# COST EVALUATION OF EARTHQUAKE RESISTANT STRUCTURES

A Dissertation Submitted in Partial Fulfillment For the Award of the Degree of

> Master of Engineering in Civil Engineering With specialization in

### STRUCTURAL ENGINEERING

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#### 2004 - 2006

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# **CERTIFICATE**

This is to certify that the project entitled "COST EVALUATION OF EARTHQUAKE RESISTANT STRUCTURES" is being submitted by me (Mr. Anand Singh), is a bonafide record of student's own work carried by him under our guidance and supervision in partial fulfillment of requirement for the award of the Degree of Master of Engineering in Structural Engineering, Department of Civil and Environmental Engineering, Delhi College of Engineering, Delhi, University of Delhi.

The matter embodied in this project has not been submitted for the award of any other degree.

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This is to certify that the above statement made by the candidate is correct of the best of our knowledge.

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### **Acknowledgement**

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### <u>Abstract</u>

In this study an attempt has been made to examine the variation in the cost on structural frame component of the building structures for provision of earthquake resistant features. Various commercial / office buildings with different parameters such as number of stories, place of construction in terms of seismic zone have been considered in the study. These buildings were designed for different loading conditions - for gravity loads only and for the condition when seismic forces also are considered. The cost difference for the provision of ductile detailing as per IS: 13920-1993 has been examined vis-à-vis the application of IS: 456-2000. A set of five school buildings, already constructed, has also been considered. The results as for the variation of cost of these school buildings have been compared with the idealized buildings having the same number of stories in the study. The comparison gives an idea on the additional cost that would incure on the structural frame of buildings for provision of seismic resistance provisions as per Indian codes of practice. Wind load has not been considered in the study. Importance factor is taken 1.5 for design of office building and calculation of cost of building.

# <u>Chapter:1</u> <u>Introduction</u>

Earthquakes are one of the worst type of natural disasters. Millions of lives are lost and other unbearable economic losses take place due to them each year around the world. In a seismic activity most of the losses and casualties take place due to the failures in civil engineering structures, a majority of them are building structures.

In the past building structures have been designed without any consideration of the seismic effects. As the knowledge about the earthquake, their behaviour and their effects on structures grew with time; seismic resistant design procedures have been started being followed in the analysis and design of structures.

Even though it is understood by people that seismic resistant features, if provided in the structures, shall provide added safety against seismic fury; they are often reluctant to provide those features in their buildings. It may be due to their ignorance of the actual quantum of increase in the cost of buildings due to the provisions of earthquake resistant design features.

In the initial phase of the introduction of RC framed structures in the construction industry, the knowledge about earthquakes was inadequate. Their impacts on building structures were not clearly understood. With time as the knowledge about earthquakes grew; earthquake resistant features were applied to building structures to ensure added safety to them.

With time earthquake analysis and design related Indian standard guidelines also came in and various codes of practice are available for this purpose. Out of them the use of the Indian standard codes of practice IS 1893-2002 & IS 13920-1993 (ref. 3) have been made in this study.

In these codes, various guidelines are given to impart earthquake resistant measures to the building structures. Due to the introduction of these features; the cost of building structures may increase by a certain amount.

In this study; an effort has been made to consider a set of building structures to estimate the probable rise, if any, in the cost due to the introduction of earthquake resistant structures. The approach has been to apply the codal guidelines in the design of buildings for imparting earthquake safety. Amounts of concrete and steel reinforcement, calculated in these cases, have been compared with those found out when the buildings are designed for gravity loads only. The findings may provide some information about possible increase in the cost due to the provision of earthquake resistance. Wind load has not been considered in the study. Importance factor is taken 1.5 for design of office building and calculation of cost of building.

# <u>Chapter:2</u> <u>Literature Review</u>

During a seismic event intense lateral forces are applied on to the building structures. Due to these lateral forces all the elements in a RC building structure are subjected to increased amounts of forces. In a building structure which has been designed for gravity loads only, such increase of forces in building elements may not be sustainable. Consequently, various types of damages occur in such buildings.

To reduce damages in the buildings it is important to provide added capacity in the building elements to take on the possible increase of forces due to seismic effects. Various types of earthquake resistant measures have been devised and reported in research literature and standard codes.

The behavior of a building during earthquake depends critically on its overall shape, size, and geometry, in addition to how the earthquake forces are carried to the ground.

The various aspects of earthquake resistant designs are:-

- 1. Sound architectural & structural concept
- 2. Strengthening for seismic lateral forces
- 3. Ductile detailing

The structural system for a building block developed according to sound concept does not result in abnormal additional requirement of concrete/steel. Rather, it reduces the additional cost for the strengthening the structure against the seismic forces, particularly in the structural system, which do not have uniform and symmetrical distribution of stiffness in plan and elevation.

The strengthening for the seismic lateral forces and the provisions for ductile detailing lead to additional cost to the project.

Very little research literature is available about the possible increase in the cost of different types of buildings. In one such study conducted in the case of RC buildings, *Mittal (2001)* has reported the following observations and conclusion as quoted:

- 1. The maximum total additional steel in kg/sq.m. for beams and columns both is of the order of 12 kg/sq.m.
- 2. The additional steel due to lateral forces in beams increases with the increase in number of stories as well as increase in seismic zone coefficient. The pattern of variation of additional steel due to lateral forces in beam follow the pattern of variation for seismic base shear. It is due to the fact that the increase in additional moments in the beams due to lateral force are proportional to the increase in seismic base shear.
- 3. The additional steel due to lateral force in columns decrease with the increase in number of stories but increase with the increase in seismic zone coefficient. The total additional steel due to lateral force in beams and columns increase in no of stories as well as increase in seismic coefficient.
- 4. The additional steel due to ductile detailing in beams decrease with the increase in number of stores as well as increase in seismic zone coefficient. It is due to the fact that as the base shear increases, the additional moment in beams also increases proportionately. It leads to increase in positive and negative steel at joint face and simultaneously decrease in the difference of negative and positive steel.
- 5. Additional steel due to ductile detailing in columns increases with the increase in no of stories. As per IS :13920,the additional steel in columns is provided in the forms of lateral stirrups in confinement zone as well in the middle height of

column. The additional volume of confinement steel in the forms of lateral stirrups also increases with the size of columns.

- 6. The total additional steel required due to the ductile detailing in beams and columns increases marginally with the increase in no of stories.
- 7. Additional steel requirements in beams and columns due to seismic lateral forces is almost 3 times that of the ductile detailing as per IS :13920.
- 8. The total additional steel requirement in beams is about 3 times of the additional steel requirement in columns.
- 9. The order of total additional steel for ductile detailing in beams and columns is about 2.5 kg,approximately for seismic zones III and IV amounts to RS 50 per sq m of total floor area approximately. It is very small sum. As per the principles of earthquake resistant design philosophy, the additional resistance provided by the ductile detailing is many fold that of provided by additional strengthening for seismic lateral forces. It shows the cost effectiveness of the provision for ductile detailing.
- 10. The total additional steel requirement on an average works out to be about 9kg per sq m i.e., about Rs 180/- per sq m for seismic zone III & 11kg per sq m i.e., about Rs 220/- per sq m for seismic zone IV.Assuming total building cost for a building block with average specifications around Rs 7000/-per sq m, the additional cost due to earthquake resistant design provisions works out to be 2.65 % only for seismic zone III & about 3.2 % for seismic zone IV. By extrapolated the values of zone III & IV, the additional cost for zone V will be about 5.3%. The additional cost for ductile detailing alone is about 0.7 % only.

To estimate such additional cost it is required that the building blocks are analyzed designed and detailed for the following conditions:-

- 1. For vertical loads only,
- 2. For vertical loads and lateral loads without ductile detailing and,
- 3. For vertical loads and lateral loads with ductile detailings.

#### 2.1 Seismic Load Analysis

The effect of seismic forces on the building can be analyzed either by the static method or by the dynamic analysis. The fundamental difference between the two methods lies in their approach of calculation of the design seismic force and its distribution to different levels along the height of the structure.

The numerical problem in this report has been analyzed using the Response Spectrum Method. The design spectrum, as given by clause 6.4.2 of IS: 1893-2002, has been used to calculate the design horizontal acceleration spectrum value i.e.  $A_{h}$ .

The horizontal base shear  $V_B$  as per IS 1893-2002 is given by

$$V_B = A_h * W$$

where,

 $A_h$  = design horizontal seismic coefficient

W = effective weight of the structure

The forces and reactions obtained by the STAAD Pro 2004 analysis are used for designing the individual members, beams, columns etc.

#### 2.2 Using STAAD Pro 2004

The analysis and design of building members has been carried out using the structural analysis and design software (STAAD Pro 2004). The methodology used by the software in analysis is discussed below.

#### 2.3 Stiffness Analysis

The stiffness analysis implemented in STAAD Pro 2004 is based on the matrix displacement method. In the matrix analysis of structures by the displacement method, the structure is first idealized into an assembly of discrete structural components (frame members or finite elements). Each component has an assumed form of displacement in a manner which satisfies the force equilibrium and displacement compatibility at the joints.

#### **2.4 Assumption of the Analysis**

For a complete analysis of the structure, the necessary matrices are generated on the basis of the following assumptions:

- 1. The structure is idealized into an assembly of beam, plate and solid type elements joined together at their vertices (nodes). The assemblage is loaded and reacted by concentrated loads acting at the nodes. These loads may be both forces and moments which may act in any specified direction.
- 2. A beam member is a longitudinal structural member having a constant, doubly symmetric or near-doubly symmetric cross section along its length. Beam members always carry axial forces. They may also be subjected to shear and bending in two arbitrary perpendicular planes, and they may also be subjected to torsion.
- 3. Internal and external loads acting on each node are in equilibrium. If torsional or bending properties are defined for any member, six degrees of freedom are considered at each node (i.e. three translational and three rotational) in the generation of relevant matrices. If the member is defined as truss member (i.e. carrying only axial forces) then only the three degrees (translational) of freedom are considered at each node.
- 4. Two types of coordinate systems are used in the generation of the required matrices and are referred to as local and global systems.

#### 2.5 Seismic Load Input in STAAD

The seismic load analysis of the structures can be done using STAAD by employing the response spectrum method of dynamic analysis. The following steps are involved in this process.

- 1. User provides the value for Z\*I/2R as factors for input spectrum.
- 2. Program calculates time periods for first twelve modes.
- 3. Program calculates Sa/g for each mode utilizing time period and damping for each mode.
- 4. The program then calculates mode participation factor for different modes.
- 5. The peak lateral seismic force at each floor in each mode is calculated.
- 6. All response quantities for each mode are calculated.
- The peak response quantities are then combined as per method(CQC or SRSS or ABS or TEN or CSM) as defined by the user to get the final results.

