Concrete | 9810 | 4.42 | 6.07 |
|  |  | 14715 | 6.62 |  |
|  |  | 14715 | 6.62 |  |
|  |  | 14715 | 6.62 |  |
| 2. | Concrete without Crumbs \& 0.5 \% Admixture | 14715 | 6.62 | 6.62 |
|  |  | 14715 | 6.62 |  |
|  |  | 9810 | 4.42 |  |
|  |  | 19620 | 8.83 |  |
| 3. | Concrete without Crumbs \& 1.0 \% Admixture | 19620 | 8.83 | 7.17 |
|  |  | 14715 | 6.62 |  |
|  |  | 14715 | 6.62 |  |
|  |  | 14715 | 6.62 |  |
| 4. | Concrete without Crumbs \& 1.5 \% Admixture | 9810 | 4.42 | 5.52 |
|  |  | 14715 | 6.62 |  |
|  |  | 14715 | 6.62 |  |
|  |  | 9810 | 4.42 |  |
| 5. | $\begin{array}{\|l} \hline \text { Concrete with 5\% Crumbs \& 0.5 \% } \\ \text { Admixture } \end{array}$ | 9810 | 4.42 | 4.97 |
|  |  | 9810 | 4.42 |  |
|  |  | 9810 | 4.42 |  |
|  |  | 14715 | 6.62 |  |
| 6. | Concrete with 5\% Crumbs \& 1.0 \% Admixture | 14715 | 6.62 | 5.52 |
|  |  | 14715 | 6.62 |  |
|  |  | 9810 | 4.42 |  |
|  |  | 9810 | 4.42 |  |
| 7. | Concrete with 5\% Crumbs \& 1.5 \% Admixture. | 9810 | 4.42 | 3.86 |
|  |  | 9810 | 4.42 |  |
|  |  | 9810 | 4.42 |  |
|  |  | 4905 | 2.21 |  |
| 8. | Concrete with 10\% Crumbs \& 0.5 \%Admixture | 14715 | 6.62 | 4.97 |
|  |  | 9810 | 4.42 |  |
|  |  | 9810 | 4.42 |  |
|  |  | 9810 | 4.42 |  |
| 9. | Concrete with 10\% Crumbs \& 1.0 \% Admixture | 9810 | 4.42 | 5.52 |
|  |  | 14715 | 6.62 |  |
|  |  | 9810 | 4.42 |  |
|  |  | 14715 | 6.62 |  |
| 10. | Concrete with 10\% Crumbs \& 1.5 \% Admixture | 4905 | 2.21 | 3.32 |
|  |  | 9810 | 4.42 |  |
|  |  | 4905 | 2.21 |  |
|  |  | 9810 | 4.42 |  |

 Concrete with 5\% Crumbs


Effect of Percentage of Admixture on the Flexural Strength of Concrete with 10\% Crumbs


## Comparison of Flexural Strength with the IS: 456: 2000

As per IS: 456: 2000, the Flexural Strength of Concrete is given by the relation:-

$$
\sigma=0.7 \sqrt{ } \mathbf{f}_{\mathrm{ck}}
$$

Where,
$\mathrm{f}_{\mathrm{ck}}=$ Characteristic Compressive Strength of Concrete
$=20 \mathrm{~N} / \mathrm{mm}^{2}$ (Used in the test)

| S.No. | Mix | Flexural <br> Strength of <br> Concrete as per <br> IS: 456:2000 <br> (MPa) | Flexural <br> Strength of <br> Concrete as per <br> actual test <br> (MPa) |
| :---: | :---: | :---: | :---: |
| 1. |  <br> admixture | $\mathbf{3 . 1 3}$ | $\mathbf{6 . 0 7}$ |
| 2. |  <br> $0.5 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{6 . 6 2}$ |
| 3. |  <br> $1.0 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{7 . 1 7}$ |
| 4. |  <br> $1.5 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{5 . 5 2}$ |
| 5. |  <br> $0.5 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{4 . 9 7}$ |
| 6. |  <br> $1.0 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{5 . 5 2}$ |
| 7. |  <br> $1.5 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{3 . 8 6}$ |
| 8. |  <br> $0.5 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{4 . 9 7}$ |
| 9. |  <br> $1.0 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{5 . 5 2}$ |
| 10. |  <br> $1.5 \%$ admixture | $\mathbf{3 . 1 3}$ | $\mathbf{3 . 3 2}$ |

It is clear from the above table that with the addition of $0.5 \%$ and $\mathbf{1 . 0 \%}$ Admixture in the concrete, the value of Flexural Strength of concrete increases, but with $1.5 \%$ admixture, it decreases. With the addition of Crumbs, the flexural strength decreases. For concrete with $10 \%$ Crumbs and $1.0 \%$ admixture, the flexural strength is almost comparable to control mix.

## CHAPTER-8

## COST ANALYSIS

## Case-I (Concrete without Crumbs \& Admixture)

Let,

```
Length of Roadway \(=1.0 \mathrm{~km}\)
Width of Roadway \(=3.75 \mathrm{~m}\) (Single Lane)
Thickness of Roadway \(=15.0 \mathrm{~cm}\)
Grade of Concrete \(=\mathbf{M 2 0}\)
Type of Cement = ACC (OPC) (Specific Gravity -3.05)
Size of Coarse Aggregate \(=\mathbf{2 0} \mathbf{m m}\)
Specific Gravity of Coarse Aggregate = 2.8
Specific Gravity of Fine Aggregate = 2.44
Type of Admixture = CEMWET SP 3000 (Super Plasticizer)
Specific Gravity of Admixture \(=\mathbf{1 . 2 2}\)
```

As per the Mix Design by Indian Standard, mix proportion for M20 grade concrete comes out to be:-

| Water | Cement | Fine Aggregate | Coarse Aggregate |
| :---: | :---: | :---: | :---: |
| 0.480 | 1.0 | 1.430 | 3.400 |

Now,
Volume of Concrete required for the construction of Road is

$$
=1000 \times 3.75 \times 0.15=562.5 \mathrm{~m}^{3}
$$

So,

$$
\text { Mass }=562.5 \times 24 \times 1000 / 9.81=1376146.789 \mathrm{~kg}
$$

Then,
Cement Required $=1 /(1+1.430+3.400) \times 1376146.789=236045.762 \mathrm{~kg}$ $=236045.762 / 50=4721$
Bags
Water Required $=0.480 /(1+1.430+3.400) \times 1376146.789=113301.966 \mathrm{~kg}$
Fine Aggregate Required $=1.430 /(1+1.430+3.400) \times 1376146.789=337545.439 \mathrm{~kg}$

$$
=120.56 \mathrm{~m}^{3}
$$

Coarse Aggregate Required $=3.400 /(1+1.430+3.400) \times 1376146.789=802555.589$
kg

$$
=328.92 \mathrm{~m}^{3}
$$

Now,
Cost of Cement @ Rs. $250 /$ Bag $=4721 \times 250=$ Rs. $11,80,250 /-$
Cost of Fine Aggregate @ Rs. $350 / \mathrm{m}^{3}=120.56 \times 350=$ Rs. 42 , $196 /-$
Cost of Coarse Aggregate @ Rs. $200 / \mathrm{m}^{3}=328.92 \times 200=$ Rs. $65,784 /-$

## CASE 2

## (Concrete with Crumbs \& Admixture)

If we replace $10 \%$ of Fine Aggregate with Crumbs and $1.0 \%$ of Cement with Admixture, then the total cost comes out to be:

Now,
Quantity of Crumbs $=10 \%$ of Quantity of Fine Aggregate

$$
=10 \% \text { of } 337545.439 \mathrm{~kg}=33754.5439 \mathrm{~kg}
$$

So,
Net quantity of Fine Aggregate added

$$
\begin{aligned}
& =337545.439 \mathrm{~kg}-33754.5439 \mathrm{~kg} \\
& =303790.895 \mathrm{~kg} \\
& =108.50 \mathrm{~m}^{3}
\end{aligned}
$$

Similarly,

$$
\begin{aligned}
\text { Quantity of Admixture added } & =1.0 \% \text { of Quantity of Cement } \\
& =1.0 \% \text { of } 236045.762 \mathrm{~kg} \\
& =1935 \mathrm{~m}^{3} \\
& =1935 \text { liters }
\end{aligned}
$$

Then,
Net quantity of Cement added $\quad=236045.762 \mathrm{~kg}-2360.458 \mathrm{~kg}$

$$
\begin{aligned}
& =233685.305 \mathrm{~kg} \\
& =4674 \mathrm{Bags}
\end{aligned}
$$

Now,
Cost of Cement @ Rs.250/Bag = 4674 x 250 Rs.11, 68,428/-
Cost of Fine Aggregate @ Rs. $350 / \mathrm{m}^{3}=108.50 \times 350 \quad$ Rs.37, $975 /-$
Cost of Coarse Aggregate @ Rs.200/-m3 Rs.65, 784/-
Cost of Admixture @ Rs.30/litre $=1935 \times 30$ Rs.58, 050/-
Total Cost Rs.13, 30,237/-
Therefore,
Percentage increase in cost $=($ Rs.13, 30,237 - Rs.12,88,230 $) /$ Rs.13,30,237/-

$$
=3.26 \%
$$

## CONCLUSIONS

1. Improper disposal of the tyre waste can result in serious health hazard to the public.
2. It is clear from the Graph on page number ......that Crumbs having size $>4.75$ mm , gives higher strength of Concrete when 5\% of Fine Aggregate is replaced by this size of Crumbs in the Concrete Mix. Hence Crumbs of size $>4.75 \mathrm{~mm}$ only will be used in the rest of the laboratory investigations.
3. A comparative study of compressive strength at 28 days and 7 days shows that:
a) For $0 \%, 5 \%$ and $10 \%$ crumbs, the 28 days strength is much higher than the 7 day strength.
b) For $15 \%$ and $20 \%$ crumbs, the 28 days strength is same as 7 days strength. This shows that the use of $15 \%$ or more crumbs is not going to give good results.
4. The Dynamic Modulus of Elasticity of concrete reduces as the percentage of Crumbs is increased. The use of $15 \%$ and $20 \%$ Crumbs gives almost the same value, showing it to be the lowest possible value, as these percentages result in very low strengths also.
5. It is found that the use of $10 \%$ Crumbs with $1.0 \%$ admixture results in a 28 Days compressive strength $34.6 \mathrm{~N} / \mathrm{mm}^{2}$ as compared to $34.07 \mathrm{~N} / \mathrm{mm}^{2}$ strength of the control mix. The compressive strength has increased.
6. It is noted that the use of $10 \%$ of crumbs with $1.0 \%$ admixture has resulted in a one year compressive strength $55.65 \mathrm{~N} / \mathrm{mm}^{2}$ as compared to $52.86 \mathrm{~N} / \mathrm{mm}^{2}$ one year strength of control mix. The one year strength with the use of $10 \%$ crumbs is also more than control mix.
7. The Dynamic Modulus of Elasticity for concrete, with $5 \%$ Crumbs and $10 \%$ Crumbs, using $0.5 \%$ and $1.0 \%$ admixture are in the range of 35.24 to 35.77 GPa. These values compare very well with the value 35.86 GPa for the control mix. Hence, the use of $10 \%$ Crumbs with admixture gives the same Dynamic Modulus of Elasticity as of the control mix.
8. It is clear from the above table on page number $\qquad$ that with the addition of $0.5 \%$ and $1.0 \%$ Admixture in the concrete, the value of Flexural Strength of concrete increases, but with $1.5 \%$ admixture, it decreases. With the addition of Crumbs, the flexural strength decreases. For concrete with $10 \%$ Crumbs and $1.0 \%$ admixture, the flexural strength is almost comparable to control mix.
9. The use of $10 \%$ Crumbs results in a cost increase of $3.26 \%$.

From the above conclusion it is seen that $\mathbf{1 0 \%}$ fine aggregate can be safely replaced by used tyre Crumbs of size greater than 4.75 mm , but less than about 6.0 mm , if a suitable admixture is used. The compressive strength, flexural strength and dynamic modulus of elasticity are not much affected. After one year, the compressive strength of concrete with $\mathbf{1 0 \%}$ Crumbs is found to be even greater than control mix.

## REFERENCES

1. IS: 383-1970, Specification for Coarse and Fine Aggregates from Natural Sources for Concrete. Bureau of Indian Standards, New Delhi.
2. IS: 383 (Part 1)-1992, Non- Destructive Testing of Concrete - Methods of Tests. Bureau of Indian Standards, New Delhi.
3. IS: 9013-1978, Methods of Making, Curing and Determining Compressive Strength of Accelerated- Cured Concrete Test Specimens. Bureau of Indian Standards, New Delhi.
4. BS 1881: Part 203: 1986, Recommendations for Measurement of Velocity of Ultrasonic Pulses in Concrete. British Standards.
5. SP: 23, Handbook of Concrete Mix Design. Bureau of Indian Standards, New Delhi.
6. Gambhir, ‘Concrete Technology.’ Tata Mc Graw Hill Publishing Company, New Delhi. 1995, pp 83-101.
7. Dr. G.V. Rao, 'Principles of Transportation Engineering' Tata Mc Graw Hill Publishing Company, New Delhi. 1996, pp 291-298.
8. M.S. Shetty, 'Concrete Technology.' Standard Publications. New Delhi. 2003, pp 281-287 \& 372-375.
9. Premamoy Ghosh, 'Polymer Science \& Technology of Plastic \& Rubbers.'
10. Khanna \& Justo, 'Highway Engineering' Nem Chand \& Brothers. Roorkee.1997, pp 482-490.
11. A.M. Nivelle, J.J. Brooks, ‘Concrete Technology.' Educational Low-Priced Books Scheme. Addison Wesley Longman Limited. 1987, pp 303-310 \& 209214.

## APPENDIX - A1

## CONTROL MIX

Volume of concrete required for casting 3 cubes with $10 \%$ compensation is given as

$$
=3 \mathrm{x}(0.15)^{3} \times 1.1=0.0121 \mathrm{~m}^{3}
$$

So,

$$
\text { Mass }=29.60 \mathrm{~kg}
$$

Now
Cement required $=1 /(1+1.430+3.400) \times 29.60=5.077 \mathrm{~kg}$
Water required $=0.48 /(1+1.430+3.400) \times 29.60=2.437 \mathrm{~kg}$
Fine Aggregate required $=1.430 /(1+1.430+3.400) \times 29.60=7.260 \mathrm{~kg}$
Coarse Aggregate required $=3.400 /(1+1.430+3.400) \times 29.60=17.252 \mathrm{~kg}$ Now,

Extra water added for absorption of Fine aggregate $=1 \%$ of F.A. Quantity

$$
=1 \% \text { of } 7.250 \mathrm{~kg}=0.0726 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of C.A. Quantity

$$
=0.5 \% \text { of } 17.262 \mathrm{~kg}=0.0863 \mathrm{~kg}
$$

So,
Total extra quantity of Water added $=0.07256+0.0863=0.159 \mathrm{~kg}$ Then,

Total quantity of Water added in the $\mathrm{mix}=2.437+0.159=2.595 \mathrm{~kg}$ Therefore,

Actual quantity of Fine Aggregate required $=7.260-0.0726=7.187 \mathrm{~kg}$
Actual quantity of Coarse Aggregate required $=17.262-0.0863=17.176 \mathrm{~kg}$
Final Mix proportion per batch is

| Water | Cement | Fine Aggregate | Coarse Aggregate |
| :---: | :---: | :---: | :---: |
| 2.595 kg | 5.077 kg | 7.187 kg | 17.176 kg |

## CONCRETE WITH 5\% CRUMBS

Volume of concrete required for casting 3 cubes with $10 \%$ compensation is given as

$$
=3 \mathrm{x}(0.15)^{3} \times 1.1=0.0121 \mathrm{~m}^{3}
$$

So,

$$
\text { Mass }=29.60 \mathrm{~kg}
$$

Now, Cement required $=1 /(1+1.430+3.400) \times 29.60=5.077 \mathrm{~kg}$
Water required $=0.48 /(1+1.430+3.400) \times 29.60=2.437 \mathrm{~kg}$
Fine Aggregate required $=1.430 /(1+1.430+3.400) \times 29.60=7.260 \mathrm{~kg}$
Coarse Aggregate required $=3.400 /(1+1.430+3.400) \times 29.60=17.262 \mathrm{~kg}$
Then, Quantity of Crumbs required $=5 \%$ of Quantity of Fine Aggregate

$$
=5 \% \text { of } 7.260 \mathrm{~kg}=0.363 \mathrm{~kg}
$$

So,
Net quantity of Fine Aggregate required $=7.260-0.363=6.897 \mathrm{~kg}$
Now, Extra water added for absorption of Fine aggregate $=1 \%$ of F.A. Quantity

$$
=1 \% \text { of } 6.897 \mathrm{~kg}=0.0689 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of C.A. Quantity

$$
=0.5 \% \text { of } 17.262 \mathrm{~kg}=0.0863 \mathrm{~kg}
$$

So, $\quad$ Total extra quantity of Water added $=0.155 \mathrm{~kg}$
Then, Total quantity of water added in the mix $=2.437+0.155=2.592 \mathrm{~kg}$
Therefore, $\quad$ Actual $\mathrm{W} / \mathrm{C}$ ratio $=2.592 / 5.077=0.51$
Also, Actual quantity of Fine Aggregate required $=6.897-0.0689=6.828 \mathrm{~kg}$
Actual quantity of Coarse Aggregate required $=17.262-0.0863=17.176 \mathrm{~kg}$
So,