# <u>Chapter:3</u> <u>Numerical Study</u>

In this study an attempt has been made to examine the variation in the cost of a selected component of structural frame of the building structures. Various commercial / office buildings with different parameters such as number of stories, place of construction in terms of seismic zone have been considered in the study. These buildings were designed for different loading conditions - for gravity loads only & for the condition when seismic forces also are considered. The cost difference in view of the provision of ductile detailing as per IS: 13920-1993 also has been examined in addition to the application of IS: 456-2000. A set of five school buildings, already constructed, has also been considered. The results as for the variation of cost of these school buildings have been compared with the idealized buildings having the same number of stories in the study. The comparison gives some idea about the relative effect of provision of seismic resistant measures in buildings for different functions.

The building has been analysed for the following load combinations.

- **1.5\*(Dead Load + Imposed Load).**
- 1.2\*(Dead Load + Imposed Load ± Earthquake Load).
- **1.5\*(Dead Load ± Earthquake Load).**
- 0.9\*Dead Load ± 1.5\*Earthquake Load.

The seismic analysis has been carried in accordance to the IS: 1893 (Part-1)-2002.

In the analysis response spectrum method has been used for seismic analysis of structures. In the response spectrum all buildings has been analysed for each zones. Importance factor has been taken 1.5 for building and soil type chosen is medium soil. For seismic analysis IS 1893:2002 has been used in addition to that help of IS 13920-1993 has also been taken for ductile detailing. For the calculation of dead load &live load

IS 875-1987has been used. The load combinations have been taken from different codes which were mentioned above.

#### 3.1 FOR ONE STOREY:

#### **Building Data:**

Number of Storeys	=	1
Height of the storey	=	3.3 m.
Height of the building	=	5.55m.
Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.3m X 0.3 m
Beam size	=	0.25 m X 0.35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
Load Data:		
<u>Louu Duiu.</u>		
DL	=	4.75 KN /m <sup>2</sup>
LL	=	4 KN /m <sup>2</sup> (For Commercial And Office Build.)

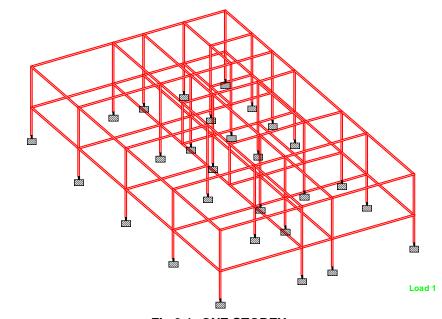


Fig 3.1: ONE STOREY

# **3.2 FOR THREE STOREY:**

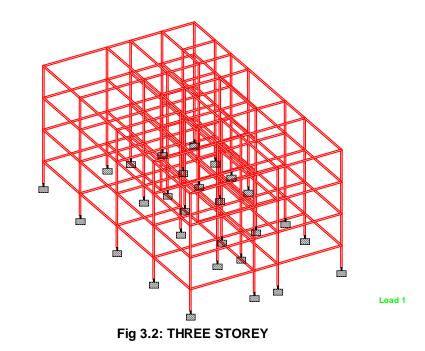
# **Building Data:**

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Number of Storeys	=	3
Height of the storey	=	3.3 m.
Height of the building	=	12.15 m.
Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.3m X 0.3 m
Beam size	=	0.25 m X 0.35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415

Seismic	Data:

Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
Load Data:		
DL	=	4.75 KN /m <sup>2</sup>
LL	=	4 KN /m <sup>2</sup> (For Commercial And Office Build.)

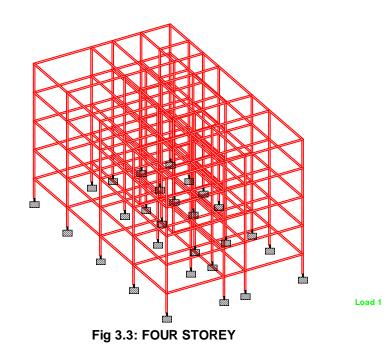


# **3.3 FOR FOUR STOREY:**

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Building Data:		
Number of Storeys	=	4
Height of the storey	=	3.3 m.
Height of the building	=	15.45m.

Length of the building	=	26.40 m.
Width of the building	=	15.65 m.
Column size	=	0.35m X 0.35 m
Beam size	=	0.25 m X 0.35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415
Sajamia Data.		
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
Load Data:		
DL	=	4.75 KN /m <sup>2</sup>



### **<u>3.4 FOR FIVE STOREY:</u>**

### **Building Data:**

Number of Storeys	=	5
Height of the storey	=	3.3 m.
Height of the building	=	18.75m.
Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.35m X 0.35 m
Beam size	=	0.25 m X 0.35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
<u>Load Data:</u>		
DL	=	4.75 KN /m <sup>2</sup>
LL	=	4 KN /m <sup>2</sup> (For Commercial And Office Build.)

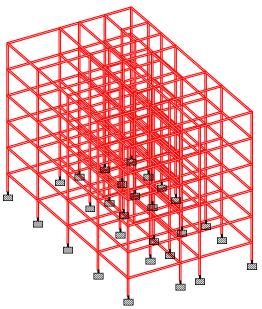


Fig 3.4: FIVE STOREY

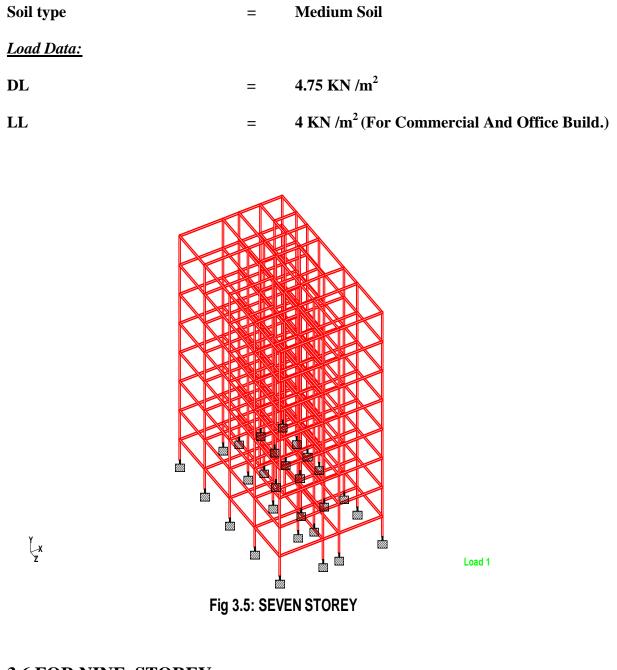
Load 1

# **3.5 FOR SEVEN STOREY:**

#### <u>Building Data:</u>

ζ**x** 

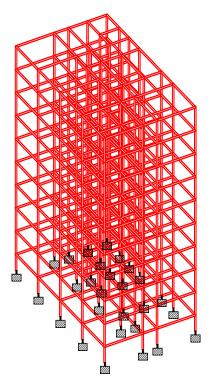
Number of Storeys	=	7
Height of the storey	=	3.3 m.
Height of the building	=	25.35m.
Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.4m X 0.4 m
Beam size	=	0.25 m X 0. 35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V



# **<u>3.6 FOR NINE STOREY:</u>**

<u>Building Data:</u>		
Number of Storeys	=	9
Height of the storey	=	3.3 m.
Height of the building	=	31.95m.

Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.45m X 0.45 m
Beam size	=	0.25 m X 0. 35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
Load Data:		
DL	=	4.75 KN /m <sup>2</sup>



Load 1

Fig 3.6 : NINE STOREY

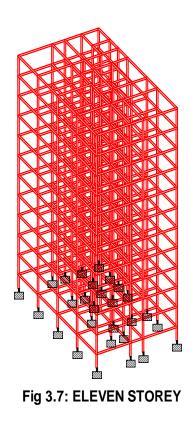
## **<u>3.7 FOR ELEVEN STOREY:</u>**

<u>Building Data:</u>

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Number of Storeys	=	11
Height of the storey	=	3.3 m.
Height of the building	=	38.55m.
Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.5m X 0.5 m
Beam size	=	0.25 m X 0. 35 m
Concrete Grade used	=	M20

Steel Grade used	=	Fe 415
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
Load Data:		
DL	=	4.75 KN /m <sup>2</sup>
LL	=	4 KN /m <sup>2</sup> (For Commercial And Office Build.)

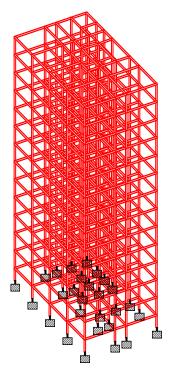


ζ**x** 

Load 1

### **<u>3.8 FOR THIRTEEN STOREY:</u>**

Number of Storeys	=	13
Height of the storey	=	3.3 m.
Height of the building	=	45.15m.
Length of the building	=	26.40m.
Width of the building	=	15.65m.
Column size	=	0.55m X 0.55 m
Beam size	=	0.25 m X 0. 35 m
Concrete Grade used	=	M20
Steel Grade used	=	Fe 415
<u>Seismic Data:</u>		
Seismic Zone	=	II,III,IV,V
Soil type	=	Medium Soil
Load Data:		
DL	=	4.75 KN /m <sup>2</sup>
LL	=	4 KN /m <sup>2</sup> (For Commercial And Office Build.)