Final Mix proportion per batch is

| Water | Cement | Fine Aggregate | Coarse Aggregate | Crumbs |
| :---: | :---: | :---: | :---: | :---: |
| 2.592 kg | 5.077 kg | 6.828 kg | 17.176 kg | 0.363 kg |

## APPENDIX - A2

## CONTROL MIX

Volume of Concrete required to cast 4 Cubes of 150 mm sides with $20 \%$ compensation

$$
=4 \times(150)^{3} \times 1.2=0.0162 \mathrm{~m}^{3}
$$

So,
Mass $=0.0162 \times 24 \times 1000 / 9.81=39.633 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.437+3.263) \times 39.633=6.953 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 39.633=3.338 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 39.633=9.992 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 39.633=22.668 \mathrm{~kg}$
Now,
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 22.668 kg

$$
=0.114 \mathrm{~kg}
$$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.992 kg

$$
=0.099 \mathrm{~kg}
$$

Total extra water added $=0.114+0.099=0.214 \mathrm{~kg}$
Total water added to Concrete $=3.338+0.214=3.552 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=22.688-0.114=22.574 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=9.992-0.099=9.893 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 22.574 kg $=4.515 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ |
| :---: | :---: | :---: | :---: | :---: |
| 3.552 kg | 6.953 kg | 9.893 kg | 16.059 kg | 4.515 kg |

## CONCRETE WITH 5\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 150 mm sides with $20 \%$ compensation

$$
=4 \times(150)^{3} \times 1.2=0.0162 \mathrm{~m}^{3}
$$

So,
Mass $=0.0162 \times 24 \times 1000 / 9.81=39.633 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.437+3.263) \times 39.633=6.953 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 39.633=3.338 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 39.633=9.992 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 39.633=22.668 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=5 \%$ of $9.992 \mathrm{~kg}=0.500 \mathrm{~kg}$
So,
Quantity of Fine Aggregate required $=9.992-0.500=9.493 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 22.668 kg

$$
=0.114 \mathrm{~kg}
$$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.493 kg

$$
=0.0950 \mathrm{~kg}
$$

Total extra water added $=0.114+0.0950=0.209 \mathrm{~kg}$
Total water added to Concrete $=3.338+0.209=3.547 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=22.688-0.114=22.574 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=9.493-0.0950=9.398 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 22.574 kg $=4.515 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | C $_{\mathrm{az}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.547 kg | 6.953 kg | 9.398 kg | 18.059 kg | 4.515 kg | 0.500 kg |

## CONCRETE WITH 10\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 150 mm sides with $20 \%$ compensation

$$
=4 \times(150)^{3} \times 1.2=0.0162 \mathrm{~m}^{3}
$$

So,
Mass $=0.0162 \times 24 \times 1000 / 9.81=39.633 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.437+3.263) \times 39.633=6.953 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 39.633=3.338 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 39.633=9.992 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 39.633=22.668 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=10 \%$ of $9.992 \mathrm{~kg}=0.999 \mathrm{~kg}$
So,
Quantity of Fine Aggregate required $=9.992-0.999=8.993 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 22.668 kg

$$
=0.114 \mathrm{~kg}
$$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.993 kg

$$
=0.0899 \mathrm{~kg}
$$

Total extra water added $=0.114+0.0899=0.204 \mathrm{~kg}$
Total water added to Concrete $=3.338+0.204=3.542 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=22.688-0.114=22.574 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=8.993-0.0899=8.903 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 22.574 kg $=4.515 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | C $_{\mathrm{az}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.542 kg | 6.953 kg | 8.903 kg | 18.059 kg | 4.515 kg | 0.999 kg |

## CONCRETE WITH 15\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 150 mm sides with $20 \%$ compensation

$$
=4 \times(150)^{3} \times 1.2=0.0162 \mathrm{~m}^{3}
$$

So,
Mass $=0.0162 \times 24 \times 1000 / 9.81=39.633 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.437+3.263) \times 39.633=6.953 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 39.633=3.338 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 39.633=9.992 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 39.633=22.668 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=15 \%$ of $9.992 \mathrm{~kg}=1.500 \mathrm{~kg}$
So,
Quantity of Fine Aggregate required $=9.992-1.500=8.492 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 22.668 kg $=0.114 \mathrm{~kg}$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.492 kg

$$
=0.0850 \mathrm{~kg}
$$

Total extra water added $=0.114+0.0850=0.199 \mathrm{~kg}$
Total water added to Concrete $=3.338+0.199=3.537 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=22.688-0.114=22.574 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=8.492-0.0850=8.407 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 22.574 kg $=4.515 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | C $_{\mathrm{az}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.537 kg | 6.953 kg | 8.407 kg | 18.059 kg | 4.515 kg | 1.500 kg |

## CONCRETE WITH 20\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 150 mm sides with $20 \%$ compensation

$$
=4 \times(150)^{3} \times 1.2=0.0162 \mathrm{~m}^{3}
$$

So,
Mass $=0.0162 \times 24 \times 1000 / 9.81=39.633 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.437+3.263) \times 39.633=6.953 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 39.633=3.338 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 39.633=9.992 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 39.633=22.668 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=20 \%$ of $9.992 \mathrm{~kg}=1.998 \mathrm{~kg}$
So,
Quantity of Fine Aggregate required $=9.992-1.998=7.994 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 22.668 kg $=0.114 \mathrm{~kg}$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 7.994 kg

$$
=0.0799 \mathrm{~kg}
$$

Total extra water added $=0.114+0.0799=0.194 \mathrm{~kg}$
Total water added to Concrete $=3.338+0.194=3.532 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=22.688-0.114=22.574 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=7.994-0.0799=7.914 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 22.574 kg $=4.515 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | C $_{\mathbf{a z}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.532 kg | 6.953 kg | 7.914 kg | 18.059 kg | 4.515 kg | 1.998 kg |

APPENDIX - A3

## Determination of Coarse Aggregate \& Fine Aggregate

To find compressive strength of Concrete by Accelerated Curing Method, small cubes having 7.05 cm side and coarse aggregate of 10 mm size is used. So, from SP: 23$\mathbf{1 9 8 2}$, for 10 mm size the percentage of voids is $\mathbf{3 . 0 \%}$.

Take 3.0\% Voids from Table 41 of SP: 23-1982, then

$$
\begin{aligned}
\mathrm{V} & =\left(\mathrm{W}+\mathrm{C} / \mathrm{S}_{\mathrm{c}}+1 / \text { p. } \mathrm{f}_{\mathrm{a}} / \mathrm{Sf}_{\mathrm{a}}\right) \times 1 / 1000 \\
0.97 & =\left(180.42+375.87 / 3.05+1 / 0.326 \times \mathrm{f}_{\mathrm{a}} / 2.45\right) \times 1 / 1000 \\
\mathrm{f}_{\mathrm{a}} & =532.174 \mathrm{~kg} / \mathrm{m}^{3}
\end{aligned}
$$

Similarly,

$$
\begin{aligned}
& \mathrm{V}=\left(\mathrm{W}+\mathrm{C} / \mathrm{S}_{\mathrm{c}}+1 / 1-\mathrm{p} . \mathrm{C}_{\mathrm{az}} / \mathrm{S}_{\mathrm{caz}}\right) \times 1 / 1000 \\
& 980=\left(180.42+375.87 / 3.05+1 / 0.674 \times \mathrm{C}_{\mathrm{az}} / 2.70\right) \\
& \mathrm{C}_{\mathrm{az}}=1212.532 \mathrm{~kg} / \mathrm{m}^{3}
\end{aligned}
$$

So,
Mix Proportion is

| Water | Cement | Fine Aggregate | Coarse Aggregate |
| :---: | :---: | :---: | :---: |
| 180.42 kg | 375.87 kg | 532.174 kg | 1212.532 kg |
| 0.480 | 1.0 | 1.146 | 3.226 |

## CONTROL MIX

Volume of Concrete required to cast 4 Cubes of 7.05 mm sides with $20 \%$ compensation

$$
=4 \times(0.0705)^{3} \times 1.2=0.00168 \mathrm{~m}^{3}
$$

So,
Mass $=0.00168 \times 24 \times 1000 / 9.81=4.115 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.416+3.226) \times 4.115=0.730 \mathrm{~kg}$
Water Required $=0.480 /(1+1.416+3.226) \times 4.115=0.350 \mathrm{~kg}$
Fine Aggregate Required $=1.416 /(1+1.416+3.226) \times 4.115=1.033 \mathrm{~kg}$
Coarse Aggregate Required $=3.226 /(1+1.416+3.226) \times 4.115=2.353 \mathrm{~kg}$
Now,
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 2.353 kg $=0.0118 \mathrm{~kg}$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 1.033 kg

$$
=0.0103 \mathrm{~kg}
$$

Total extra water added $=0.0118+0.0103=0.0220 \mathrm{~kg}$
Total water added to Concrete $=0.350+0.0220=0.372 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=2.353-0.0118=2.341 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=1.033-0.0103=1.023 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate |
| :---: | :---: | :---: | :---: |
| 0.372 kg | 0.730 kg | 1.023 kg | 2.341 kg |

## CONCRETE WITH 5\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 7.05 mm sides with $20 \%$ compensation

$$
=4 \times(0.0705)^{3} \times 1.2=0.00168 \mathrm{~m}^{3}
$$

So,
Mass $=0.00168 \times 24 \times 1000 / 9.81=4.115 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.416+3.226) \times 4.115=0.730 \mathrm{~kg}$
Water Required $=0.480 /(1+1.416+3.226) \times 4.115=0.350 \mathrm{~kg}$
Fine Aggregate Required $=1.416 /(1+1.416+3.226) \times 4.115=1.033 \mathrm{~kg}$
Coarse Aggregate Required $=3.226 /(1+1.416+3.226) \times 4.115=2.353 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=5 \%$ of $1.033 \mathrm{~kg}=0.052 \mathrm{~kg}$
So,
Quantity of Fine Aggregate added $=1.033-0.052=0.982 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 2.353 kg

$$
=0.0118 \mathrm{~kg}
$$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 0.982 kg $=0.00982 \mathrm{~kg}$

Total extra water added $=0.0118+0.00982=0.0216 \mathrm{~kg}$
Total water added to Concrete $=0.350+0.0216=0.372 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=2.353-0.0118=2.341 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=0.982-0.00982=0.972 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | Crumbs |
| :---: | :---: | :---: | :---: | :---: |
| 0.372 kg | 0.730 kg | 0.972 kg | 2.342 kg | 0.0520 kg |

## CONCRETE WITH 10\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 7.05 mm sides with $20 \%$ compensation

$$
=4 \times(0.0705)^{3} \times 1.2=0.00168 \mathrm{~m}^{3}
$$

So,
Mass $=0.00168 \times 24 \times 1000 / 9.81=4.115 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.416+3.226) \times 4.115=0.730 \mathrm{~kg}$
Water Required $=0.480 /(1+1.416+3.226) \times 4.115=0.350 \mathrm{~kg}$
Fine Aggregate Required $=1.416 /(1+1.416+3.226) \times 4.115=1.033 \mathrm{~kg}$
Coarse Aggregate Required $=3.226 /(1+1.416+3.226) \times 4.115=2.353 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=10 \%$ of $1.033 \mathrm{~kg}=0.104 \mathrm{~kg}$
So,
Quantity of Fine Aggregate added $=1.033-0.104=0.929 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 2.353 kg

$$
=0.0118 \mathrm{~kg}
$$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 0.982 kg $=0.00929 \mathrm{~kg}$

Total extra water added $=0.0118+0.00929=0.0211 \mathrm{~kg}$
Total water added to Concrete $=0.350+0.0211=0.371 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=2.353-0.0118=2.342 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=0.929-0.00929=0.920 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | Crumbs |
| :---: | :---: | :---: | :---: | :---: |
| 0.372 kg | 0.730 kg | 0.920 kg | 2.342 kg | 0.104 kg |

## CONCRETE WITH 15\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 7.05 mm sides with $20 \%$ compensation

$$
=4 \times(0.0705)^{3} \times 1.2=0.00168 \mathrm{~m}^{3}
$$

So,
Mass $=0.00168 \times 24 \times 1000 / 9.81=4.115 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.416+3.226) \times 4.115=0.730 \mathrm{~kg}$
Water Required $=0.480 /(1+1.416+3.226) \times 4.115=0.350 \mathrm{~kg}$
Fine Aggregate Required $=1.416 /(1+1.416+3.226) \times 4.115=1.033 \mathrm{~kg}$
Coarse Aggregate Required $=3.226 /(1+1.416+3.226) \times 4.115=2.353 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=15 \%$ of $1.033 \mathrm{~kg}=0.155 \mathrm{~kg}$
So,
Quantity of Fine Aggregate added $=1.033-0.155=0.878 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 2.353 kg $=0.0118 \mathrm{~kg}$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 0.878 kg $=0.00878 \mathrm{~kg}$

Total extra water added $=0.0118+0.00878=0.021 \mathrm{~kg}$
Total water added to Concrete $=0.350+0.021=0.371 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=2.353-0.0118=2.342 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=0.878-0.00878=0.869 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | Crumbs |
| :---: | :---: | :---: | :---: | :---: |
| 0.372 kg | 0.730 kg | 0.869 kg | 2.342 kg | 0.155 kg |

## CONCRETE WITH 20\% CRUMBS

Volume of Concrete required to cast 4 Cubes of 7.05 mm sides with $20 \%$ compensation

$$
=4 \times(0.0705)^{3} \times 1.2=0.00168 \mathrm{~m}^{3}
$$

So,
Mass $=0.00168 \times 24 \times 1000 / 9.81=4.115 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.416+3.226) \times 4.115=0.730 \mathrm{~kg}$
Water Required $=0.480 /(1+1.416+3.226) \times 4.115=0.350 \mathrm{~kg}$
Fine Aggregate Required $=1.416 /(1+1.416+3.226) \times 4.115=1.033 \mathrm{~kg}$
Coarse Aggregate Required $=3.226 /(1+1.416+3.226) \times 4.115=2.353 \mathrm{~kg}$
Now,
Quantity of Crumbs added $=20 \%$ of $1.033 \mathrm{~kg}=0.207 \mathrm{~kg}$
So,
Quantity of Fine Aggregate added $=1.033-0.207=0.827 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 2.353 kg $=0.0118 \mathrm{~kg}$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 0.827 kg $=0.00827 \mathrm{~kg}$

Total extra water added $=0.0118+0.00827=0.0200 \mathrm{~kg}$
Total water added to Concrete $=0.350+0.0200=0.370 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=2.353-0.0118=2.342 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=0.827-0.00827=0.819 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | Crumbs |
| :---: | :---: | :---: | :---: | :---: |
| 0.370 kg | 0.730 kg | 0.819 kg | 2.342 kg | 0.207 kg |

## APPENDIX - A4

## CONTROL MIX

Volume of Concrete required for Casting 3 Cylinders of 150 mm Diameter \& 300 mm Height with $20 \%$ compensation

$$
\begin{aligned}
& =3\left[\pi(0.15)^{2} / 4 \times 0.30\right] \times 1.2 \\
& =0.0191 \mathrm{~m}^{3}
\end{aligned}
$$

Therefore,

$$
\text { Mass }=[24 \times 1000 / 9.81] \times 0.0191 \mathrm{~m}^{3}=46.729 \mathrm{~kg}
$$

Then,
Cement Required $=1 /(1+1.437+3.263) \times 46.729=8.198 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 46.729=3.935 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 46.729=11.780 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 46.729=26.749 \mathrm{~kg}$ Now,

Extra Water added for the absorption of Fine Aggregate $=1.0 \%$ of $11.780 \mathrm{~kg}=0.118$ kg