Load 1

Fig 3.8: THIRTEEN STOREY

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# Chapter:4

# Cost Calculation of Different Storey Heights

#### 4.1 For One Storey:

#### **For gravity load:**

Total concrete	$= 51.18 \text{ m}^3$
Total steel (By IS 456)	= 4676.32 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= <b>Rs. 1,12,596=00</b>
COST OF STEEL (BY IS 456)	= Rs. 1,07,556=00
TOTAL COST	= <b>Rs. 2,20,152=00</b>

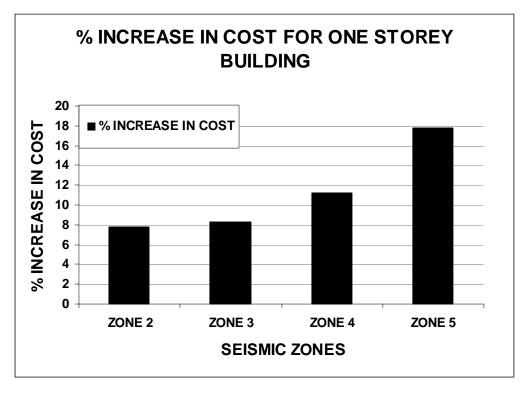


Fig 4.1: % Increase in cost for One storey building

### For seismic load :

## ZONE II

Total concrete	$= 51.18 \text{ m}^3$
Total steel (By IS 456)	= 4567.72 kg
Total steel (By IS 13920)	= 5750.94 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 1,12,596=00
COST OF STEEL (BY IS 456)	= Rs. 1,05,057=00
COST OF STEEL (BY IS 13920)	= Rs 1,32,271=00
TOTAL COST (BY IS 456)	= Rs. 2,17,653=00
TOTAL COST (BY IS 13920)	= Rs 2,38,308=00

## ZONE III

Total concrete	$= 51.18 \text{ m}^3$
Total steel (By IS 456)	= 4590.04 kg
Total steel (By IS 13920)	= 5891.74 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= <b>Rs. 1,12,596=00</b>
COST OF STEEL (BY IS 456)	= Rs. 1,05,571=00
COST OF STEEL (BY IS 13920)	= Rs 1,35,510=00
TOTAL COST (BY IS 456)	= Rs. 2,17,653=00
TOTAL COST (BY IS 13920)	= Rs 2,48,106=00

### ZONE IV

Total concrete	$= 51.18 \text{ m}^3$
Total steel (By IS 456)	= 4833.93 kg

Total steel (By IS 13920)	= 6012.94  kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 1,12,596=00
COST OF STEEL (BY IS 456)	= <b>Rs. 1,11,180=00</b>
COST OF STEEL (BY IS 13920)	= Rs 1,38,298=00
TOTAL COST (BY IS 456)	= Rs. 2,23,776=00
TOTAL COST (BY IS 13920)	= Rs 2,50,893=00

## ZONE V

Total concrete	$= 51.18 \text{ m}^3$
Total steel (By IS 456)	= 5441.31 kg
Total steel (By IS 13920)	= 6378.60 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 1,12,596=00
COST OF STEEL (BY IS 456)	= Rs. 1,25,150=00
COST OF STEEL (BY IS 13920)	= Rs 1,46,707=00
TOTAL COST (BY IS 456)	= Rs. 2,37,746=00
TOTAL COST (BY IS 13920)	= Rs 2,59,303=00

# 4.2 For Three Storey

## For gravity load :

Total concrete	$= 105.59 \text{ m}^3$
Total steel (By IS 456)	= 10137.85 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 2,32,298=00
COST OF STEEL (BY IS 456)	= Rs. 2,33,170=00
TOTAL COST	= Rs. 4,65,468=00

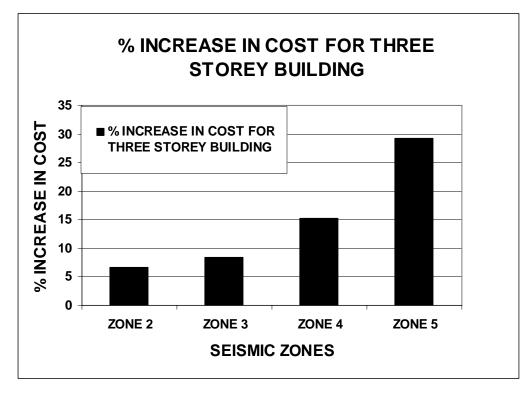


Fig 4.2: % Increase in cost for Three storey building

#### **For seismic load :**

#### ZONE II

Total concrete	$= 105.59 \text{ m}^3$
Total steel (By IS 456)	= 9664.17 kg
Total steel (By IS 13920)	= 11466.42 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 2,32,298=00

COST OF CONCRETE	- NS. 2,32,290–00
COST OF STEEL (BY IS 456)	= <b>Rs.</b> 2,22,276=00

COST OF STEEL (BY IS 13920)	= Rs 2,63,728=00
TOTAL COST (BY IS 456)	= Rs. 4,54,574=00
TOTAL COST (BY IS 13920)	= Rs 4,96,025=00

### ZONE III

Total concrete	$= 105.59 \text{ m}^3$
Total steel (By IS 456)	= 9984.03 kg
Total steel (By IS 13920)	= 11831.21 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 2,32,298=00
COST OF STEEL (BY IS 456)	= Rs. 2,29,633=00
COST OF STEEL (BY IS 13920)	= Rs 2,72,118=00
TOTAL COST (BY IS 456)	= Rs. 4,61,931=00
TOTAL COST (BY IS 13920)	= Rs 5,04,416=00

## ZONE IV

Total concrete	$= 105.59 \text{ m}^3$
Total steel (By IS 456)	= 11328.02 kg
Total steel (By IS 13920)	= 13206.38 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 2,32,298=00
COST OF STEEL (BY IS 456)	= Rs. 2,60,5
COST OF STEEL (BY IS 13920)	= Rs 3,03,746=00
TOTAL COST (BY IS 456)	= Rs. 4,92,842=00
TOTAL COST (BY IS 13920)	= Rs 5,36,044=00

## ZONE V

= 1	otal concrete	$= 117.29 \text{ m}^3$
= 13	<b>Cotal steel (By IS 456)</b>	= 13136.84 kg
= 14	Total steel (By IS 13920)	= 14939.60 kg
= <b>R</b>	Cost of Concrete Per m <sup>3</sup>	$= \text{Rs.2200/m}^3$
= <b>R</b>	Cost of Steel Per Kg	= <b>Rs.23/kg</b>
= 1; = 14 = R	Cotal steel (By IS 456) Cotal steel (By IS 13920) Cost of Concrete Per m <sup>3</sup>	= 13136.84 kg = 14939.60 kg = Rs.2200/ m <sup>3</sup>

COST OF CONCRETE	= Rs. 2,58,038=00
COST OF STEEL (BY IS 456)	= Rs. 3,02,147=00
COST OF STEEL (BY IS 13920)	= Rs 3,43,610=00
TOTAL COST (BY IS 456)	= Rs. 5,60,185=00
TOTAL COST (BY IS 13920)	= Rs 6,01,648=00

# 4.3 For Four Storey :

# For gravity load

Total concrete	$= 147.71 \text{ m}^3$
Total steel (By IS 456)	= 13151.44 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 3,24,962=00
COST OF STEEL (BY IS 456)	= Rs. 3,02,483=00
TOTAL COST	= <b>Rs. 6,27,445</b>

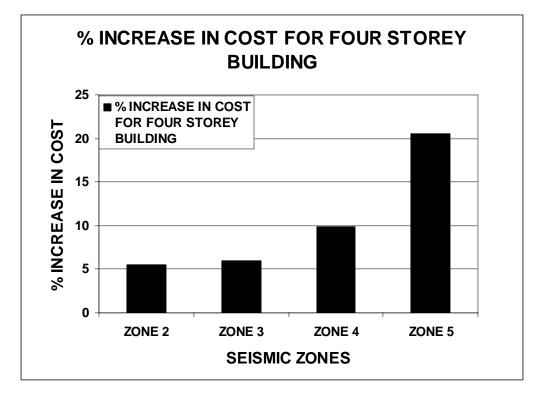


Fig 4.3: % Increase in cost for Four storey building

#### **ZONE II**

Total concrete	$= 147.71 \text{ m}^3$
Total steel (By IS 456)	= 12496 kg
Total steel (By IS 13920)	= 14663.56 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 3,24,962=00
COST OF STEEL (BY IS 456)	= Rs. 2,87,408=00
COST OF STEEL (BY IS 13920)	= Rs 3,37,262=00
TOTAL COST (BY IS 456)	= Rs. 6,12,370=00
TOTAL COST (BY IS 13920)	= Rs 6,62,224=00

## ZONE III

Total concrete	$= 147.71 \text{ m}^3$
Total steel (By IS 456)	= 12591.84 kg
Total steel (By IS 13920)	= 14783 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 3,24,962=00
COST OF STEEL (BY IS 456)	= Rs. 2,89,612=00
COST OF STEEL (BY IS 13920)	= Rs 3,40,009=00
TOTAL COST (BY IS 456)	= Rs. 6,14,574=00
TOTAL COST (BY IS 13920)	= Rs 6,64,971=00

#### ZONE IV

Total concrete	$= 147.71 \text{ m}^3$
Total steel (By IS 456)	= 13594.43 kg
Total steel (By IS 13920)	= 15845.47 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 3,24,962=00
COST OF STEEL (BY IS 456)	= Rs. 3,12,672=00
COST OF STEEL (BY IS 13920)	= Rs 3,64,446=00
TOTAL COST (BY IS 456)	= Rs. 6,37,634=00
TOTAL COST (BY IS 13920)	= Rs 6,89,408=00

# ZONE V

Total concrete	$= 147.71 \text{ m}^3$
Total steel (By IS 456)	= 16497.16 kg
Total steel (By IS 13920)	= 18765.2 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>

Rs.23/kg
K

COST OF CONCRETE	= Rs. 3,24,962=00
COST OF STEEL (BY IS 456)	= Rs. 3,79,435=00
COST OF STEEL (BY IS 13920)	= Rs 4,31,600=00
TOTAL COST (BY IS 456)	= Rs. 7,04,397=00
TOTAL COST (BY IS 13920)	= Rs 7,56,562=00

#### **<u>4.4 For Five Storey</u>**

For gravity load :	
Total concrete	= 183.59 m <sup>3</sup>
Total steel (By IS 456)	= 16579.52 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= <b>Rs. 4,03,898=00</b>

COST OF CONCRETE	= Rs. 4,03,898=00
COST OF STEEL (BY IS 456)	= Rs. 3,81,329=00
TOTAL COST	= Rs. 7,85,227=00

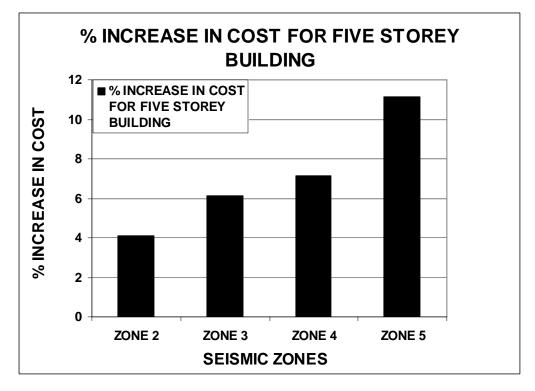


Fig 4.4: % Increase in cost for Five storey building

#### **ZONE II**

Total concrete	$= 183.59 \text{ m}^3$
Total steel (By IS 456)	= 19000.00 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 4,03,898=00
COST OF STEEL (BY IS 456)	= Rs. 3,77,923=00
COST OF STEEL (BY IS 13920)	= Rs 4,37,006=00
TOTAL COST (BY IS 456)	= Rs. 7,81,821=00
TOTAL COST (BY IS 13920)	= Rs 8,40,904=00

## ZONE III

Total concrete	$= 183.59 \text{ m}^3$
Total steel (By IS 456)	= 18482 kg
Total steel (By IS 13920)	= 21061 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 4,03,898=00
COST OF STEEL (BY IS 456)	= Rs. 4,25,085=00
COST OF STEEL (BY IS 13920)	= Rs 4,84,400=00
TOTAL COST (BY IS 456)	= Rs. 8,28,983=00
TOTAL COST (BY IS 13920)	= Rs 8,88,298=00

#### ZONE IV

Total concrete	$= 183.59 \text{ m}^3$
Total steel (By IS 456)	= 15796.39 kg
Total steel (By IS 13920)	= 18325.65 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 4,03,898=00
COST OF STEEL (BY IS 456)	= Rs. 3,63,317=00
COST OF STEEL (BY IS 13920)	= Rs 4,21,490=00
TOTAL COST (BY IS 456)	= Rs. 7,67,215=00
TOTAL COST (BY IS 13920)	= Rs 8,25,388=00

# ZONE V

Total concrete	$= 183.59 \text{ m}^3$
Total steel (By IS 456)	= 16801.74 kg
Total steel (By IS 13920)	= 19354.7 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>

Cost of Steel Per Kg	= Rs.23/kg
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COST OF CONCRETE	= Rs. 4,03,898=00
COST OF STEEL (BY IS 456)	= Rs. 3,86,440=00
COST OF STEEL (BY IS 13920)	= Rs 4,45,158=00
TOTAL COST (BY IS 456)	= Rs. 7,90,338=00
TOTAL COST (BY IS 13920)	= Rs 8,49,056=00
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**<u>4.5 For Seven Storey:</u>** 

For		laad
ror	gravity	10aa

Total concrete	$= 267.31 \text{ m}^3$
Total steel (By IS 456)	= 24578.4 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 5,88,082=00
COST OF STEEL (BY IS 456)	= Rs. 5,65,303=00
TOTAL COST	= Rs. 11,53,385=00

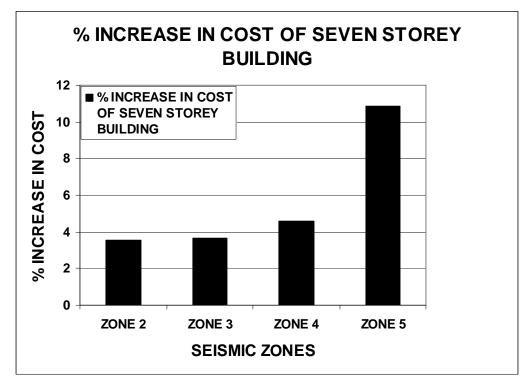


Fig 4.5: % Increase in cost for Seven storey building

#### ZONE II

Total concrete	$= 267.31 \text{ m}^3$
Total steel (By IS 456)	= 23051.13 kg
Total steel (By IS 13920)	= 26361.43 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 5,88,082=00
COST OF STEEL (BY IS 456)	= Rs. 5,30,176=00
COST OF STEEL (BY IS 13920)	= Rs 6,06,313=00
TOTAL COST (BY IS 456)	= Rs. 11,36,489=00
TOTAL COST (BY IS 13920)	= Rs 11,94,395=00

## ZONE III

Total concrete	$= 267.31 \text{ m}^3$
Total steel (By IS 456)	= 23113.15 kg
Total steel (By IS 13920)	= 26417.40 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= <b>Rs. 5,88,082=00</b>
COST OF STEEL (BY IS 456)	= Rs. 5,31,602=00
COST OF STEEL (BY IS 13920)	= Rs 6,07,600=00
TOTAL COST (BY IS 456)	= Rs. 11,19,684=00
TOTAL COST (BY IS 13920)	= Rs 11,95,682=00

## ZONE IV

Total concrete	$= 267.31 \text{ m}^3$
Total steel (By IS 456)	= 23497.4 kg
Total steel (By IS 13920)	= 26870.6 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= <b>Rs. 5,88,082=00</b>
COST OF STEEL (BY IS 456)	= Rs. 5,40,440=00
COST OF STEEL (BY IS 13920)	= Rs 6,18,023=00
TOTAL COST (BY IS 456)	= Rs. 11,28,522=00
TOTAL COST (BY IS 13920)	= Rs 12,06,105=00

#### ZONE V

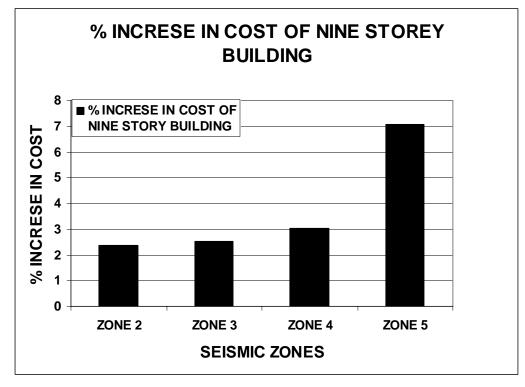
Total concrete	$= 267.31 \text{ m}^3$
Total steel (By IS 456)	= 26625.72 kg
Total steel (By IS 13920)	= 30022 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

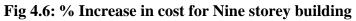
COST OF CONCRETE	= Rs. 5,88,082=00
COST OF STEEL (BY IS 456)	= Rs. 6,12,392=00
COST OF STEEL (BY IS 13920)	= Rs 6,90,506=00
TOTAL COST (BY IS 456)	= Rs. 12,00,474=00
TOTAL COST (BY IS 13920)	= Rs 12,78,588=00

#### **<u>4.6 For Nine Storey:</u>**

For gravity load	
Total concrete	$= 376.10 \text{ m}^3$
Total steel (By IS 456)	= 32988.1 kg

Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 8,27,420=00
COST OF STEEL (BY IS 456)	= Rs. 7,58,726=00
TOTAL COST	= Rs. 15,86,146=00





#### ZONE II

Total concrete	= 376.10 m <sup>3</sup>
Total steel (By IS 456)	= 30573.2 kg
Total steel (By IS 13920)	= 34612.2 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 8,27,420=00
COST OF STEEL (BY IS 456)	= Rs. 7,03,184=00

COST OF STEEL (BY IS 13920)	= Rs 7,96,081=00
TOTAL COST (BY IS 456)	= Rs. 15,30,604=00
TOTAL COST (BY IS 13920)	= Rs 16,23,501=00

#### ZONE III

Total concrete	$= 376.10 \text{ m}^3$
Total steel (By IS 456)	= 30680.8 kg
Total steel (By IS 13920)	= 34725.9 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 8,27,420=00
COST OF STEEL (BY IS 456)	= Rs. 7,05,658=00
COST OF STEEL (BY IS 13920)	= Rs 7,98,696=00
TOTAL COST (BY IS 456)	= Rs. 15,33,078=00
TOTAL COST (BY IS 13920)	= Rs 16,26,116=00

## ZONE IV

Total concrete	= 376.10 m <sup>3</sup>
Total steel (By IS 456)	= 31041.73 kg
Total steel (By IS 13920)	= 35085.2 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= <b>Rs. 8,27,420=00</b>
COST OF STEEL (BY IS 456)	= Rs. 7,13,960=00
COST OF STEEL (BY IS 13920)	= Rs 8,06,960=00
TOTAL COST (BY IS 456)	= Rs. 15,41,380=00
TOTAL COST (BY IS 13920)	= Rs 16,34,380=00

## ZONE V

Total concrete	= 376.10 m <sup>3</sup>
Total steel (By IS 456)	= 33761.24 kg
Total steel (By IS 13920)	= 37867.65 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

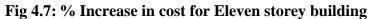
COST OF CONCRETE	= Rs. 8,27,420=00
COST OF STEEL (BY IS 456)	= <b>Rs. 7,76,508=00</b>
COST OF STEEL (BY IS 13920)	= Rs 8,70,956=00
TOTAL COST (BY IS 456)	= Rs. 16,03,929=00
TOTAL COST (BY IS 13920)	= Rs 16,98,376=00

# **4.7 For Eleven Storey**

For gravity load :	
Total concrete	= 507.47 m <sup>3</sup>
Total steel (By IS 456)	= 42180.2 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 11,16,434=00
COST OF STEEL (BY IS 456)	= <b>Rs. 9,70,145=00</b>
TOTAL COST	= Rs. 20,86,579=00





#### ZONE II

Total concrete	= 507.47 m <sup>3</sup>
Total steel (By IS 456)	= 38742.84 kg
Total steel (By IS 13920)	= 43816.42 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 11,16,434=00
COST OF STEEL (BY IS 456)	= Rs. 8,91,085=00
COST OF STEEL (BY IS 13920)	= Rs 10,07,778=00
TOTAL COST (BY IS 456)	= Rs. 18,98,863=00
TOTAL COST (BY IS 13920)	= Rs 21,24,212=00

## ZONE III

COST OF CONCRETE	= Rs. 11,16,434=00
COST OF STEEL (BY IS 456)	= Rs. 8,94,756=00
COST OF STEEL (BY IS 13920)	= Rs 10,11,839=00
TOTAL COST (BY IS 456)	= Rs. 20,11,190=00
TOTAL COST (BY IS 13920)	= Rs 21,28,273=00

## ZONE IV

= 507.47 m <sup>3</sup>
= 39306.41 kg
= 44388.63 kg
= Rs.2200/m <sup>3</sup>
= Rs.23/kg

COST OF CONCRETE	= Rs. 11,16,434=00
COST OF STEEL (BY IS 456)	= <b>Rs. 9,04,047=00</b>
COST OF STEEL (BY IS 13920)	= Rs 10,20,938=00
TOTAL COST (BY IS 456)	= Rs. 20,20,481=00
TOTAL COST (BY IS 13920)	= Rs 21,37,372=00

#### ZONE V

Total concrete	= 507.47 m <sup>3</sup>
Total steel (By IS 456)	= 41842.76 kg
Total steel (By IS 13920)	= 47001.25 kg

Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

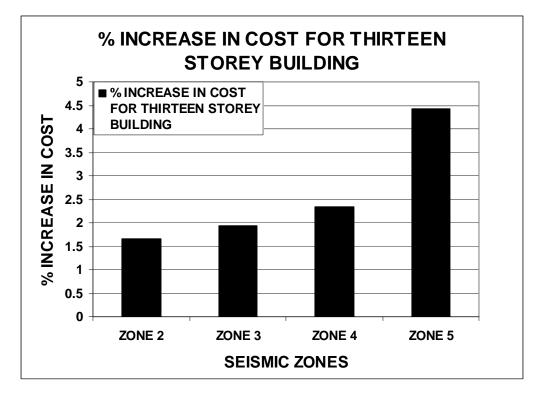
COST OF CONCRETE	= Rs. 11,16,434=00
COST OF STEEL (BY IS 456)	= Rs. 9,62,383=00
COST OF STEEL (BY IS 13920)	= Rs 10,81,029=00
TOTAL COST (BY IS 456)	= Rs. 20,78,817=00
TOTAL COST (BY IS 13920)	= Rs 21,97,463=00