Extra Water added for the absorption of Coarse Aggregate $=0.5 \%$ of 26.749 kg

$$
=0.134 \mathrm{~kg}
$$

Then,
Total Extra Water added $=0.118+0.134=0.252 \mathrm{~kg}$
Also,
Total Water added to the Concrete $=3.935+0.252=4.187 \mathrm{~kg}$
Actual Quantity of Fine aggregate added $=11.780-0.118=11.662 \mathrm{~kg}$
Actual Quantity of Coarse aggregate added $=26.749-0.134=26.615 \mathrm{~kg}$
Coarse aggregate of 10 mm size required $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of $26.615 \mathrm{~kg}=5.323 \mathrm{~kg}$ So,

Coarse Aggregate added $=26.615-5.323=21.292 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ |
| :---: | :---: | :---: | :---: | :---: |
| 4.187 kg | 8.198 kg | 11.662 kg | 21.292 kg | 5.323 kg |

Volume of Concrete required for Casting 3 Cylinders of 150 mm Diameter \& 300 mm Height with $20 \%$ compensation

$$
\begin{aligned}
& =3\left[\pi(0.15)^{2} / 4 \times 0.30\right] \times 1.2 \\
& =0.0191 \mathrm{~m}^{3}
\end{aligned}
$$

Therefore,

$$
\text { Mass }=[24 \times 1000 / 9.81] \times 0.0191 \mathrm{~m}^{3}=46.729 \mathrm{~kg}
$$

Then,
Cement Required $=1 /(1+1.437+3.263) \times 46.729=8.198 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 46.729=3.935 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 46.729=11.780 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 46.729=26.749 \mathrm{~kg}$
Now,
Crumbs added $=5 \%$ of $11.780 \mathrm{~kg}=0.589 \mathrm{~kg}$
So,
Fine Aggregate added $=11.780-0.589=11.191 \mathrm{~kg}$
Extra Water added for the absorption of Fine Aggregate $=1.0 \%$ of $11.191 \mathrm{~kg}=0.112$ kg

Extra Water added for the absorption of Coarse Aggregate $=0.5 \%$ of 26.749 kg

$$
=0.134 \mathrm{~kg}
$$

Then,
Total Extra Water added $=0.112+0.134=0.246 \mathrm{~kg}$
Also,
Total Water added to the Concrete $=3.935+0.246=4.181 \mathrm{~kg}$
Actual Quantity of Fine aggregate added $=11.191-0.112=11.079 \mathrm{~kg}$
Actual Quantity of Coarse aggregate added $=26.749-0.134=26.615 \mathrm{~kg}$
Coarse aggregate of 10 mm size required $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of $26.615 \mathrm{~kg}=5.323 \mathrm{~kg}$ So,

Coarse Aggregate added $=26.615-5.323=21.292 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4.181 kg | 8.198 kg | 11.079 kg | 21.292 kg | 5.323 kg | 0.589 kg |

## CONCRETE WITH 10\% CRUMBS

Volume of Concrete required for Casting 3 Cylinders of 150 mm Diameter \& 300 mm Height with $20 \%$ compensation

$$
\begin{aligned}
& =3\left[\pi(0.15)^{2} / 4 \times 0.30\right] \times 1.2 \\
& =0.0191 \mathrm{~m}^{3}
\end{aligned}
$$

Therefore,

$$
\text { Mass }=[24 \times 1000 / 9.81] \times 0.0191 \mathrm{~m}^{3}=46.729 \mathrm{~kg}
$$

Then,
Cement Required $=1 /(1+1.437+3.263) \times 46.729=8.198 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 46.729=3.935 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 46.729=11.780 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 46.729=26.749 \mathrm{~kg}$
Now,
Crumbs added $=10 \%$ of $11.780 \mathrm{~kg}=1.178 \mathrm{~kg}$
So,
Fine Aggregate added $=11.780-1.178=10.602 \mathrm{~kg}$
Extra Water added for the absorption of Fine Aggregate $=1.0 \%$ of $10.602 \mathrm{~kg}=0.106$ kg

Extra Water added for the absorption of Coarse Aggregate $=0.5 \%$ of 26.749 kg

$$
=0.134 \mathrm{~kg}
$$

Then,
Total Extra Water added $=0.106+0.134=0.240 \mathrm{~kg}$
Also,
Total Water added to the Concrete $=3.935+0.240=4.175 \mathrm{~kg}$
Actual Quantity of Fine aggregate added $=10.602-0.106=10.496 \mathrm{~kg}$
Actual Quantity of Coarse aggregate added $=26.749-0.134=26.615 \mathrm{~kg}$
Coarse aggregate of 10 mm size required $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of $26.615 \mathrm{~kg}=5.323 \mathrm{~kg}$ So,

Coarse Aggregate added $=26.615-5.323=21.292 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4.175 kg | 8.198 kg | 10.496 kg | 21.292 kg | 5.323 kg | 1.178 kg |

## CONCRETE WITH 15\% CRUMBS

Volume of Concrete required for Casting 3 Cylinders of 150 mm Diameter \& 300mm Height with $20 \%$ compensation

$$
\begin{aligned}
& =3\left[\pi(0.15)^{2} / 4 \times 0.30\right] \times 1.2 \\
& =0.0191 \mathrm{~m}^{3}
\end{aligned}
$$

Therefore,

$$
\text { Mass }=[24 \times 1000 / 9.81] \times 0.0191 \mathrm{~m}^{3}=46.729 \mathrm{~kg}
$$

Then,
Cement Required $=1 /(1+1.437+3.263) \times 46.729=8.198 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 46.729=3.935 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 46.729=11.780 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 46.729=26.749 \mathrm{~kg}$
Now,
Crumbs added $=15 \%$ of $11.780 \mathrm{~kg}=1.767 \mathrm{~kg}$
So,
Fine Aggregate added $=11.780-1.767=10.013 \mathrm{~kg}$
Extra Water added for the absorption of Fine Aggregate $=1.0 \%$ of $10.013 \mathrm{~kg}=0.100$ kg

Extra Water added for the absorption of Coarse Aggregate $=0.5 \%$ of 26.749 kg

$$
=0.134 \mathrm{~kg}
$$

Then,
Total Extra Water added $=0.100+0.134=0.234 \mathrm{~kg}$
Also,
Total Water added to the Concrete $=3.935+0.234=4.169 \mathrm{~kg}$
Actual Quantity of Fine aggregate added $=10.013-0.100=9.913 \mathrm{~kg}$
Actual Quantity of Coarse aggregate added $=26.749-0.134=26.615 \mathrm{~kg}$
Coarse aggregate of 10 mm size required $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of $26.615 \mathrm{~kg}=5.323 \mathrm{~kg}$ So,

Coarse Aggregate added $=26.615-5.323=21.292 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4.169 kg | 8.198 kg | 9.913 kg | 21.292 kg | 5.323 kg | 1.767 kg |

## CONCRETE WITH 20\% CRUMBS

Volume of Concrete required for Casting 3 Cylinders of 150 mm Diameter \& 300 mm Height with $20 \%$ compensation

$$
\begin{aligned}
& =3\left[\pi(0.15)^{2} / 4 \times 0.30\right] \times 1.2 \\
& =0.0191 \mathrm{~m}^{3}
\end{aligned}
$$

Therefore,

$$
\text { Mass }=[24 \times 1000 / 9.81] \times 0.0191 \mathrm{~m}^{3}=46.729 \mathrm{~kg}
$$

Then,

Cement Required $=1 /(1+1.437+3.263) \times 46.729=8.198 \mathrm{~kg}$
Water Required $=0.480 /(1+1.437+3.263) \times 46.729=3.935 \mathrm{~kg}$
Fine Aggregate Required $=1.437 /(1+1.437+3.263) \times 46.729=11.780 \mathrm{~kg}$
Coarse Aggregate Required $=3.263 /(1+1.437+3.263) \times 46.729=26.749 \mathrm{~kg}$
Now,
Crumbs added $=20 \%$ of $11.780 \mathrm{~kg}=2.356 \mathrm{~kg}$
So,
Fine Aggregate added $=11.780-2.356=9.424 \mathrm{~kg}$
Extra Water added for the absorption of Fine Aggregate $=1.0 \%$ of $9.424 \mathrm{~kg}=0.0943$ kg

Extra Water added for the absorption of Coarse Aggregate $=0.5 \%$ of 26.749 kg $=0.134 \mathrm{~kg}$
Then,
Total Extra Water added $=0.0943+0.134=0.228 \mathrm{~kg}$
Also,
Total Water added to the Concrete $=3.935+0.228=4.163 \mathrm{~kg}$
Actual Quantity of Fine aggregate added $=9.424-0.0943=9.329 \mathrm{~kg}$
Actual Quantity of Coarse aggregate added $=26.749-0.134=26.615 \mathrm{~kg}$
Coarse aggregate of 10 mm size required $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of $26.615 \mathrm{~kg}=5.323 \mathrm{~kg}$ So,