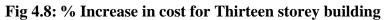
## **<u>4.8 For Thirteen Storey :</u>**

# For gravity load:

Total concrete	$= 664.38 \text{ m}^3$
Total steel (By IS 456)	= 52383.5 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 14,61,636=00
COST OF STEEL (BY IS 456)	= Rs. 12,04,820=00
TOTAL COST	= Rs. 26,66,456=00





#### **ZONE II**

Total concrete	$= 664.38 \text{ m}^3$
Total steel (By IS 456)	= 47557.7 kg
Total steel (By IS 13920)	= 54312.3 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 14,61,636=00
COST OF STEEL (BY IS 456)	= Rs. 10,93,827=00
COST OF STEEL (BY IS 13920)	= Rs 12,49,183=00
TOTAL COST (BY IS 456)	= Rs. 25,55,463=00
TOTAL COST (BY IS 13920)	= Rs 27,10,819=00

## ZONE III

Total concrete	$= 664.38 \text{ m}^3$
Total steel (By IS 456)	= 47851.42 kg
Total steel (By IS 13920)	= 54627.2 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	$= \mathbf{Rs.23/kg}$

COST OF CONCRETE	= Rs. 14,61,636=00
COST OF STEEL (BY IS 456)	= Rs. 11,00,583=00
COST OF STEEL (BY IS 13920)	= Rs 12,56,426=00
TOTAL COST (BY IS 456)	= Rs. 25,62,219=00
TOTAL COST (BY IS 13920)	= Rs 27,18,062=00

#### ZONE IV

Total concrete	$= 664.38 \text{ m}^3$
Total steel (By IS 456)	= 48288.81 kg
Total steel (By IS 13920)	= 55096.06 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/ m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= Rs. 14,61,636=00
COST OF STEEL (BY IS 456)	= Rs. 11,10,643=00
COST OF STEEL (BY IS 13920)	= Rs 12,67,209=00
TOTAL COST (BY IS 456)	= Rs. 25,72,279=00
TOTAL COST (BY IS 13920)	= Rs 27,28,845=00

# ZONE V

Total concrete	$= 664.38 \text{ m}^3$
Total steel (By IS 456)	= 50656.12 kg
Total steel (By IS 13920)	= 57504.86 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>

Cost of Steel Per Kg	= Rs.23/kg
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COST OF CONCRETE	= Rs. 14,61,636=00
COST OF STEEL (BY IS 456)	= Rs. 11,65,091=00
COST OF STEEL (BY IS 13920)	= Rs 13,22,612=00
TOTAL COST (BY IS 456)	= Rs. 26,26,727=00
TOTAL COST (BY IS 13920)	= Rs 27,84,248=00

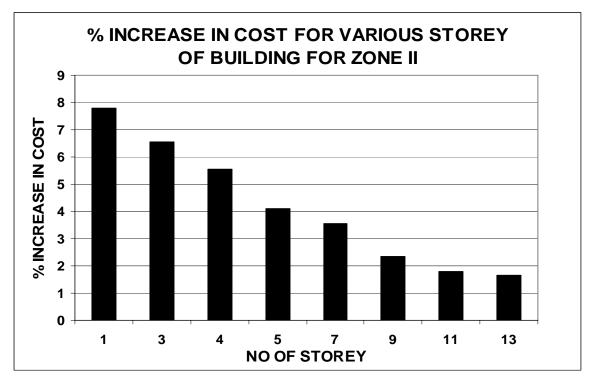


Fig 4.9: % Increase in cost for various storey of building for Zone II

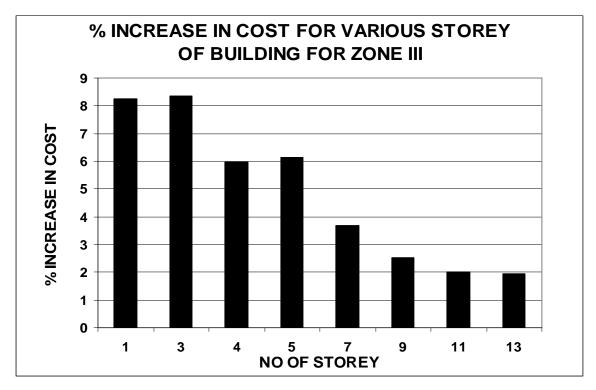


Fig 4.10: % Increase in cost for various storey of building for Zone III

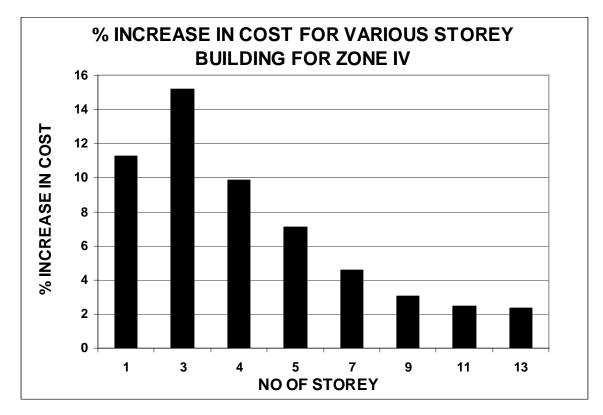


Fig 4.11: % Increase in cost for various storey of building for Zone IV

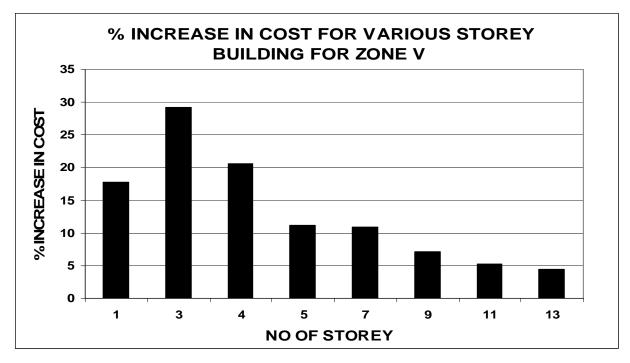


Fig 4.12: % Increase in cost for various storey of building for Zone V

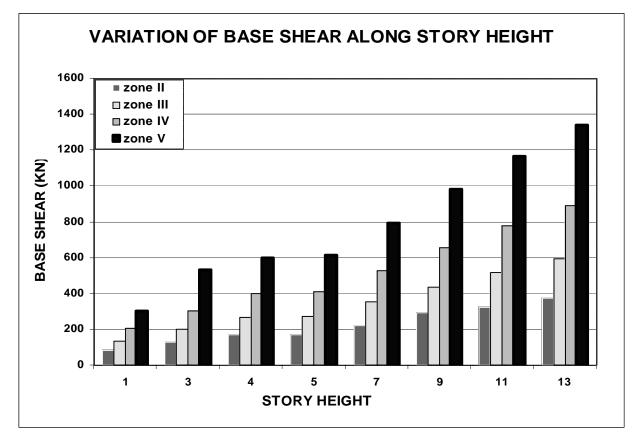


Fig 4.13: Increase in Base Shear for different storey height of buildings.

# Chapter:5

# Typical Field Example of School Building(Ref. 7)

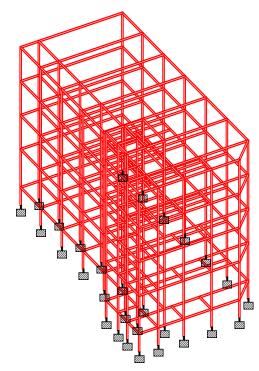
## 5.1 <u>Building-1 :</u>

ζ**x** 

#### For gravity load :

Total concrete	$= 379.38 \text{ m}^3$
Total steel (By IS 456)	= 18786.7 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 8,34,636=00
COST OF STEEL (BY IS 456)	= Rs. 4,32,094=00
TOTAL COST	= Rs. 12,66,730=00



Load 3

Fig 5.1: BUILDING-1

#### **ZONE IV**

Total concrete	$= 379.38 \text{ m}^3$
Total steel (By IS 456)	= 29788.62 kg
Total steel (By IS 13920)	= 34113.62 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= <b>Rs. 8,34,636=00</b>
COST OF STEEL (BY IS 456)	= Rs. 6,85,138=00
COST OF STEEL (BY IS 13920)	= Rs 7,84,603=00
TOTAL COST (BY IS 456)	= Rs. 15,19,774=00
TOTAL COST (BY IS 13920)	= Rs 16.19.239=00

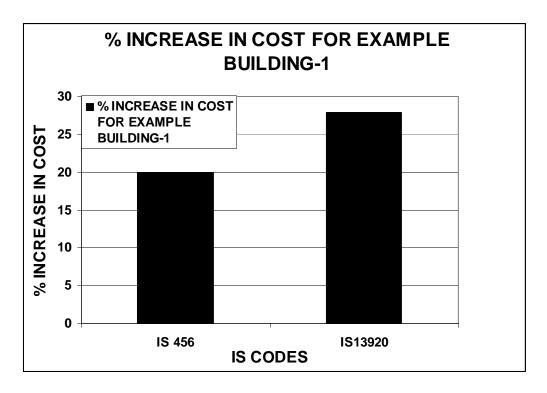


Fig 5.2: % Increase in cost for example building-1

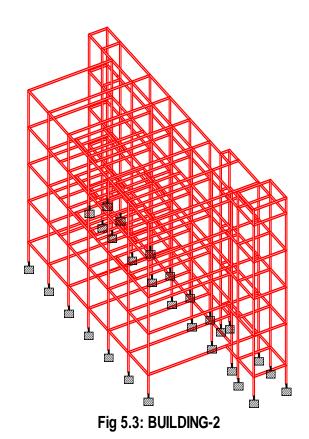
# 5.2 <u>Building-2 :</u>

# For gravity load :

¥ z

Total concrete	$= 402.23 \text{ m}^3$
Total steel (By IS 456)	= 20992.35 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 8,84,906=00
COST OF STEEL (BY IS 456)	= Rs. 4,82,824=00

COST OF STEEL (BY IS 456)	= Rs. 4,82,824=00
TOTAL COST	= Rs. 13,67,730=00



Load 3

#### ZONE IV

Total concrete	$= 402.23 \text{ m}^3$
Total steel (By IS 456)	= 30301.9 kg
Total steel (By IS 13920)	= 35286.3 kg
Cost of Concrete Per m <sup>3</sup>	= <b>Rs.2200</b> / m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg

COST OF CONCRETE	= Rs. 8,84,906=00
COST OF STEEL (BY IS 456)	= Rs. 6,96,944=00
COST OF STEEL (BY IS 13920)	= Rs 8,11,585=00
TOTAL COST (BY IS 456)	= Rs. 15,81,850=00
TOTAL COST (BY IS 13920)	= Rs 16,96,491=00

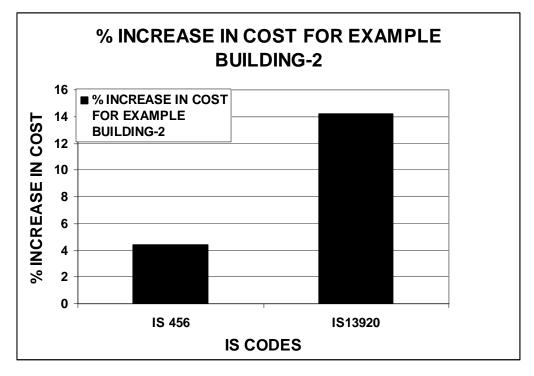


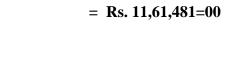
Fig 5.4: % Increase in cost for example building-2

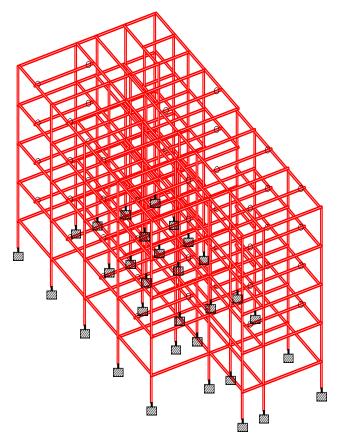
# 5.3 Building-3:

¥ z

For gravity load :-

Total concrete	$= 355.03 \text{ m}^3$
Total steel (By IS 456)	= 16539.75 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 7,81,066=00
COST OF STEEL (BY IS 456)	= Rs. 3,80,415=00
TOTAL COST	= Rs. 11,61,481=00





Load 3

Fig 5.5: BUILDING-3

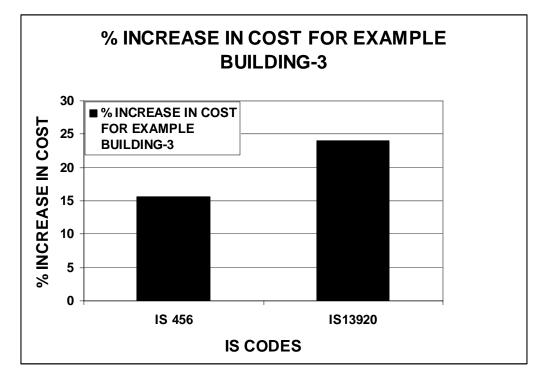


Fig 5.6: % Increase in cost for example building-3

#### **ZONE IV**:

Total concrete	$= 355.03 \text{ m}^3$
Total steel (By IS 456)	= 18773 kg
Total steel (By IS 13920)	= 23704 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= <b>Rs. 7,81,066=00</b>
COST OF STEEL (BY IS 456)	= Rs. 4,31,779=00
COST OF STEEL (BY IS 13920)	= Rs 5,45,192=00
TOTAL COST (BY IS 456)	= Rs. 12,12,845=00
TOTAL COST (BY IS 13920)	= Rs 13,26,258=00

# 5.4 Building-4:

¥ у

# For gravity load :

Total concrete	=	<b>359.97</b> m <sup>3</sup>
Total steel (By IS 456)	=	19514.1 kg
Cost of Concrete Per m <sup>3</sup>	=	<b>Rs.2200/</b> m <sup>3</sup>
Cost of Steel Per Kg	=	Rs.23/kg
COST OF CONCRETE	=	Rs. 7,91,934=00
COST OF STEEL (BY IS 456)	=	Rs. 4,48,825=00
TOTAL COST	=	Rs. 12,40,759=00

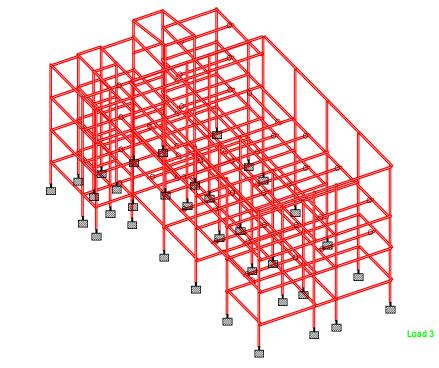
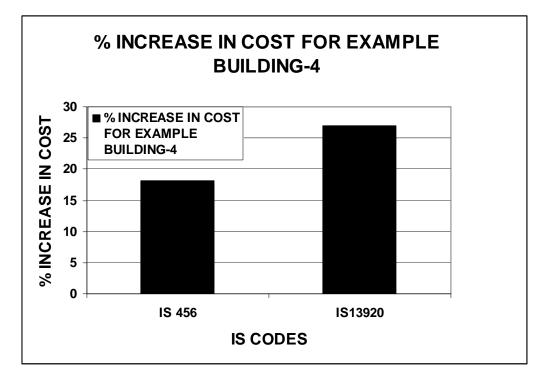
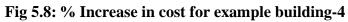


Fig 5.7: BUILDING-4





#### **ZONE IV**

Total concrete	$= 359.97 \text{ m}^3$
Total steel (By IS 456)	= 29317.54 kg
Total steel (By IS 13920)	= 34067.36 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= <b>Rs.23/kg</b>

COST OF CONCRETE	= <b>Rs. 7,91,934=00</b>
COST OF STEEL (BY IS 456)	= Rs. 6,74,304=00
COST OF STEEL (BY IS 13920)	= Rs 7,83,550=00
TOTAL COST (BY IS 456)	= Rs. 14,66,238=00
TOTAL COST (BY IS 13920)	= Rs 15,75,484=00

# 5.5 Building-5:

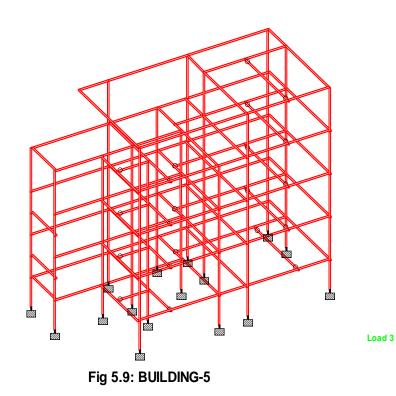
TOTAL COST

ζ**x** 

## For gravity load :

Total concrete	$= 148.31 \text{ m}^3$
Total steel (By IS 456)	= 8130.65 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 3,26,282=00
COST OF STEEL (BY IS 456)	= Rs. 1,87,005=00

= Rs. 5,13,287=00



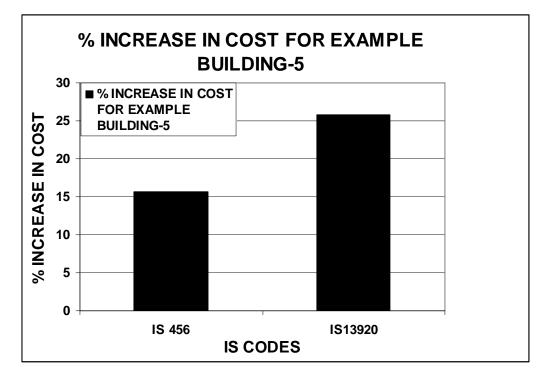


Fig 5.10: % Increase in cost for example building-5

#### **ZONE IV**

Total concrete	$= 148.31 \text{ m}^3$
Total steel (By IS 456)	= 11636.8 kg
Total steel (By IS 13920)	= 13896.42 kg
Cost of Concrete Per m <sup>3</sup>	= Rs.2200/m <sup>3</sup>
Cost of Steel Per Kg	= Rs.23/kg
COST OF CONCRETE	= Rs. 3,26,282=00
COST OF STEEL (BY IS 456)	= Rs. 6,74,304=00
COST OF STEEL (BY IS 13920)	= Rs 2,67,646=00
TOTAL COST (BY IS 456)	= Rs. 5,93,928=00
TOTAL COST (BY IS 13920)	

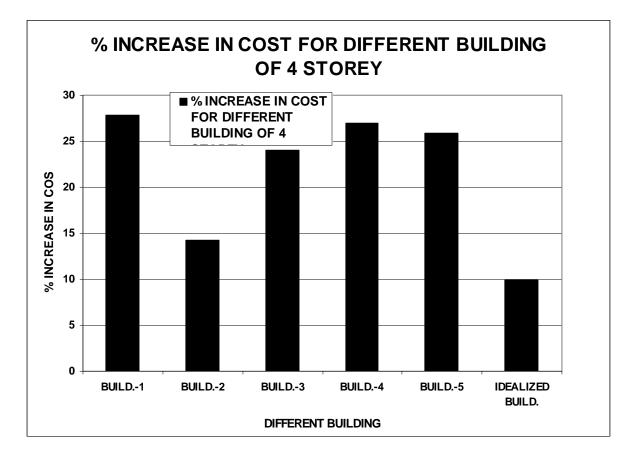


Fig 5.11: % Increase in cost for different building of 4 storey

## <u>Chapter:6</u> <u>Results &Discussion</u>

From fig 4.1 it is clear that as the seismic zone increases the value of % increase in cost increases. Fig 4.1 also shows that as value of seismic zone changes from zone II to zone IV, % increase in cost is gradual from zone II to zone IV while it becomes steep from zone IV to zone V. For one storey building the value of percentage increase in cost from zone II to zone V is varies between 7 to 17%. The three storey building also shows similar trend as that of one storey and variation in percentage increase in cost from zone II to zone V is approximately in between 6 to 30%.Four storey is similar one as one and three storey and variation in percentage increase in cost from zone II to zone V is in the range of 5 to 20%.Percentage increase in cost for five storey building is less in comparison to the building mentioned above and it ranges between 4 to 11% from zone II to zone V respectively.

From three storey it can be said that as the number of zone increases the percentage increase in cost increases gradually. Therefore similarly for seven storey building percentage increase in cost ranges between 3 to 10% as zone changes from II to V respectively. For nine storey the value of percentage increase in cost is further reduced and it ranges between 2 to 7%.Similarly for eleven storey and thirteen storey the value of percentage increase in cost 1 to 5% and 1 to 4% respectively. Finally it can be said that as the no of storey increases the percentage increase in cost is reduced gradually.

The value of maximum % increase in cost for zone II comes for one storey building, and it is approximately 8%. Similarly the value of maximum % increase in cost for zone III falls under three storey building, and it is around 8.4%. It is clear from fig 4.2 that value of maximum % increase in cost for zone IV comes for three storey building and it comes approximately 15%. Similarly for zone V it also comes for three storey building and it is approximately 30%.