Coarse Aggregate added $=26.615-5.323=21.292 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | C $_{\mathrm{az}}$ | Crumbs |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4.163 kg | 8.198 kg | 9.329 kg | 21.292 kg | 5.323 kg | 2.356 kg |

## APPENDIX - A5

## $\underline{\text { Determination of Coarse \& Fine Aggregate with Shriram Cement }}$

Take $2.0 \%$ Voids, then

$$
\begin{aligned}
& \mathrm{V}=\left(\mathrm{W}+\mathrm{C} / \mathrm{S}_{\mathrm{c}}+1 / \mathrm{p} . \mathrm{f}_{\mathrm{a}} / \mathrm{Sf}_{\mathrm{a}}\right) \times 1 / 1000 \\
& 0.98=\left(180.42+375.87 / 3.10+1 / 0.326 \times \mathrm{f}_{\mathrm{a}} / 2.45\right) \times \\
& 1 / 1000 \\
& \mathrm{f}_{\mathrm{a}}=541.78 \mathrm{~kg} / \mathrm{m}^{3}
\end{aligned}
$$

Similarly,

$$
\begin{aligned}
\mathrm{V} & =\left(\mathrm{W}+\mathrm{C} / \mathrm{S}_{\mathrm{c}}+1 / 1-\mathrm{p} . \mathrm{C}_{\mathrm{a}} / \mathrm{S}_{\mathrm{ca}}\right) \times 1 / 1000 \\
980 & =\left(180.42+375.87 / 3.10+1 / 0.674 \times \mathrm{C}_{\mathrm{a}} / 2.69\right) \\
\mathrm{C}_{\mathrm{a}} & =1229.86 \mathrm{~kg} / \mathrm{m}^{3}
\end{aligned}
$$

So,
Mix Proportion is

| Water | Cement | Fine Aggregate | Coarse <br> Aggregate |
| :---: | :---: | :---: | :---: |
| 180.42 kg | 375.87 kg | 541.78 kg | 1229.86 kg |
| 0.480 | 1.0 | 1.442 | 3.272 |

## CONTROL MIX

Volume of Concrete required to cast 4 Cubes of 150 mm sides with $10 \%$ compensation

$$
=4 \times(150)^{3} \times 1.1=0.0149 \mathrm{~m}^{3}
$$

So,
Mass $=0.0149 \times 24 \times 1000 / 9.81=36.330 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.442+3.272) \times 36.330=6.358 \mathrm{~kg}$
Water Required $=0.480 /(1+1.442+3.272) \times 36.330=3.052 \mathrm{~kg}$
Fine Aggregate Required $=1.442 /(1+1.442+3.272) \times 36.330=9.169 \mathrm{~kg}$
Coarse Aggregate Required $=3.272 /(1+1.442+3.272) \times 36.330=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.169 kg $=0.0917 \mathrm{~kg}$

Total extra water added $=0.104+0.0197=0.196 \mathrm{~kg}$
Total water added to Concrete $=3.052+0.196=3.248 \mathrm{~kg}$
Therefore,
Actual quantity of Coarse Aggregate required $=20.804-0.104=20.700 \mathrm{~kg}$
Actual quantity of Fine Aggregate required $=9.169-0.0197=9.077 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg

$$
=4.14 \mathrm{~kg}
$$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ |
| :---: | :---: | :---: | :---: | :---: |
| 3.248 kg | 6.358 kg | 9.077 kg | 16.560 kg | 4.140 kg |

## CONCRETE WITHOUT CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.169 kg $=0.0917 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg $=0.104 \mathrm{~kg}$

Total Extra Water added $=0.0917+0.104=0.196 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.196=3.248 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=9.169-0.0917=9.077 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $6.358=0.0318 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0318=6.326 \mathrm{~kg}$
Net quantity of Water added $=3.248-0.0318=3.216 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.216 kg | 6.326 kg | 9.077 kg | 16.560 kg | 4.140 kg | 0.0318 kg |

## CONCRETE WITHOUT CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.169 kg $=0.0917 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg $=0.104 \mathrm{~kg}$

Total Extra Water added $=0.0917+0.104=0.196 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.196=3.248 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=9.169-0.0917=9.077 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $6.358=0.0636 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0636=6.295 \mathrm{~kg}$
Net quantity of Water added $=3.248-0.0636=3.185 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.185 kg | 6.295 kg | 9.077 kg | 16.560 kg | 4.140 kg | 0.0636 kg |

## CONCRETE WITHOUT CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.169 kg $=0.0917 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg $=0.104 \mathrm{~kg}$

Total Extra Water added $=0.0917+0.104=0.196 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.196=3.248 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=9.169-0.0917=9.077 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $6.358=0.0954 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0954=6.263 \mathrm{~kg}$
Net quantity of Water added $=3.248-0.0954=3.153 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.153 kg | 6.263 kg | 9.077 kg | 16.560 kg | 4.140 kg | 0.0954 kg |

## CONCRETE WITH 5\% CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $9.169 \mathrm{~kg}=0.458 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=9.169-0.458=8.710 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.710 kg $=0.0871 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Total Extra Water added $=0.0871+0.104=0.191 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.191=3.243 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=8.710-0.0871=8.623 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $6.358=0.0318 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0318=6.326 \mathrm{~kg}$
Net quantity of Water added $=3.243-0.0318=3.211 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.211 kg | 6.326 kg | 8.623 kg | 16.560 kg | 4.140 kg | 0.458 kg | 0.0318 kg |

## CONCRETE WITH 5\% CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $9.169 \mathrm{~kg}=0.458 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=9.169-0.458=8.710 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.710 kg $=0.0871 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Total Extra Water added $=0.0871+0.104=0.191 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.191=3.243 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=8.710-0.0871=8.623 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $6.358=0.0636 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0636=6.295 \mathrm{~kg}$
Net quantity of Water added $=3.243-0.0636=3.180 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.180 kg | 6.295 kg | 8.623 kg | 16.560 kg | 4.140 kg | 0.458 kg | 0.0636 kg |

## CONCRETE WITH 5\% CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $9.169 \mathrm{~kg}=0.458 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=9.169-0.458=8.710 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.710 kg $=0.0871 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Total Extra Water added $=0.0871+0.104=0.191 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.191=3.243 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=8.710-0.0871=8.623 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $6.358=0.0954 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0954=6.263 \mathrm{~kg}$
Net quantity of Water added $=3.243-0.0954=3.148 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.148 kg | 6.263 kg | 8.623 kg | 16.560 kg | 4.140 kg | 0.458 kg | 0.0954 kg |

## CONCRETE WITH 10\% CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Crumbs added $=10 \%$ of $9.169 \mathrm{~kg}=0.917 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=9.169-0.917=8.252 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.252 kg $=0.0825 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Total Extra Water added $=0.0825+0.104=0.186 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.186=3.239 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=8.252-0.0825=8.170 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $6.358=0.0318 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0318=6.326 \mathrm{~kg}$
Net quantity of Water added $=3.239-0.0318=3.207 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.207 kg | 6.326 kg | 8.170 kg | 16.560 kg | 4.140 kg | 0.917 kg | 0.0318 kg |

## CONCRETE WITH 10\% CRUMBS \& 1.0 \% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Crumbs added $=10 \%$ of $9.169 \mathrm{~kg}=0.917 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=9.169-0.917=8.252 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.252 kg $=0.0825 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Total Extra Water added $=0.0825+0.104=0.186 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.186=3.239 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=8.252-0.0825=8.170 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $6.358=0.0636 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0636=6.294 \mathrm{~kg}$
Net quantity of Water added $=3.239-0.0636=3.176 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.176 kg | 6.294 kg | 8.170 kg | 16.560 kg | 4.140 kg | 0.917 kg | 0.0636 kg |

## CONCRETE WITH 10\% CRUMBS \& 1.5 \% ADMIXTURE

Quantity of Fine Aggregate Required $=9.169 \mathrm{~kg}$
Quantity of Crumbs added $=10 \%$ of $9.169 \mathrm{~kg}=0.917 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=9.169-0.917=8.252 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.804 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 8.252 kg $=0.0825 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.804 kg

$$
=0.104 \mathrm{~kg}
$$

Total Extra Water added $=0.0825+0.104=0.186 \mathrm{~kg}$
Total quantity of Water added $=3.052+0.186=3.239 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=8.252-0.0825=8.170 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.804-0.104=20.700 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 20.700 kg $=4.140 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.700-4.140=16.560 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $6.358=0.0954 \mathrm{~kg}$
So,
Net quantity of Cement added $=6.358-0.0954=6.263 \mathrm{~kg}$
Net quantity of Water added $=3.239-0.0954=3.144 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.144 kg | 6.263 kg | 8.170 kg | 16.560 kg | 4.140 kg | 0.917 kg | 0.0954 kg |