From fig 4.9 to 4.12 it is clear that as the no of storey increases the percentage increase in cost decreases significantly. From fig 4.9 & 4.10 it is clear that as the no of storey increases the % increase in cost decreases almost gradually, while in fig 4.11 & 4.12 it is found that decreasing trend of % increase in cost is not similar as that of like fig 4.9 & 4.10. In fig 4.11 & 4.12 first of all upto three storey building there is increase in % increase of cost while afterwards there is gradual decrease in % increase in cost. Therefore it may be said that some critical point is there in terms of no of storey below which there is gradual decrease and beyond which there is first increase and then decrease.

In vertical variation, i.e. storey wise for a particular zone it can be said that for zone II percentage increase in cost varies between 7.8% to 1.67% for one storey building to thirteen storey building respectively. Similar pattern has also be seen for zone III except that for one storey building and three storey building percentage increase in cost is same around afterwards it is gradually reduced to 1.9% for thirteen storey building. Therefore it can be said that as the storey height increases the value of percentage increase in cost reduces gradually. Now somewhat different pattern has been shown in fig 4.11& 4.12. In these figures, it has been observed that percentage increase in cost first increases upto three storey building and then afterwards decreases to thirteen storey building. The percentage increase in cost for zone IV from one storey building to three storey building is about 11 to 15% respectively and then afterwards it decreases to around 2% for thirteen storey building.

From fig 5.2 it is clear that % increase in cost with IS 456 for example building-1 is around 20%. Similarly from fig 5.4, 5.6, 5.8, 5.10 it is clear that % increase in cost with IS 456 for example building-2,3,4,5 5%,16%,18% &16% respectively. Therefore it can be said that except example building-2 all others buildings have % increase in cost to be around 15 to 20%. This difference in cost even under uniform conditions shows that some other parameters like irregularity etc., also plays an important role which was not considered here.

In the study percentage increase in cost coming is more than A.K.Mittal's paper. It may be due to two reasons one is the importance factor chosen in the study is 1.5 and other one is wind load is not consider in the study.

## <u>Chapter :7</u> <u>Conclusion</u>

- Percentage increase in cost for one storey building with ductile detailing in zone II, III, IV and zone V are 7.8%, 8.25%, 11.23%, 17.8% respectively.
- Percentage increase in cost for three storey building with ductile detailing in zone II, III, IV and zone V are 6.57%, 8.37%, 15.16%, 24.26% respectively.
- Percentage increase in cost for four storey building with ductile detailing in zone II, III, IV and zone V are 5.54%, 5.98%, 9.87%, 20.57% respectively.
- Percentage increase in cost for five storey building with ductile detailing in zone II, III, IV and zone V are 4.1%, 6.13%, 7.11%, 11.13% respectively.
- Percentage increase in cost for seven storey building with ductile detailing in zone II, III, IV and zone V are 3.55%, 3.67%, 4.57%, 10.86% respectively.
- Percentage increase in cost for nine storey building with ductile detailing in zone II, III, IV and zone V are 2.36%, 2.52%, 3.04%, 7.07% respectively.
- Percentage increase in cost for eleven storey building with ductile detailing in zone II, III, IV and zone V are 1.80%, 2.00%, 2.43%, 5.31% respectively.
- Percentage increase in cost for thirteen storey building with ductile detailing in zone II, III, IV and zone V are 1.66%, 1.93%, 2.33%, 4.42% respectively.
- 9. In vertical height wise variation for different storey height in zone II percentage increase in cost ranges between 1.66% to 7.8%.
- 10. In vertical height wise variation for different storey height in zone III percentage increase in cost ranges between 1.93% to 8.37%.
- 11. In vertical height wise variation for different storey height in zone IV percentage increase in cost ranges between 2.33% to 15.16%.
- 12. In vertical height wise variation for different storey height in zone V percentage increase in cost ranges between 4.42% to 24.26%.
- 13. In example building-1 of four storey percentage increase in cost in zone IV for earthquake and ductile detailing are 19.87% and 27.83% respectively.
- 14. In example building-2 of four storey percentage increase in cost in zone IV for earthquake and ductile detailing are 4.42% and 14.19% respectively.

- 15. In example building-3 of four storey percentage increase in cost in zone IV for earthquake and ductile detailing are 18.20% and 27.00% respectively.
- 16. In example building-4 of four storey percentage increase in cost in zone IV for earthquake and ductile detailing are 15.65% and 24.04% respectively.
- 17. In example building-5 of four storey percentage increase in cost in zone IV for earthquake and ductile detailing are 15.71% and 25.84% respectively.
- 18. From above conclusions it can be said that variation of percentage increase in cost for non ductile detailing in zone IV varies between 4.42% to19.97%.
- 19. From above conclusions it can be said that variation of percentage increase in cost for ductile detailing in zone IV varies between 14.19% to27.83%.
- 20. It was observed that as the number of seismic zone increases the cost of concrete and steel in a building increases.
- 21. It has been seen that for same zone and height of building, cost is more with IS: 13920 as compared to that with IS: 456 and it is mainly contributed by an increase in quantity of steel.
- 22. For different heights of building, change in total cost of concrete and steel from one zone to other is different, and the variation is not similar for all the zones. It seems that some other particulars also effect the cost variations. Identification of those factors may be in the scope of future work.
- 23. Increase in cost of concrete and steel in example buildings has been found to be different. Some other factors, such as regularity of building ,distribution and magnitude of imposed loads etc., may be responsible for it. The trend may be further explored in a future study.
- 24. For same storey in school buildings and commercial buildings; increase in cost is different. So it can be concluded that occupancy, type of building (commercial, educational as taken in this study ) also affect the cost.

# <u>Chapter :8</u> <u>Scope Of Future Work</u>

- Increase in cost of concrete and steel in example buildings has been found to be different. Some other factors, such as regularity of building ,distribution and magnitude of imposed loads etc, may be responsible for it. The trend may be further explored in a future study.
- 2. For different heights of building, change in total cost of concrete and steel from one zone to other is different, and the variation is not similar for all the zones. It seems that some other particulars also effect the cost variations. Identification of those factors may be in the scope of future work.
- 3. In the study wind load has not been consider it can be consider in future work.

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  - (iii)www.google.com
  - (iv) www.yahoo.com

## ANNEXURE- STADD Pro 2004 Input File of Three storey

STAAD SPACE START JOB INFORMATION **ENGINEER DATE 21-Apr-05** END JOB INFORMATION **INPUT WIDTH 79** UNIT METER KN JOINT COORDINATES 1 0 0 0; 2 6.6 0 0; 3 9.05 0 0; 4 15.65 0 0; 5 0 0 6.6; 6 6.6 0 6.6; 7 9.05 0 6.6; 8 15.65 0 6.6; 9 9.05 0 9.7; 10 15.65 0 9.7; 11 0 0 13.2; 12 6.6 0 13.2; 13 9.05 0 16.3; 14 15.65 0 16.3; 15 9.05 0 19.8; 16 15.65 0 19.8; 17 0 0 19.8; 18 6.6 0 19.8; 19 9.05 0 26.4; 20 15.65 0 26.4; 21 0 0 26.4; 22 6.6 0 26.4; 26 0 2.25 0; 27 6.6 2.25 0; 28 9.05 2.25 0; 29 15.65 2.25 0; 30 0 2.25 6.6; 31 6.6 2.25 6.6; 32 9.05 2.25 6.6; 33 15.65 2.25 6.6; 34 9.05 2.25 9.7; 35 12.65 2.25 9.775; 36 0 2.25 13.2; 37 6.6 2.25 13.2; 38 15.65 2.25 9.7; 39 9.05 2.25 16.3; 40 12.65 2.25 16.225; 41 15.65 2.25 16.3; 42 9.05 2.25 19.8; 43 15.65 2.25 19.8; 44 0 2.25 19.8; 45 6.6 2.25 19.8; 46 0 2.25 26.4; 47 6.6 2.25 26.4; 48 9.05 2.25 26.4; 49 15.65 2.25 26.4; 53 12.65 0 9.775; 54 12.65 0 16.225; 55 6.6 2.25 9.7; 56 6.6 2.25 16.3; 57 12.305 0 0; 58 12.305 0 3.74; 59 12.305 0 6.6; 60 15.65 0 3.74; 61 12.305 2.25 0; 62 12.305 2.25 3.74; 63 12.305 2.25 6.6; 64 15.65 2.25 3.74; 65 0 5.55 0; 66 6.6 5.55 0; 67 9.05 5.55 0; 68 15.65 5.55 0; 69 0 5.55 6.6; 70 6.6 5.55 6.6; 71 9.05 5.55 6.6; 72 15.65 5.55 6.6; 73 9.05 5.55 9.7; 74 12.65 5.55 9.775; 75 0 5.55 13.2; 76 6.6 5.55 13.2; 77 15.65 5.55 9.7; 78 9.05 5.55 16.3; 79 12.65 5.55 16.225; 80 15.65 5.55 16.3; 81 9.05 5.55 19.8; 82 15.65 5.55 19.8; 83 0 5.55 19.8; 84 6.6 5.55 19.8; 85 0 5.55 26.4; 86 6.6 5.55 26.4; 87 9.05 5.55 26.4; 88 15.65 5.55 26.4; 92 6.6 5.55 9.7; 93 6.6 5.55 16.3; 94 12.305 5.55 0; 95 12.305 5.55 3.74; 96 12.305 5.55 6.6; 97 15.65 5.55 3.74; 98 0 8.85 0; 99 6.6 8.85 0; 100 9.05 8.85 0; 101 15.65 8.85 0; 102 0 8.85 6.6; 103 6.6 8.85 6.6; 104 9.05 8.85 6.6; 105 15.65 8.85 6.6; 106 9.05 8.85 9.7; 107 12.65 8.85 9.775; 108 0 8.85 13.2; 109 6.6 8.85 13.2; 110 15.65 8.85 9.7; 111 9.05 8.85 16.3; 112 12.65 8.85 16.225; 113 15.65 8.85 16.3; 114 9.05 8.85 19.8; 115 15.65 8.85 19.8; 116 0 8.85 19.8; 117 6.6 8.85 19.8; 118 0 8.85 26.4; 119 6.6 8.85 26.4; 120 9.05 8.85 26.4; 121 15.65 8.85 26.4; 125 6.6 8.85 9.7; 126 6.6 8.85 16.3; 127 12.305 8.85 0; 128 12.305 8.85 3.74; 129 12.305 8.85 6.6; 130 15.65 8.85 3.74; 131 0 12.15 0; 132 6.6 12.15 0; 133 9.05 12.15 0; 134 15.65 12.15 0; 135 0 12.15 6.6; 136 6.6 12.15 6.6; 137 9.05 12.15 6.6; 138 15.65 12.15 6.6; 139 9.05 12.15 9.7; 140 12.65 12.15 9.775; 141 0 12.15 13.2; 142 6.6 12.15 13.2; 143 15.65 12.15 9.7: 144 9.05 12.15 16.3: 145 12.65 12.15 16.225: 146 15.65 12.15 16.3; 147 9.05 12.15 19.8; 148 15.65 12.15 19.8; 149 0 12.15 19.8; 150 6.6 12.15 19.8; 151 0 12.15 26.4; 152 6.6 12.15 26.4;