## APPENDIX - A6

## CONTROL MIX

Volume of Concrete required for Casting 3 Cylinders of 150 mm Diameter \& 300 mm Height with $10 \%$ compensation

$$
\begin{aligned}
& =3\left[\pi(0.15)^{2} / 4 \times 0.30\right] \times 1.1 \\
& =0.0175 \mathrm{~m}^{3}
\end{aligned}
$$

Therefore,

$$
\text { Mass }=[24 \times 1000 / 9.81] \times 0.0175 \mathrm{~m}^{3}=42.800 \mathrm{~kg}
$$

Then,
Cement Required $=1 /(1+1.442+3.272) \times 42.800=7.490 \mathrm{~kg}$
Water Required $=0.480 /(1+1.442+3.272) \times 42.800=3.595 \mathrm{~kg}$
Fine Aggregate Required $=1.442 /(1+1.442+3.272) \times 42.800=10.801 \mathrm{~kg}$
Coarse Aggregate Required $=3.272 /(1+1.442+3.272) \times 42.800=24.509 \mathrm{~kg}$
Extra Water added for the absorption of Fine Aggregate $=1.0 \%$ of $10.801 \mathrm{~kg}=0.108$ kg

Extra Water added for the absorption of Coarse Aggregate $=0.5 \%$ of 24.509 kg

$$
=0.123 \mathrm{~kg}
$$

Then,
Total Extra Water added $=0.108+0.123=0.231 \mathrm{~kg}$
Also,
Total Water added to the Concrete $=3.595+0.231=3.826 \mathrm{~kg}$
Actual Quantity of Fine aggregate added $=10.801-0.108=10.693 \mathrm{~kg}$
Actual Quantity of Coarse aggregate added $=24.509-0.123=24.386 \mathrm{~kg}$
Coarse aggregate of 10 mm size required $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of $24.386 \mathrm{~kg}=4.877 \mathrm{~kg}$ So,

Coarse Aggregate added $=24.386-4.877=19.509 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ |
| :---: | :---: | :---: | :---: | :---: |
| 3.826 kg | 7.490 kg | 10.693 kg | 19.509 kg | 4.877 kg |

## CONCRETE WITH 5\% CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $10.801 \mathrm{~kg}=0.540 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=10.801-0.540=10.261 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.509 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 10.261 kg $=0.103 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg

$$
=0.123 \mathrm{~kg}
$$

Total Extra Water added $=0.103+0.123=0.226 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.226=3.820 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=10.261-0.103=10.158 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $7.490=0.0375 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.0375=7.453 \mathrm{~kg}$
Net quantity of Water added $=3.820-0.0375=3.783 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.783 kg | 7.453 kg | 10.158 kg | 19.509 kg | 4.877 kg | 0.540 kg | 0.0375 kg |

## CONCRETE WITH 5\% CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $10.801 \mathrm{~kg}=0.540 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=10.801-0.540=10.261 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.509 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 10.261 kg $=0.103 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg

$$
=0.123 \mathrm{~kg}
$$

Total Extra Water added $=0.103+0.123=0.226 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.226=3.820 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=10.261-0.103=10.158 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $7.490=0.0749 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.0749=7.415 \mathrm{~kg}$
Net quantity of Water added $=3.820-0.0749=3.745 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.745 kg | 7.415 kg | 10.158 kg | 19.509 kg | 4.877 kg | 0.540 kg | 0.0749 kg |

## CONCRETE WITH 5\% CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $10.801 \mathrm{~kg}=0.540 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=10.801-0.540=10.261 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.509 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 10.261 kg

$$
=0.103 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg

$$
=0.123 \mathrm{~kg}
$$

Total Extra Water added $=0.103+0.123=0.226 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.226=3.820 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=10.261-0.103=10.158 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $7.490=0.113 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.113=7.378 \mathrm{~kg}$
Net quantity of Water added $=3.820-0.113=3.707 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.707 kg | 7.378 kg | 10.158 kg | 19.509 kg | 4.877 kg | 0.540 kg | 0.113 kg |

## CONCRETE WITH 10\% CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Quantity of Crumbs added $=10.0 \%$ of $10.801 \mathrm{~kg}=1.080 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=10.801-1.080=9.720 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.509 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.720 kg $=0.0972 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg

$$
=0.123 \mathrm{~kg}
$$

Total Extra Water added $=0.0972+0.123=0.219 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.219=3.814 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=9.720-0.0972=9.623 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $7.490=0.0375 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.0375=7.453 \mathrm{~kg}$
Net quantity of Water added $=3.814-0.0375=3.777 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.777 kg | 7.453 kg | 9.623 kg | 19.509 kg | 4.877 kg | 1.080 kg | 0.0375 kg |

## CONCRETE WITH 10\% CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Quantity of Crumbs added $=10.0 \%$ of $10.801 \mathrm{~kg}=1.080 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=10.801-1.080=9.720 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.509 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.720 kg

$$
=0.0972 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg $=0.123 \mathrm{~kg}$

Total Extra Water added $=0.0972+0.123=0.219 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.219=3.814 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=9.720-0.0972=9.623 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $7.490=0.0749 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.0749=7.415 \mathrm{~kg}$
Net quantity of Water added $=3.814-0.0749=3.739 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.739 kg | 7.415 kg | 9.623 kg | 19.509 kg | 4.877 kg | 1.080 kg | 0.0749 kg |

## CONCRETE WITH 10\% CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Quantity of Crumbs added $=10.0 \%$ of $10.801 \mathrm{~kg}=1.080 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=10.801-1.080=9.720 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=20.509 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 9.720 kg $=0.0972 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg

$$
=0.123 \mathrm{~kg}
$$

Total Extra Water added $=0.0972+0.123=0.219 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.219=3.814 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=9.720-0.0972=9.623 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $7.490=0.113 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.113=7.378 \mathrm{~kg}$
Net quantity of Water added $=3.814-0.113=3.702 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.702 kg | 7.378 kg | 9.623 kg | 19.509 kg | 4.877 kg | 1.080 kg | 0.113 kg |

## CONCRETE WITHOUT CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 10.801 kg

$$
=0.108 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg $=0.123 \mathrm{~kg}$

Total Extra Water added $=0.108+0.123=0.231 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.231=3.826 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=10.801-0.108=10.693 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $7.490=0.0375 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.0375=7.453 \mathrm{~kg}$
Net quantity of Water added $=3.814-0.0375=3.777 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.777 kg | 7.453 kg | 10.693 kg | 19.509 kg | 4.877 kg | 0.0375 kg |

## CONCRETE WITHOUT CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 10.801 kg

$$
=0.108 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg $=0.123 \mathrm{~kg}$

Total Extra Water added $=0.108+0.123=0.231 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.231=3.826 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=10.801-0.108=10.693 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $7.490=0.0749 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.0749=7.415 \mathrm{~kg}$
Net quantity of Water added $=3.814-0.0749=3.739 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.739 kg | 7.415 kg | 10.693 kg | 19.509 kg | 4.877 kg | 0.0749 kg |

## CONCRETE WITHOUT CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=10.801 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 10.801 kg

$$
=0.108 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 20.509 kg $=0.123 \mathrm{~kg}$

Total Extra Water added $=0.108+0.123=0.231 \mathrm{~kg}$
Total quantity of Water added $=3.595+0.231=3.826 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=10.801-0.108=10.693 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=20.509-0.123=24.386 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 24.386 kg $=4.877 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=20.386-4.877=19.509 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $7.490=0.113 \mathrm{~kg}$
So,
Net quantity of Cement added $=7.490-0.113=7.378 \mathrm{~kg}$
Net quantity of Water added $=3.814-0.113=3.702 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 3.702 kg | 7.378 kg | 10.693 kg | 19.509 kg | 4.877 kg | 0.113 kg |

## APPENDIX - A7

## CONTROL MIX

Volume of Concrete required to cast 4 Beam Specimen of size $50 \mathrm{~cm} \times 10 \mathrm{~cm} \times 10 \mathrm{~cm}$ with $10 \%$ compensation.

$$
=4 \times 0.00530 \times 1.1=0.0234 \mathrm{~m} 3
$$

So,
Mass $=0.0234 \times 24 \times 1000 / 9.81=57.079 \mathrm{~kg}$
Then,
Cement Required $=1 /(1+1.442+3.272) \times 57.079=9.989 \mathrm{~kg}$
Water Required $=0.480 /(1+1.442+3.272) \times 57.079=4.795 \mathrm{~kg}$
Fine Aggregate Required $=1.442 /(1+1.442+3.272) \times 57.079=14.405 \mathrm{~kg}$
Coarse Aggregate Required $=3.272 /(1+1.442+3.272) \times 57.079=32.685 \mathrm{~kg}$ Now,

Extra water added for absorption of Fine Aggregate $=1.0 \%$ of $14.405=0.144$ kg

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of $32.685=0.164 \mathrm{~kg}$
So,
Total extra water added in Concrete $=0.308 \mathrm{~kg}$
Actual quantity of water added $=4.795+0.038=5.103 \mathrm{~kg}$
Therefore,
Actual quantity of Fine Aggregate added $=14.405-0.144=14.261 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$

So,
Coarse Aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$

## Mix Proportion

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ |
| :---: | :---: | :---: | :---: | :---: |
| 5.103 kg | 9.989 kg | 14.261 kg | 26.017 kg | 6.504 kg |

## CONCRETE WITHOUT CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 14.405 kg $=0.144 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg $=0.164 \mathrm{~kg}$

Total Extra Water added $=0.144+0.164=0.308 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.308=5.103 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=14.405-0.144=14.261 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $9.989=0.0500 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.0500=9.939 \mathrm{~kg}$
Net quantity of Water added $=5.103-0.0500=5.053 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 5.053 kg | 9.939 kg | 14.261 kg | 26.017 kg | 6.504 kg | 0.0500 kg |

## CONCRETE WITHOUT CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 14.405 kg $=0.144 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg $=0.164 \mathrm{~kg}$

Total Extra Water added $=0.144+0.164=0.308 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.308=5.103 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=14.405-0.144=14.261 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $9.989=0.0999 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.0999=9.889 \mathrm{~kg}$
Net quantity of Water added $=5.103-0.0999=5.003 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 5.003 kg | 9.889 kg | 14.261 kg | 26.017 kg | 6.504 kg | 0.0999 kg |

## CONCRETE WITHOUT CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 14.405 kg $=0.144 \mathrm{~kg}$
Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg $=0.164 \mathrm{~kg}$

Total Extra Water added $=0.144+0.164=0.308 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.308=5.103 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=14.405-0.144=14.261 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $9.989=0.150 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.150=9.839 \mathrm{~kg}$
Net quantity of Water added $=5.103-0.150=4.953 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4.953 kg | 9.839 kg | 14.261 kg | 26.017 kg | 6.504 kg | 0.150 kg |

## CONCRETE WITH 5\% CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $14.405 \mathrm{~kg}=0.720 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=14.405-0.720=13.685 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 13.685 kg

$$
=0.137 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg

$$
=0.164 \mathrm{~kg}
$$

Total Extra Water added $=0.137+0.164=0.300 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.300=5.096 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=13.685-0.137=13.548 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $9.989=0.0500 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.0500=9.939 \mathrm{~kg}$
Net quantity of Water added $=5.096-0.0500=5.046 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5.046 kg | 9.939 kg | 13.548 kg | 26.017 kg | 6.504 kg | 0.720 kg | 0.0500 kg |

## CONCRETE WITH 5\% CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $14.405 \mathrm{~kg}=0.720 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=14.405-0.720=13.685 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 13.685 kg

$$
=0.137 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg

$$
=0.164 \mathrm{~kg}
$$

Total Extra Water added $=0.137+0.164=0.300 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.300=5.096 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=13.685-0.137=13.548 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg

$$
=6.504 \mathrm{~kg}
$$

Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $9.989=0.0999 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.0999=9.889 \mathrm{~kg}$
Net quantity of Water added $=5.096-0.0999=4.996 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4.996 kg | 9.889 kg | 13.548 kg | 26.017 kg | 6.504 kg | 0.720 kg | 0.0999 kg |

## CONCRETE WITH 5\% CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Crumbs added $=5.0 \%$ of $14.405 \mathrm{~kg}=0.720 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=14.405-0.720=13.685 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 13.685 kg

$$
=0.137 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg

$$
=0.164 \mathrm{~kg}
$$

Total Extra Water added $=0.137+0.164=0.300 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.300=5.096 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=13.685-0.137=13.548 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $9.989=0.150 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.150=9.840 \mathrm{~kg}$
Net quantity of Water added $=5.096-0.150=4.946 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4.946 kg | 9.840 kg | 13.548 kg | 26.017 kg | 6.504 kg | 0.720 kg | 0.150 kg |

## CONCRETE WITH 10\% CRUMBS \& 0.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Crumbs added $=10.0 \%$ of $14.405 \mathrm{~kg}=1.440 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=14.405-1.440=12.965 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 12.965 kg

$$
=0.129 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg

$$
=0.164 \mathrm{~kg}
$$

Total Extra Water added $=0.129+0.164=0.293 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.293=5.087 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=12.965-0.129=12.836 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg

$$
=6.504 \mathrm{~kg}
$$

Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=0.5 \%$ of $9.989=0.0500 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.0500=9.939 \mathrm{~kg}$
Net quantity of Water added $=5.096-0.0500=5.046 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5.037 kg | 9.939 kg | 12.836 kg | 26.017 kg | 6.504 kg | 1.440 kg | 0.0500 kg |

## CONCRETE WITH 10\% CRUMBS \& 1.0\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Crumbs added $=10.0 \%$ of $14.405 \mathrm{~kg}=1.440 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=14.405-1.440=12.965 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 12.965 kg

$$
=0.129 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg

$$
=0.164 \mathrm{~kg}
$$

Total Extra Water added $=0.129+0.164=0.293 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.293=5.087 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=12.965-0.129=12.836 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg $=6.504 \mathrm{~kg}$
Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.0 \%$ of $9.989=0.0999 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.0999=9.889 \mathrm{~kg}$
Net quantity of Water added $=5.087-0.0999=4.987 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathbf{a z}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4.987 kg | 9.889 kg | 12.836 kg | 26.017 kg | 6.504 kg | 1.440 kg | 0.0999 kg |

## CONCRETE WITH 10\% CRUMBS \& 1.5\% ADMIXTURE

Quantity of Fine Aggregate Required $=14.405 \mathrm{~kg}$
Quantity of Crumbs added $=10.0 \%$ of $14.405 \mathrm{~kg}=1.440 \mathrm{~kg}$
So,
Net quantity of Fine Aggregate added $=14.405-1.440=12.965 \mathrm{~kg}$
Quantity of Coarse Aggregate Required $=32.685 \mathrm{~kg}$
Now,
Extra water added for absorption of Fine Aggregate $=1.0 \%$ of 12.965 kg

$$
=0.129 \mathrm{~kg}
$$

Extra water added for absorption of Coarse Aggregate $=0.5 \%$ of 32.685 kg

$$
=0.164 \mathrm{~kg}
$$

Total Extra Water added $=0.129+0.164=0.293 \mathrm{~kg}$
Total quantity of Water added $=4.795+0.293=5.087 \mathrm{~kg}$
So,
Actual quantity of Fine Aggregate added $=12.965-0.129=12.836 \mathrm{~kg}$
Actual quantity of Coarse Aggregate added $=32.685-0.164=32.521 \mathrm{~kg}$
Quantity of Coarse Aggregate of 10 mm size added $\left(\mathrm{C}_{\mathrm{az}}\right)=20 \%$ of 32.521 kg

$$
=6.504 \mathrm{~kg}
$$

Therefore,
Coarse aggregate added $=32.521-6.504=26.017 \mathrm{~kg}$
Now,
Quantity of Admixture added $=1.5 \%$ of $9.989=0.150 \mathrm{~kg}$
So,
Net quantity of Cement added $=9.989-0.150=9.839 \mathrm{~kg}$
Net quantity of Water added $=5.096-0.150=4.946 \mathrm{~kg}$

## Mix Proportion:-

| Water | Cement | Fine <br> Aggregate | Coarse <br> Aggregate | $\mathbf{C}_{\mathrm{az}}$ | Crumbs | Admixture |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4.946 kg | 9.839 kg | 12.836 kg | 26.017 kg | 6.504 kg | 1.440 kg | 0.150 kg |