311 150 152; 312 147 153; 313 148 154; 319 136 137; 320 150 147; 321 152 153; 322 158 142; 323 159 150; 324 158 139; 325 159 144; 326 160 134; 327 163 138; 328 162 138; 329 160 161; 330 161 163; 331 161 162; 332 146 143; 333 147 148; 334 144 145; DEFINE MATERIAL START **ISOTROPIC CONCRETE** E 2.236e+007 POISSON 0.17 **DENSITY 25** ALPHA 1e-005 **DAMP 0.05** END DEFINE MATERIAL CONSTANTS MATERIAL CONCRETE MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 194 -195 TO 229 235 TO 274 278 TO 313 319 TO 334 MEMBER PROPERTY INDIAN \*COLUMN 36 TO 57 61 62 76 TO 79 81 TO 104 108 TO 113 167 TO 190 194 TO 199 -251 TO 274 278 TO 283 PRIS YD 0.3 ZD 0.3 MEMBER PROPERTY INDIAN 1 TO 30 63 TO 75 80 114 TO 143 149 TO 166 200 TO 229 235 TO 250 284 TO 313 -319 TO 334 PRIS YD 0.35 ZD 0.25 **SUPPORTS** 1 TO 22 53 54 57 TO 60 FIXED SLAVE RIGID MASTER 26 JOINT 27 TO 49 55 56 61 TO 64 SLAVE RIGID MASTER 65 JOINT 66 TO 88 92 TO 97 SLAVE RIGID MASTER 98 JOINT 99 TO 121 125 TO 130 SLAVE RIGID MASTER 131 JOINT 132 TO 154 158 TO 163 **CUT OFF MODE SHAPE 12** DEFINE 1893 LOAD ZONE 0.24 RF 3 I 1.5 SS 2 ST 2 SELFWEIGHT LOAD 1 EOX JOINT LOAD 1 Fx 2.531 2 Fx 2.531 3 Fx 2.531 4 Fx 2.531 5 Fx 2.531 6 Fx 2.531 7 Fx 2.531 8 Fx 2.531 9 Fx 2.531 10 Fx 2.531 11 Fx 2.531

12 Fx 2.531
13 Fx 2.531
14 Fx 2.531
15 Fx 2.531
16 Fx 2.531
17 Fx 2.531
18 Fx 2.531
19 Fx 2.531
20 Fx 2.531
21 Fx 2.531
22 Fx 2.531
26 Fx 90.589
27 Fx 134.537
28 Fx 69.361
29 Fx 32.988
30 Fx 175.774
31 Fx 218.474
32 Fx 102.01
33 Fx 48.875
34 Fx 125.75
35 Fx 120.055
36 Fx 173.869
37 Fx 147.485
38 Fx 78.272
39 Fx 129.443
40 Fx 123.721
41 Fx 73.939
42 Fx 182.574
43 Fx 140.605
44 Fx 176.166
45 Fx 207.829
46 Fx 90.874
47 Fx 123.3
48 Fx 123.2
49 Fx 92.243
53 Fx 2.531
54 Fx 2.531
55 Fx 80.302
56 Fx 80.434
57 Fx 2.531
58 Fx 2.531
59 Fx 2.531
60 Fx 2.531
61 Fx 60.966
62 Fx 94.851
63 Fx 74.662
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64 Fx 55.774
65 Fx 92.215
66 Fx 134.804
67 Fx 71.886
68 Fx 34.413
69 Fx 176.944
70 Fx 217.481
71 Fx 104.917
72 Fx 50.725
73 Fx 127.737
74 Fx 120.698
75 Fx 175.215
76 Fx 147.064
77 Fx 79.442
78 Fx 131.701
79 Fx 124.191
80 Fx 75.937
81 Fx 183.475
82 Fx 141.478
83 Fx 177.272
84 Fx 207.527
85 Fx 92.451
86 Fx 124.585
87 Fx 124.489
88 Fx 93.701
92 Fx 84.45
93 Fx 84.816
94 Fx 62.028
95 Fx 95.376
96 Fx 75.653
97 Fx 56.751
98 Fx 92.994
99 Fx 134.014
100 Fx 73.361
101 Fx 34.721
102 Fx 177.102
103 Fx 214.166
104 Fx 106.128
105 Fx 51.513
106 Fx 128.568
107 Fx 120.239
107 Fx 120.239 108 Fx 175.788
109 Fx 146.066
110 Fx 79.245
111 Fx 132.889
112 Fx 123.374

2 Fz 2.531
3 Fz 2.531
4 Fz 2.531
5 Fz 2.531
6 Fz 2.531
7 Fz 2.531
8 Fz 2.531
9 Fz 2.531
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14 Fz 2.531 15 Fz 2.531
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17 Fz 2.531
18 Fz 2.531
19 Fz 2.531
20 Fz 2.531
21 Fz 2.531
22 Fz 2.531
26 Fz 90.589
27 Fz 134.537
28 Fz 69.361
29 Fz 32.988
30 Fz 175.774
31 Fz 218.474
32 Fz 102.01
33 Fz 48.875
34 Fz 125.75
35 Fz 120.055
36 Fz 173.869
37 Fz 147.485 38 Fz 78.272
39 Fz 129.443
40 Fz 123.721
41 Fz 73.939
42 Fz 182.574
43 Fz 140.605
44 Fz 176.166
45 Fz 207.829
46 Fz 90.874
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49 Fz 92.243
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61 Fz 60.966
62 Fz 94.851
63 Fz 74.662
64 Fz 55.774
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103 Fz 214.166
104 Fz 106.128 105 Fz 51.513
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109 Fz 146.066 110 Fz 79.245
111 Fz 132.889
112 Fz 123.374
113 Fz 76.836
114 Fz 182.676 115 Fz 141.191
116 Fz 177.358
117 Fz 205.317
118 Fz 93.174
119 Fz 124.692 120 Fz 124.649
120 Fz 94.36
125 Fz 85.095
126 Fz 85.487
127 Fz 61.992 128 Fz 94.577
129 Fz 75.423
130 Fz 56.423
131 Fz 84.918
132 Fz 136.364 133 Fz 59.503
134 Fz 29.033
135 Fz 172.877
136 Fz 232.877
137 Fz 91.946
138 Fz 42.575 139 Fz 120.631
140 Fz 119.974
141 Fz 169.445
142 Fz 148.411
143 Fz 76.278 144 Fz 123.093
145 Fz 123.093
146 Fz 66.982
147 Fz 182.86
148 Fz 139.624
149 Fz 173.622 150 Fz 215.319
151 Fz 85.475

152 Fz 120.201 153 Fz 119.978 154 Fz 87.331 158 Fz 75.483 159 Fz 75.554 160 Fz 58.848 161 Fz 96.078 162 Fz 73.198 163 Fz 54.606 SPECTRUM CQC 1893 TOR X 0.024 ACC SCALE 1 DAMP 0.05 LIN MIS SOIL TYPE 2 LOAD 2 EQZ SPECTRUM COC 1893 TOR Z 0.024 ACC SCALE 1 DAMP 0.05 LIN MIS SOIL TYPE 2 LOAD 3 DL **SELFWEIGHT Y -1** FLOOR LOAD YRANGE 0 12.15 FLOAD -4.75 XRANGE 0 15.65 ZRANGE 0 33 LOAD 4 LL FLOOR LOAD YRANGE 0 12.15 FLOAD -4 XRANGE 0 15.65 ZRANGE 0 33 LOAD COMB 5 1.2\*(DL+LL+EQX) 1 1.2 3 1.2 4 1.2 LOAD COMB 6 1.2\*(DL+LL+EQZ) 2 1.2 3 1.2 4 1.2 LOAD COMB 7 1.2\*(DL+LL-EQX) 1 -1.2 3 1.2 4 1.2 LOAD COMB 8 1.2\*(DL+LL-EQZ) 2 -1.2 3 1.2 4 1.2 LOAD COMB 9 1.5\*(DL+EQX) 1 1.5 3 1.5 LOAD COMB 10 1.5\*(DL+EQZ) 2 1.5 3 1.5 LOAD COMB 11 1.5\*(DL-EQX) 1 -1.5 3 1.5 LOAD COMB 12 1.5\*(DL-EQZ) 2 - 1.5 3 1.5 LOAD COMB 13 0.9\*DL+1.5\*EQX 1 1.5 3 0.9 LOAD COMB 14 0.9\*DL+1.5\*EQZ 2 1.5 3 0.9 LOAD COMB 15 0.9\*DL-1.5\*EQX 1 -1.5 3 0.9 LOAD COMB 16 0.9\*DL-1.5\*EQZ 2 - 1.5 3 0.9 LOAD COMB 17 1.5\*(DL+LL)

3 1.5 4 1.5

PERFORM ANALYSIS LOAD LIST 5 TO 16 PRINT SUPPORT REACTION PRINT STORY DRIFT START CONCRETE DESIGN **CODE INDIAN** FC 20000 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 194 TO 229 -235 TO 274 278 TO 313 319 TO 334 FYMAIN 415000 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 -194 TO 229 235 TO 274 278 TO 313 319 TO 334 FYSEC 415000 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 -194 TO 229 235 TO 274 278 TO 313 319 TO 334 MAXMAIN 25 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 194 TO 229 -235 TO 274 278 TO 313 319 TO 334 MAXSEC 20 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 194 TO 229 -235 TO 274 278 TO 313 319 TO 334 MINMAIN 10 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 194 TO 229 -235 TO 274 278 TO 313 319 TO 334 MINSEC 8 MEMB 1 TO 30 36 TO 57 61 TO 104 108 TO 143 149 TO 190 194 TO 229 -235 TO 274 278 TO 313 319 TO 334 DESIGN BEAM 1 TO 30 63 TO 75 80 114 TO 143 149 TO 166 200 TO 229 235 TO 250 -284 TO 313 319 TO 334 DESIGN COLUMN 36 TO 57 61 62 76 TO 79 81 TO 104 108 TO 113 167 TO 190 194 -195 TO 199 251 TO 274 278 TO 283 CONCRETE TAKE END CONCRETE DESIGN FINISH